



Living Clay: A Sinking Auction

By Maxime Ostroverhy



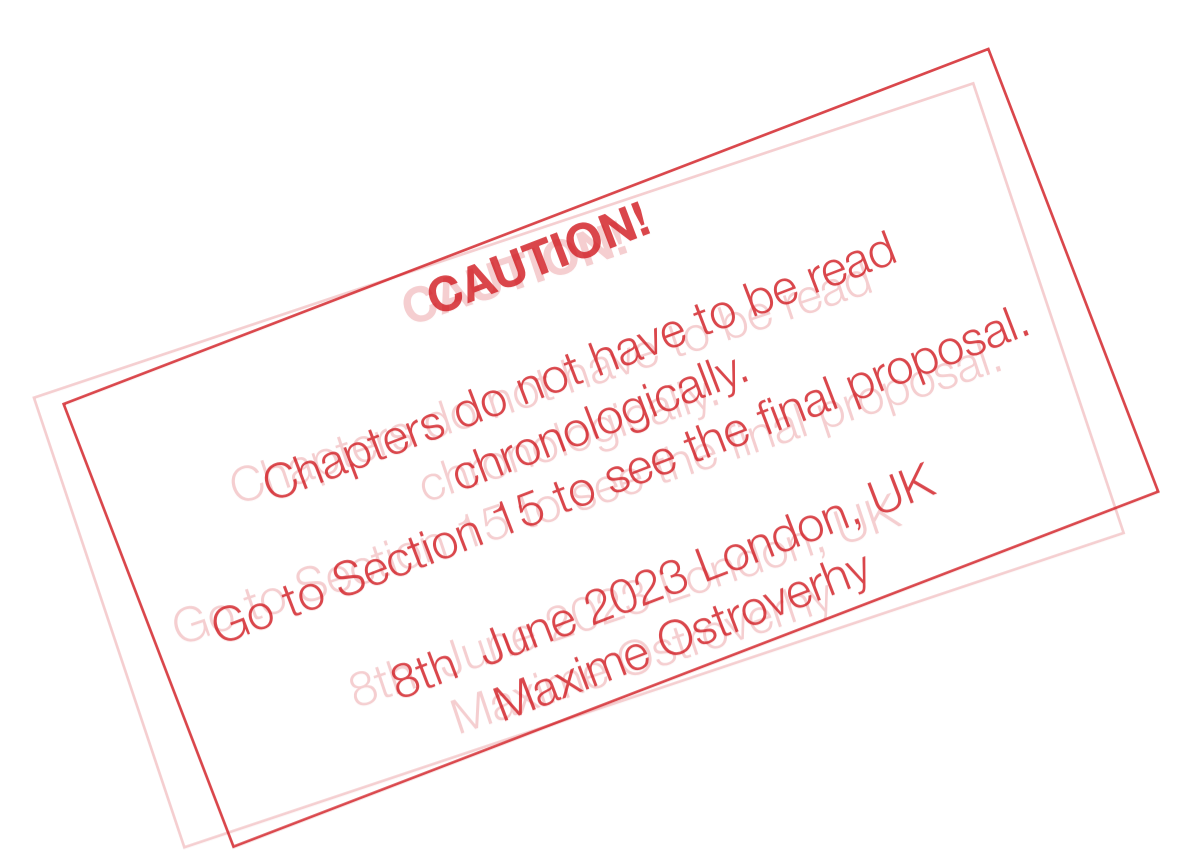
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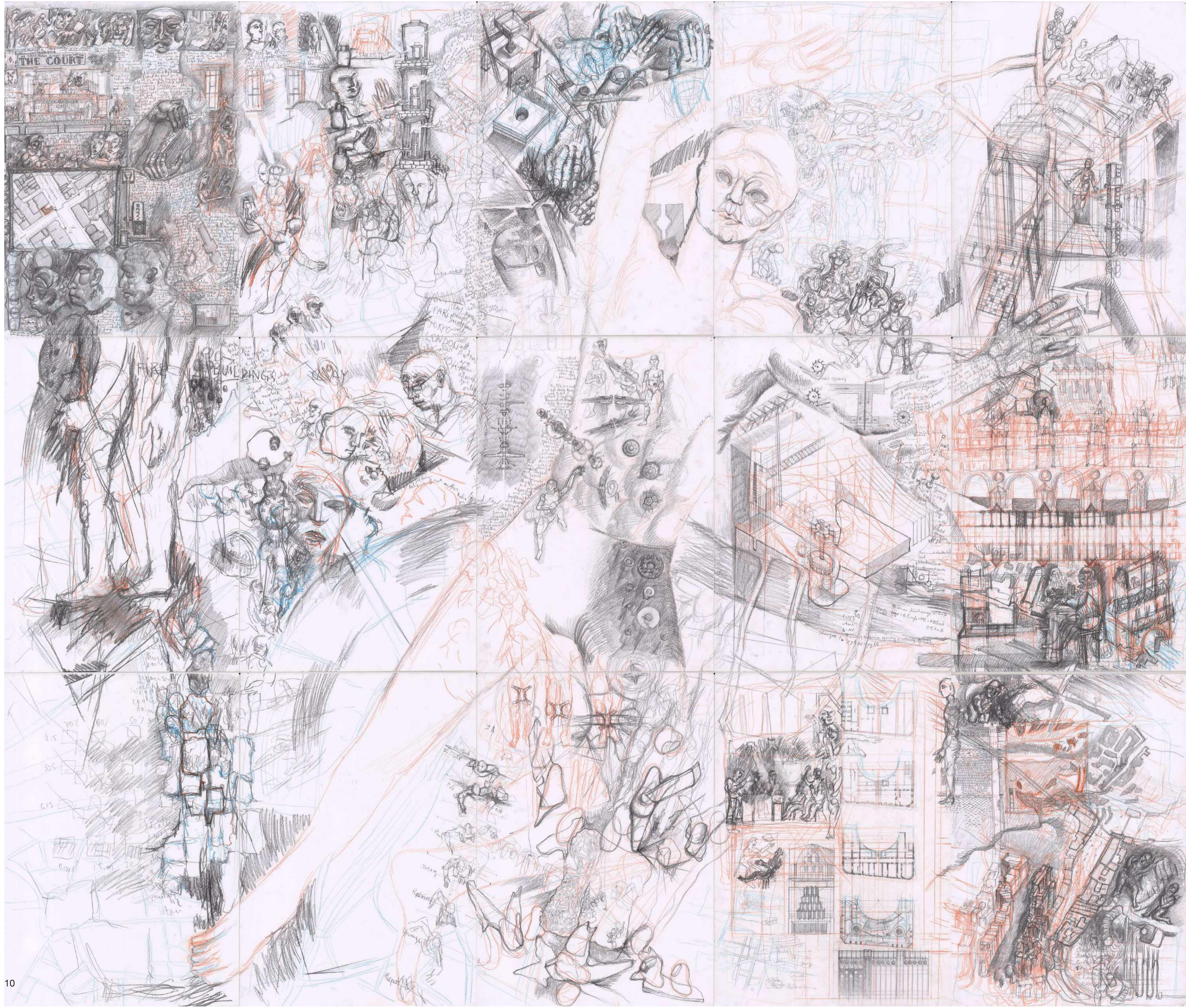
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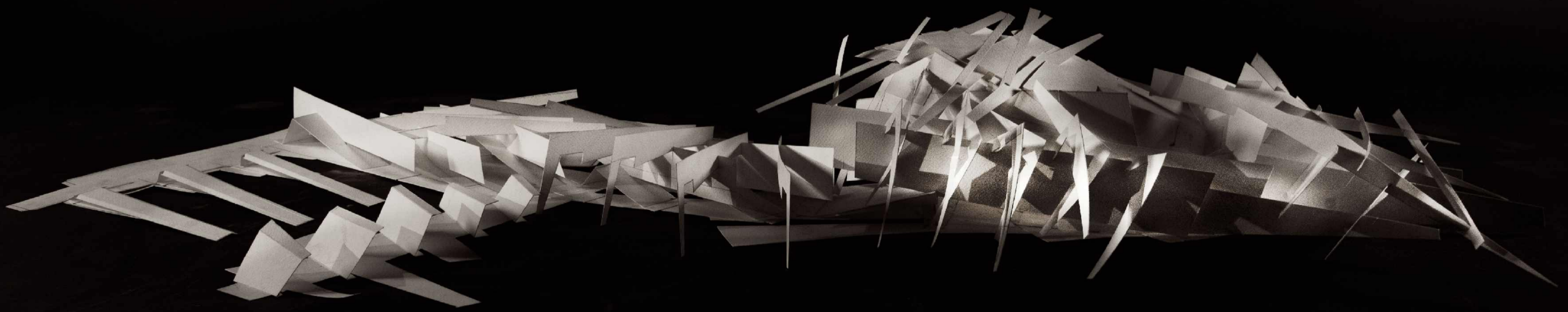


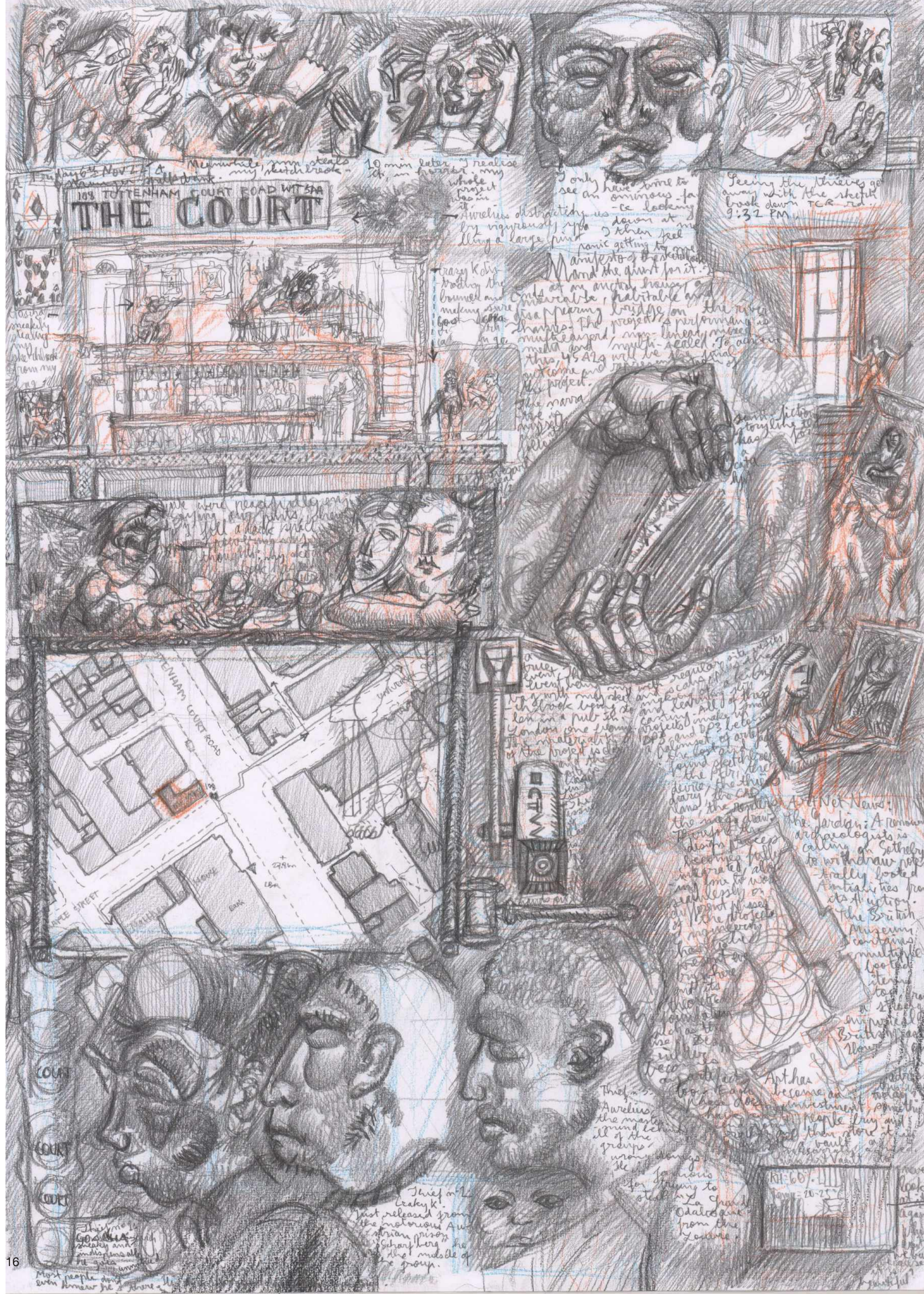
Introduction

Manifesto

This project is an auction house that sits on the Thames near Tower Bridge. It tries to highlight the imbalances and inequalities in the art world. Too often the artist is overshadowed by the dealer and then the artwork is not put into worth by being stored in a vault. It also points out the legitimacy of artworks, as they can often be stolen and then sold at an auction. In the floating bridge, the art market is brought back to the people. The symbolic bridging of the inequalities is portrayed by bringing together the north bank of the river that is historically wealthier and has the business district, with the south bank that is more industrial. The material aspect of this endeavour evolves around London clay, found on the shores of the Thames. Its properties as a building material are explored in a rigorous manner, and its therapeutic "muddying" properties are explored in a more hypothetical manner.







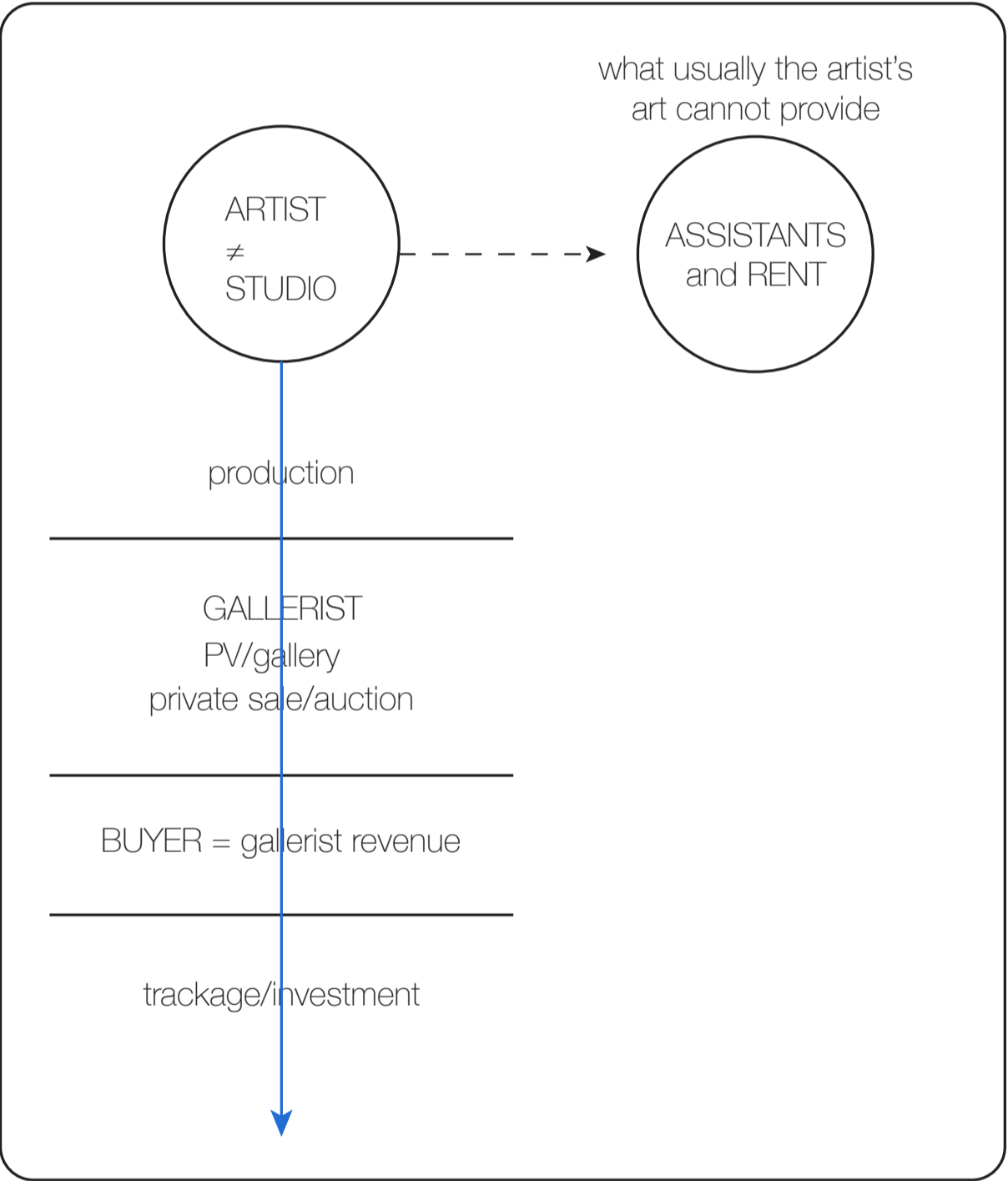
Section 1. Design Topic: Narrative

The idea of the art industry, and artworks cycle emerged when I got my sketchbook stolen with my bag in a pub on Tottenham Court Rd. Buying an artwork just as an investment can be seen as "stealing" from humanity's right to appreciate beauty. Oftentimes the artist is left out of this economy and is underpaid too.

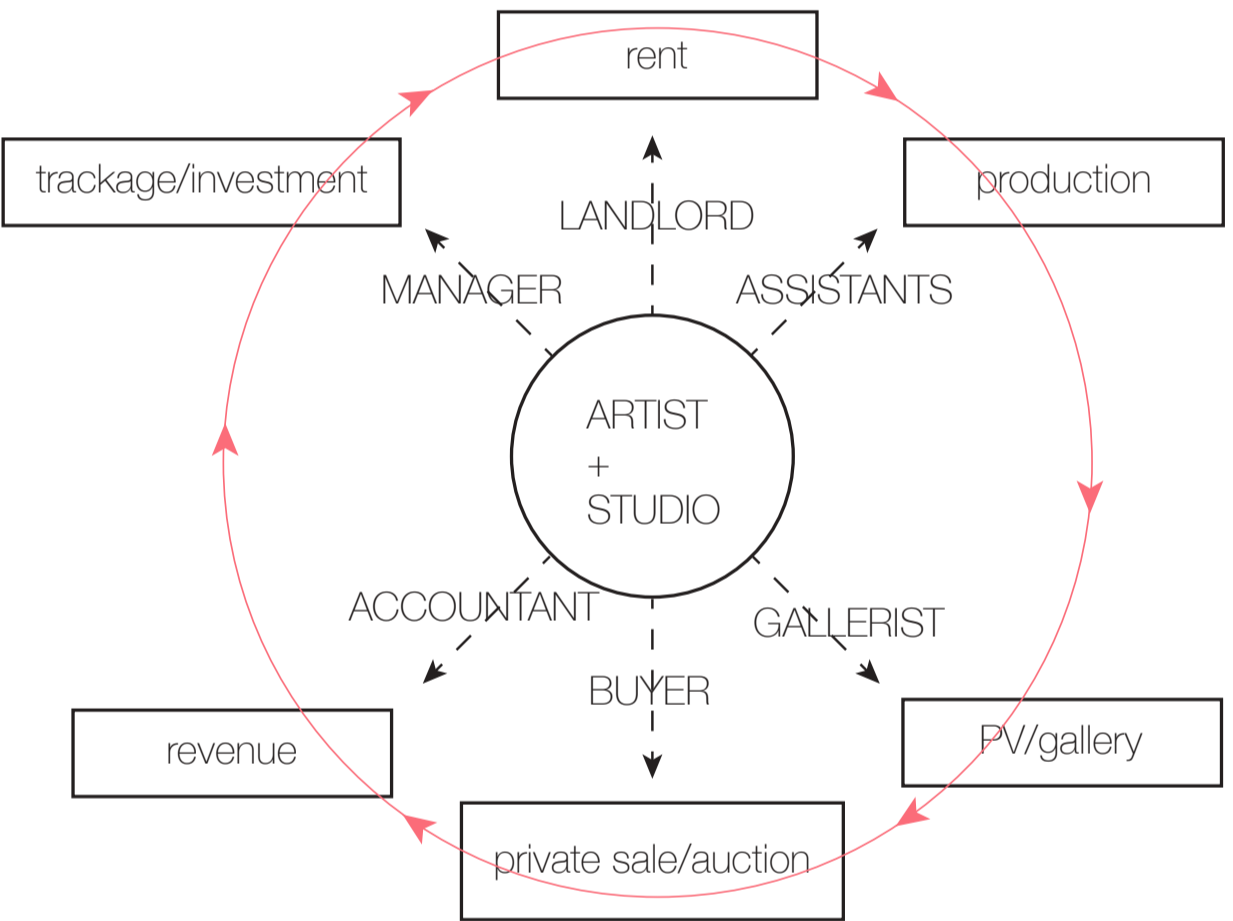
In a rapidly changing world becoming increasingly more volatile, bringing light onto the artist and the creative individual was something I hoped to emphasise. It is a message of reconciliation. The auction house serves its purpose in shedding light on the financial injustice revolving around artists, who are vulnerable to financial exploitation by dealers. These same people who, often in an auction, misuse their artworks for their own financial interests. In creating an auction house that is neither south nor north, yet both, I had the idea that a bridge, reminiscent of the medieval residential bridges that used to be prevalent in cities like London and Paris, with lingering traces still to be found in modern-day Bath, would bring about a symbolic message of unity. The auction house reflects a unity that aims to emphasise reconciliation between the traditionally richer north bank and the more industrial and poorer south bank. The floating bridge takes on the world with a new purpose, to reduce inequalities, unite people and bring back art to the people. The auction house is a revolutionary idea, in which an art auction is no longer sold off in exclusive art salons. These same saloons that are generally not so easily accessible to the public suddenly become the new hub for people to gather. Art can be created in private, but once finished its aim is to be displayed to the world, and not sold off and locked into a vault for no prying eyes to see. Art has the potential to change people and artists should display their thoughts on current affairs, or have people ponder over issues that artists think are prevalent. The auction house embodies this movement, in which the artist hands art over for the people, in which people of all backgrounds, symbolically represented in the geographic location of the Thames, neither north or south, are able to exchange ideas.

Position

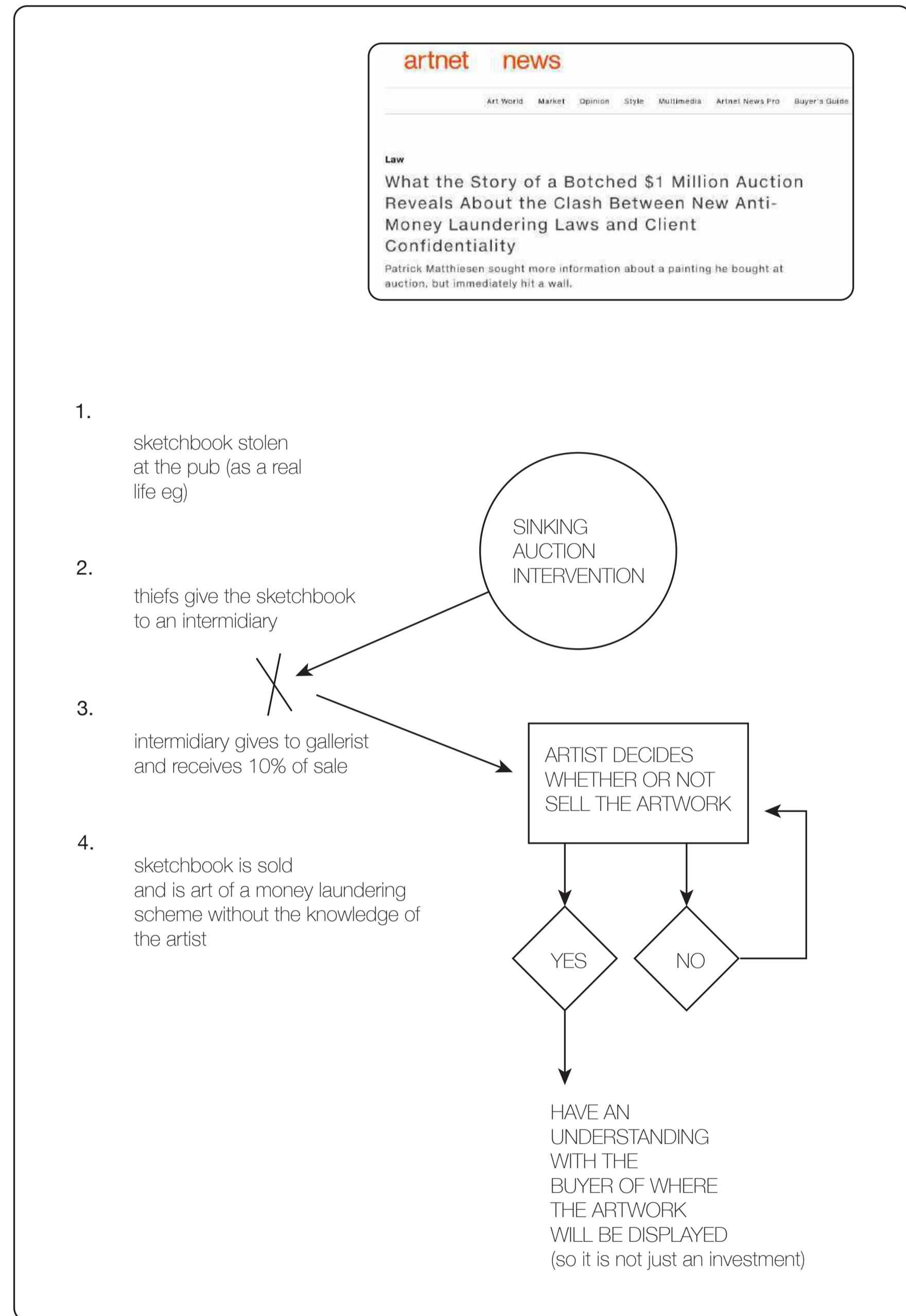
STANDARD AUCTION HOUSE:
Artist's struggle for being underpaid and left out of the global art market

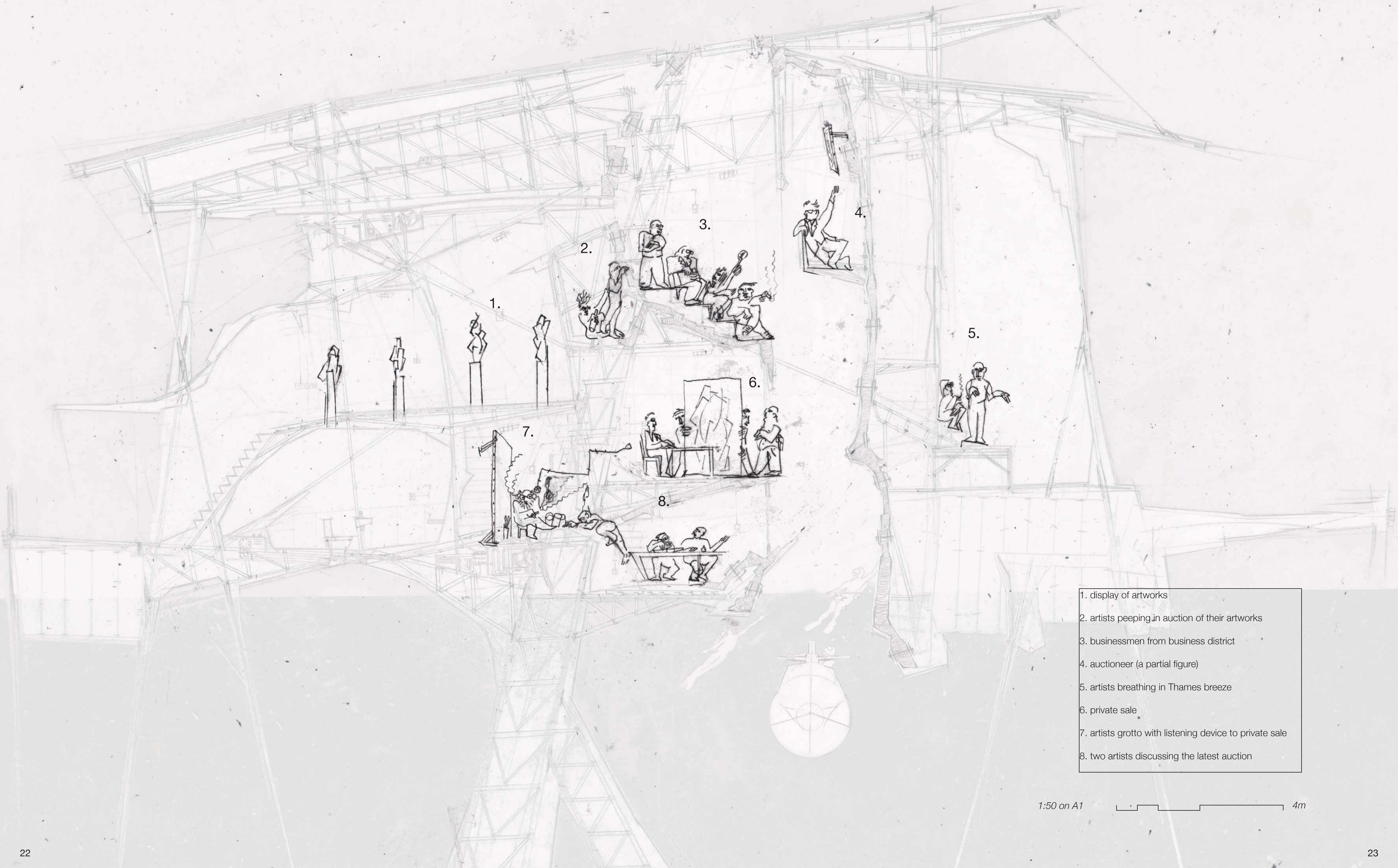
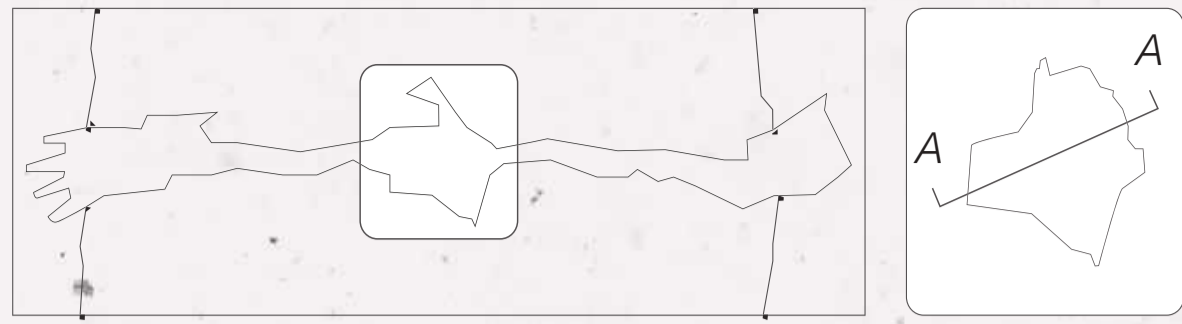


PROPOSED PROGRAMME:
Sinking Auction on the Thames



Hypothesis

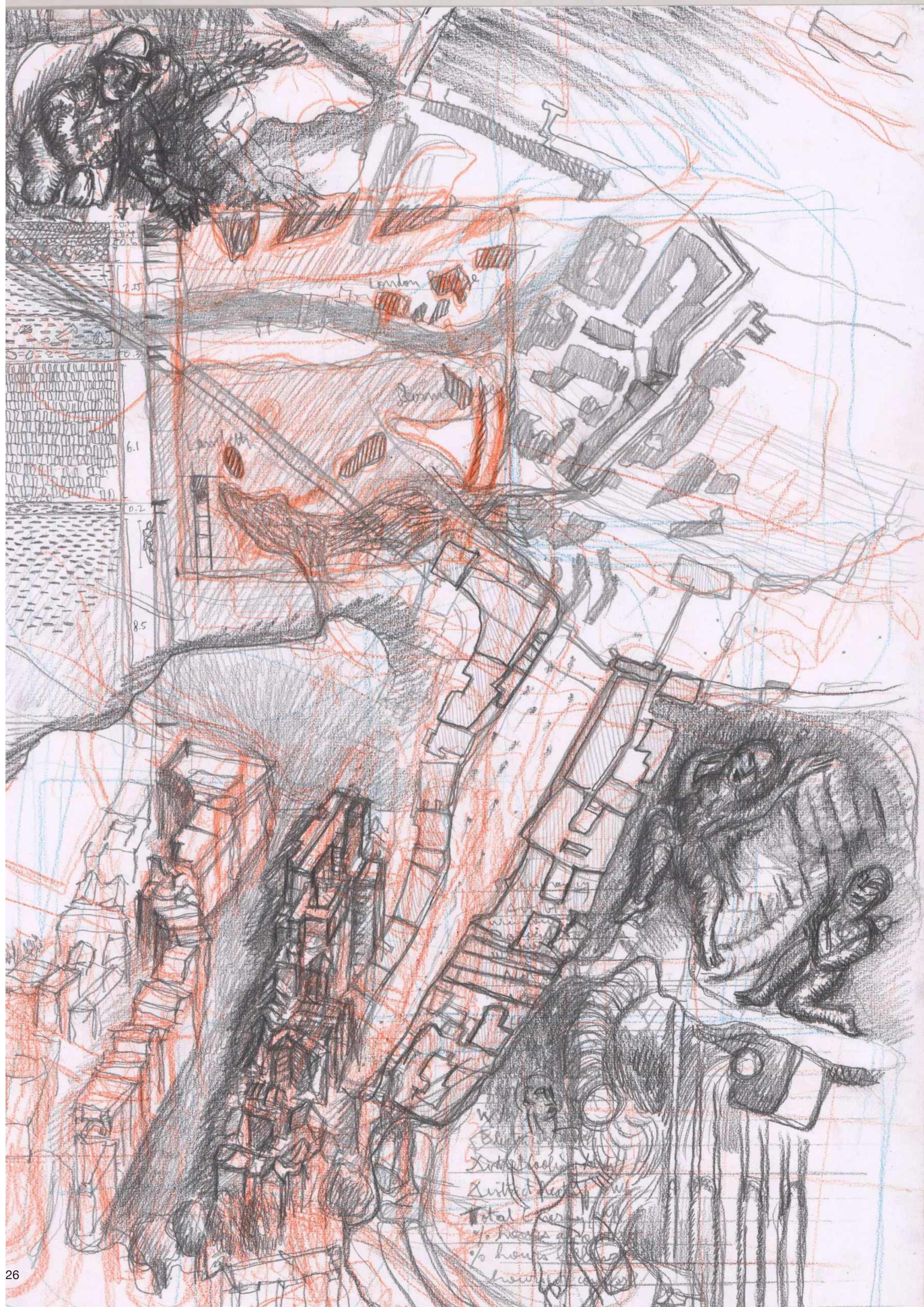




1. display of artworks
2. artists peeping in auction of their artworks
3. businessmen from business district
4. auctioneer (a partial figure)
5. artists breathing in Thames breeze
6. private sale
7. artists grotto with listening device to private sale
8. two artists discussing the latest auction

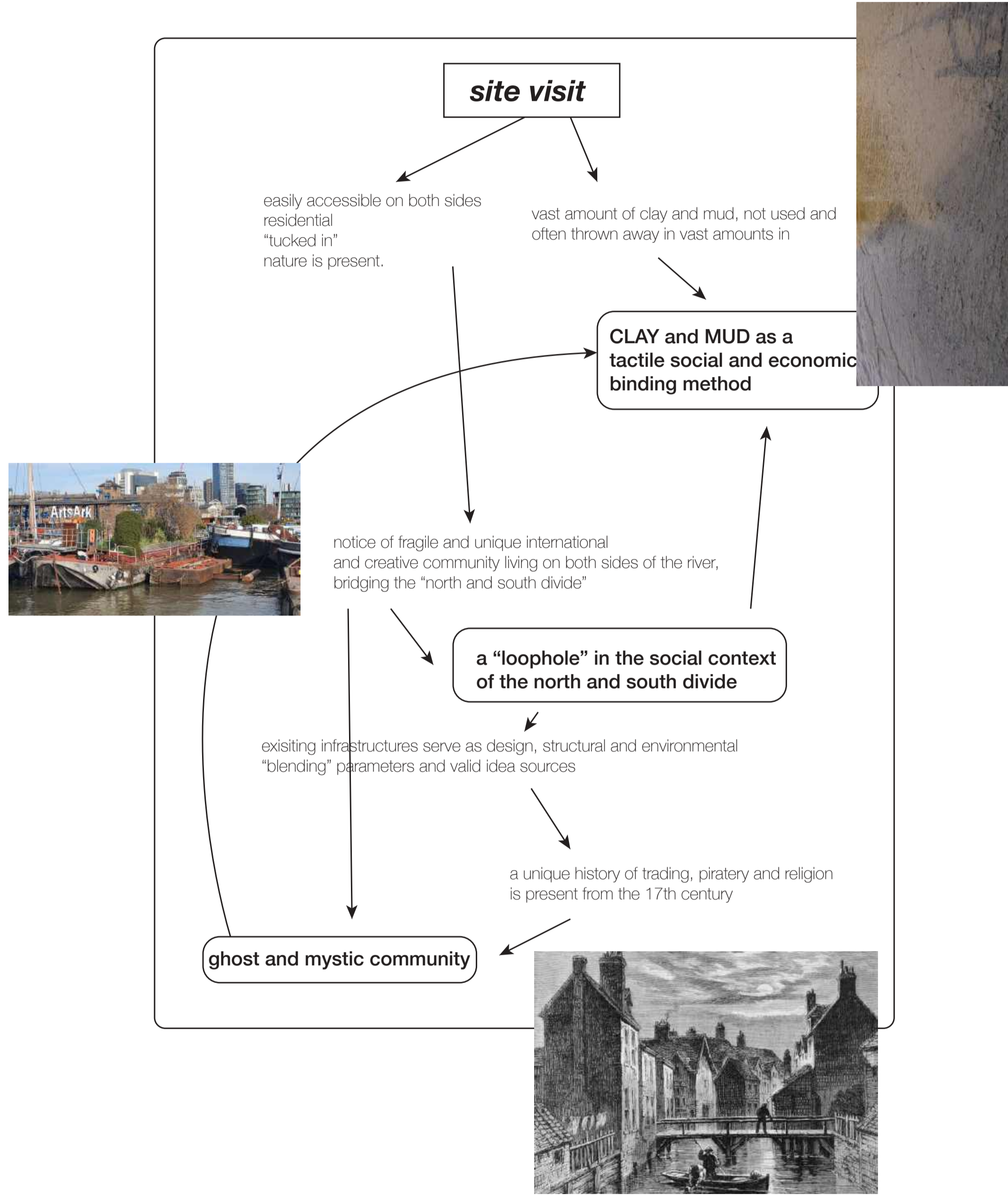
1:50 on A1 4m





Section 2. Pre-Design Research Site Analysis

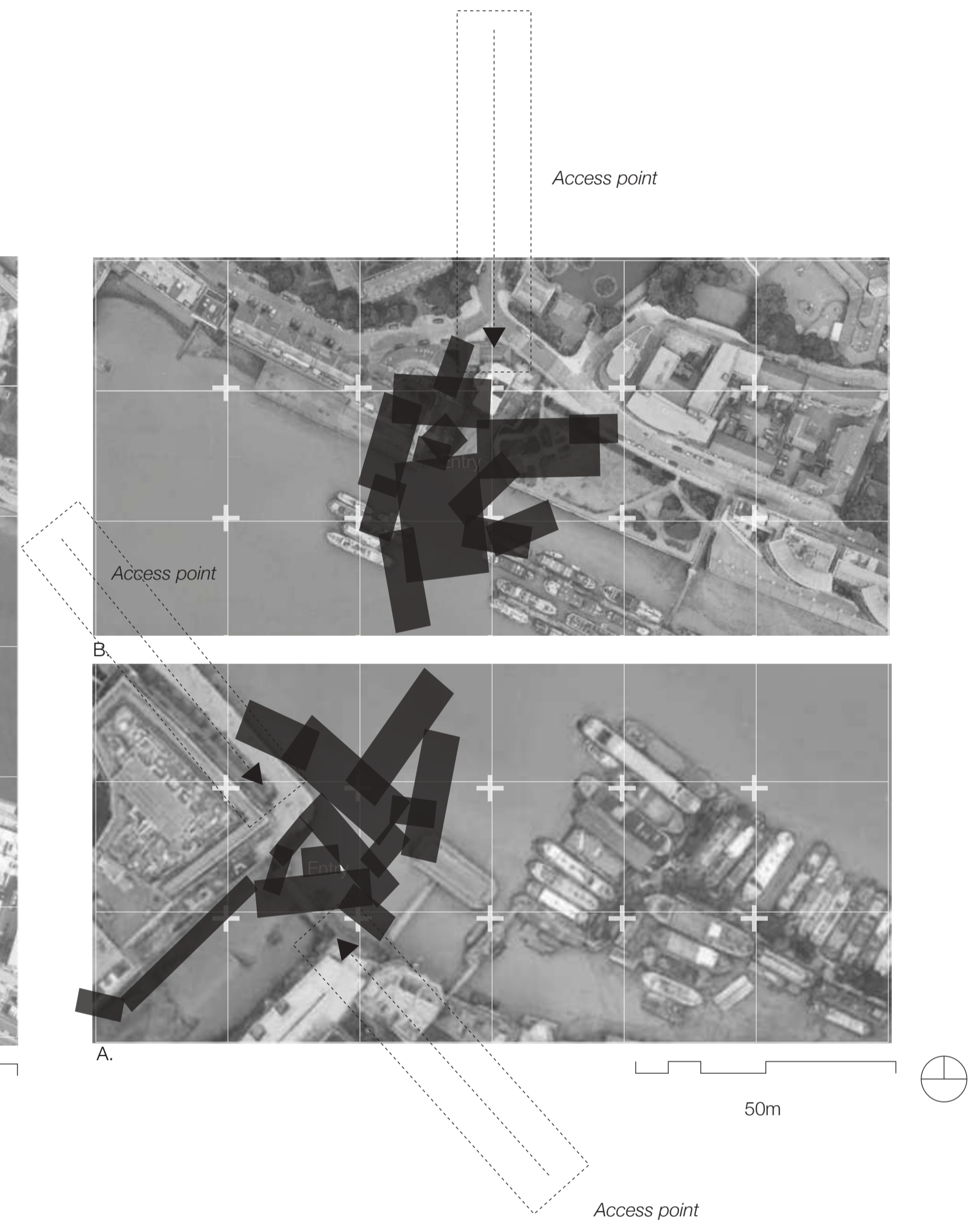
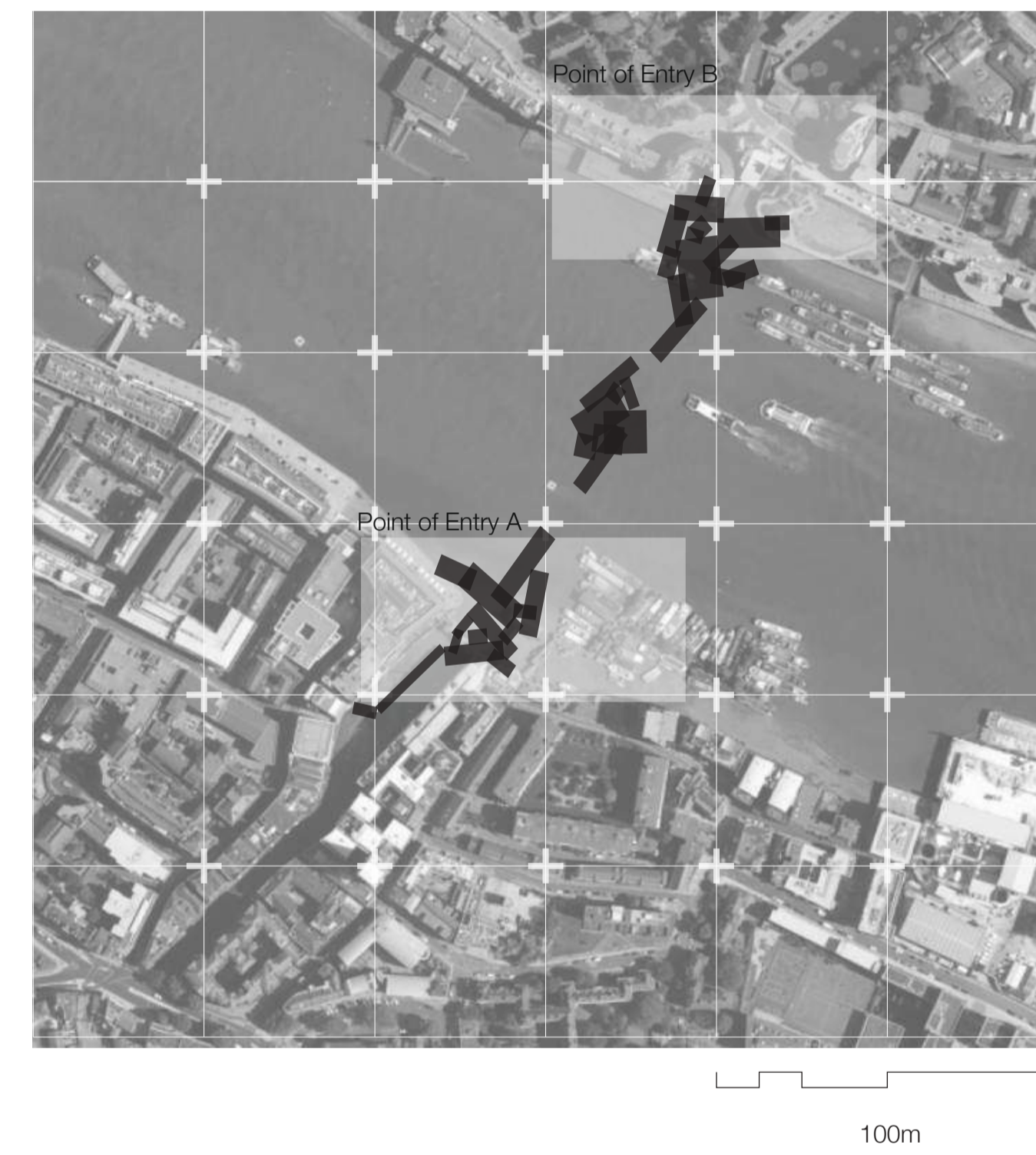
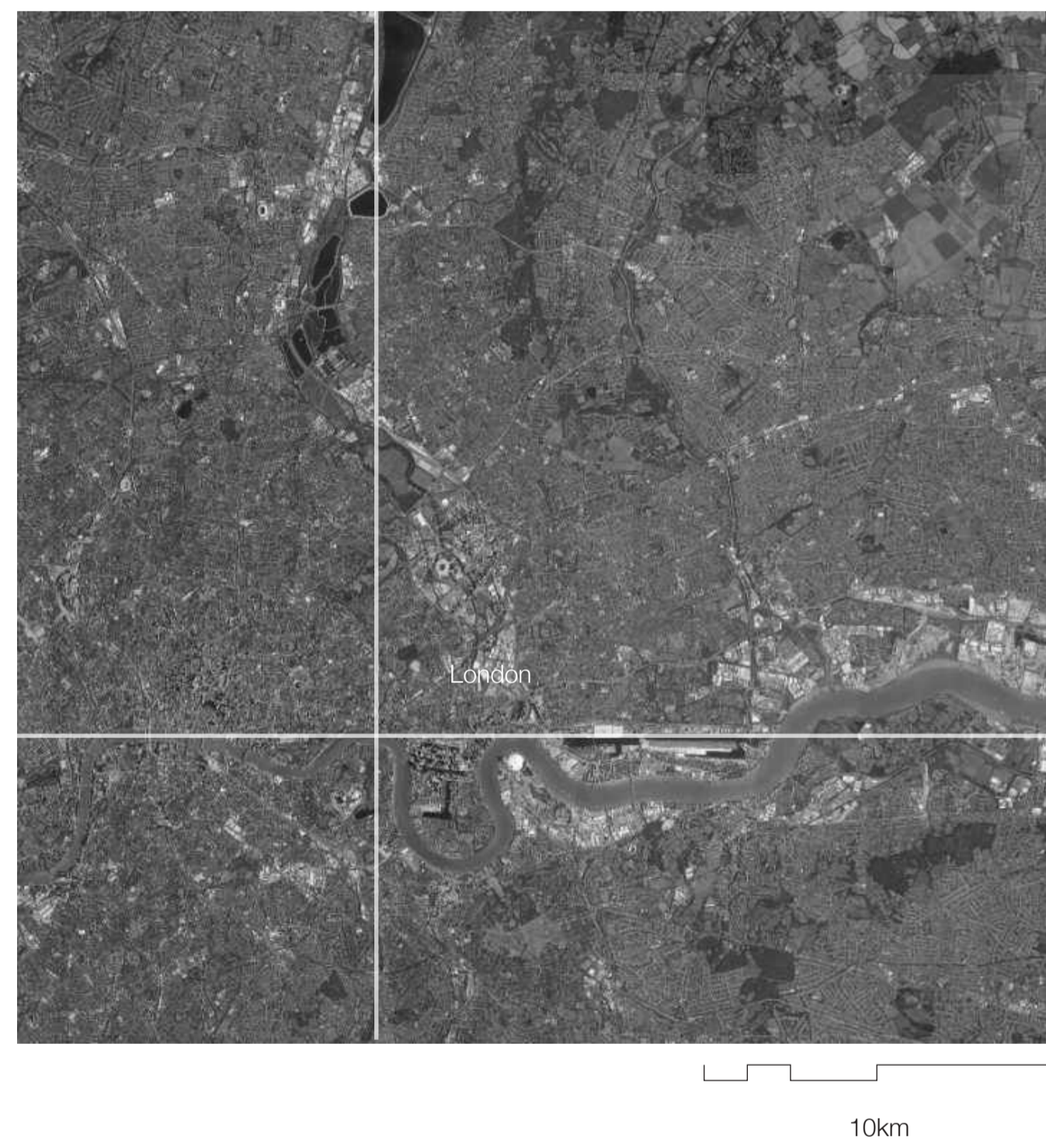
In order to make the auction house into a realistic and hypothetically feasible project to be created, it was important to research about the site in which the auction house would be situated. A floating bridge in the heart of London can only be created when it is qualified to be built if it conforms to local and national legislative laws. A bridge has to be safe for its people, something that is very important especially when innovative and almost experimental materials, such as clay, will be used. My preliminary steps for research was to look at the buildings surrounding the site, whether they are grade-listed and whether the bridge will pose any hindrances to its environment. The local natural environment was researched, particularly the Thames river. As a constantly flowing and changing aspect of nature, the Thames river can be both the subject of utility and of destruction. Therefore, researching about wind direction, waves, and local geological conditions (even just through visual means such as photographs) to the site made me understand what steps had to be made to ensure that this building would be both safe and feasible.



51.5055° N, 0.0754° W



Macro: 1:2000, Meso: 1:200, Micro: 1:20





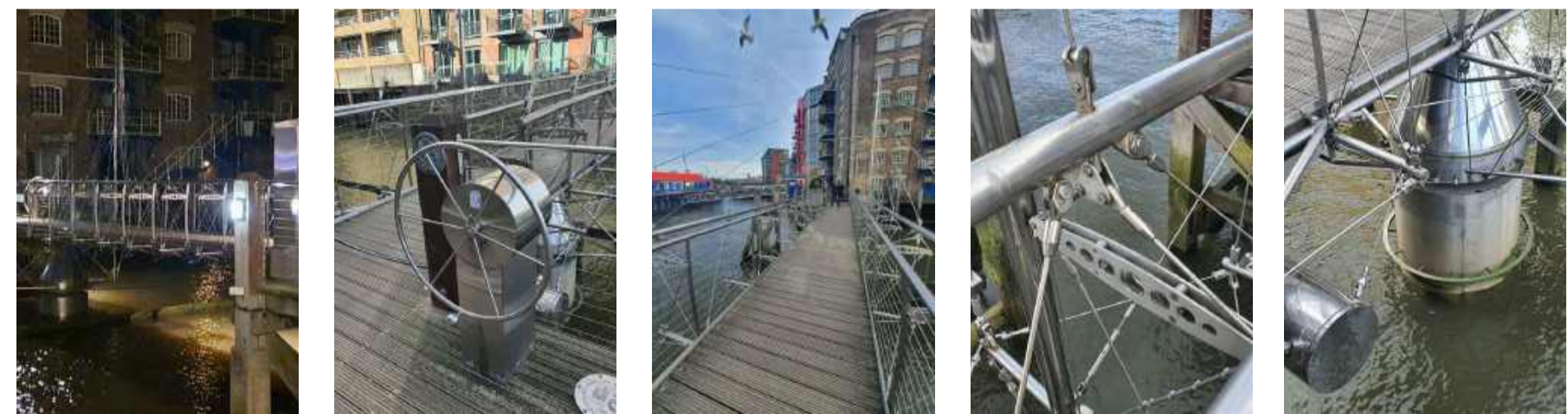
Social, Programmatic and Typological Context



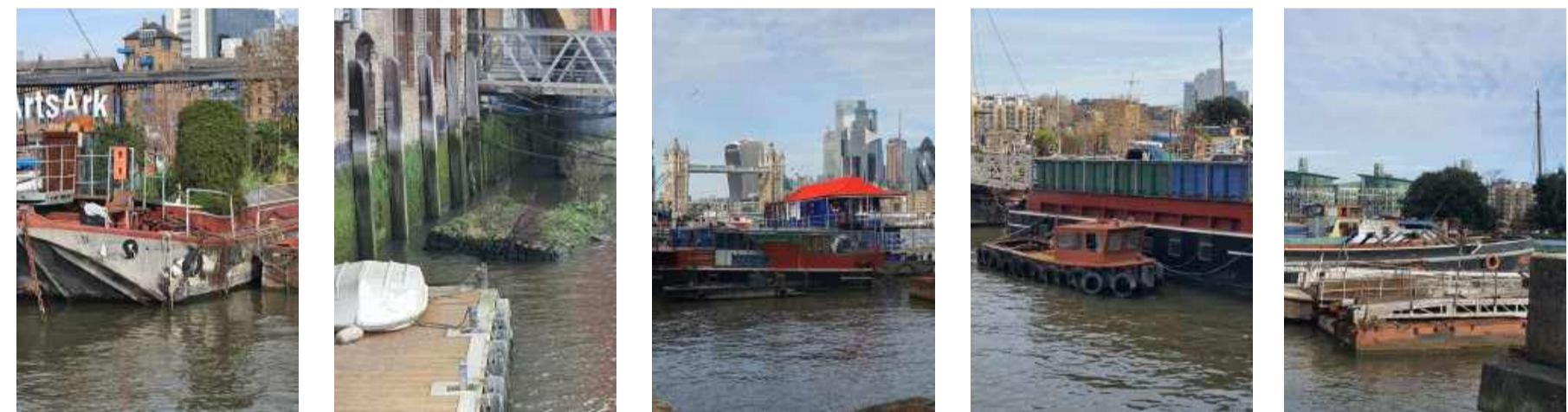
1. Mud



2. Piers



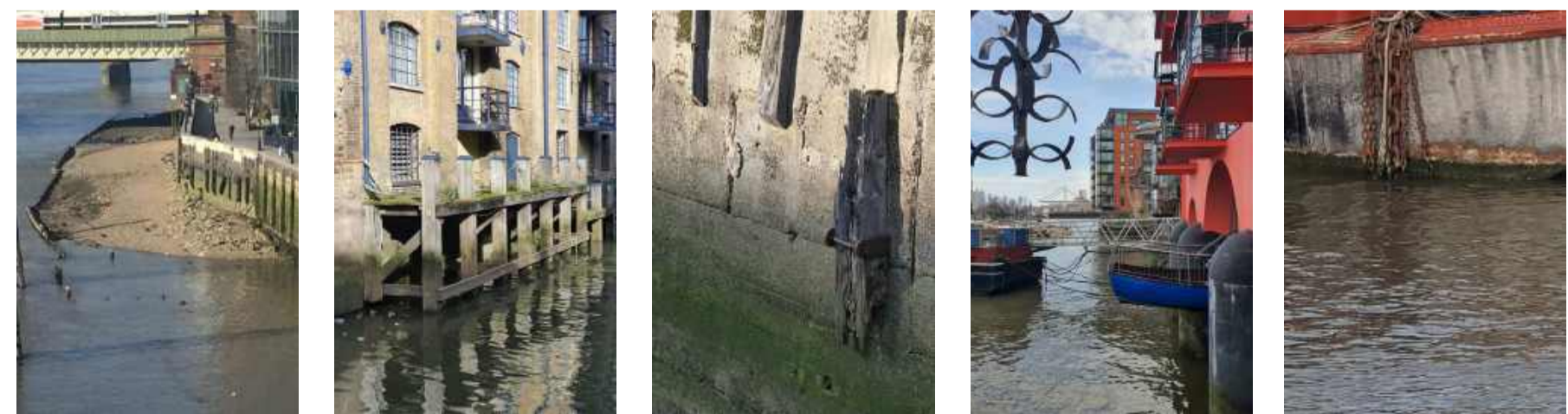
3. Existing structures



4. Living boats



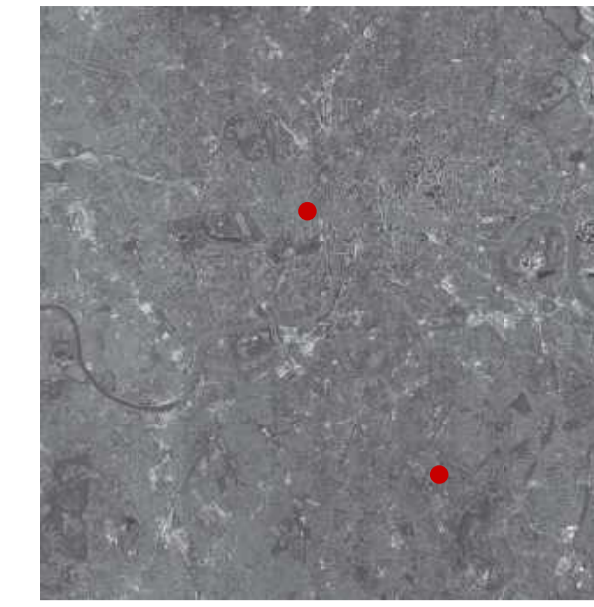
5. Cranes



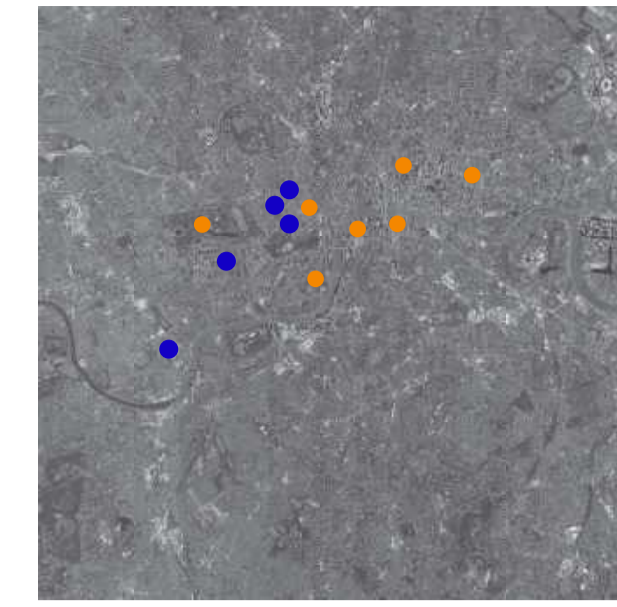
6. Tides



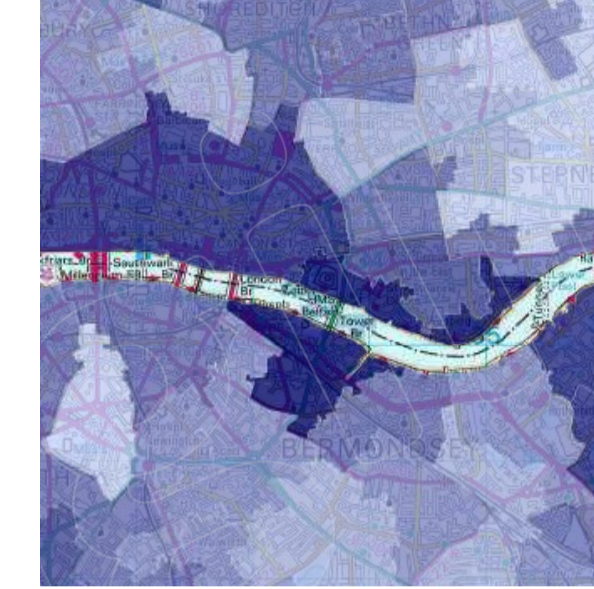
Overlaid base map 1:100



Notorious Art thefts London 1:50 000



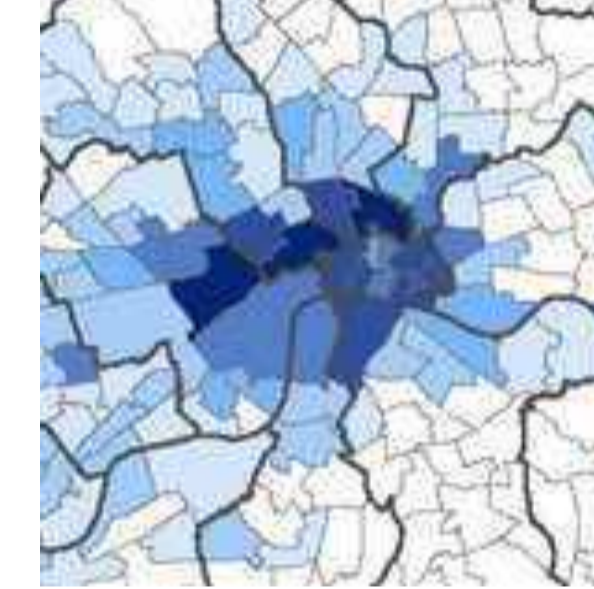
Auction houses, galleries nearby 1:50 000



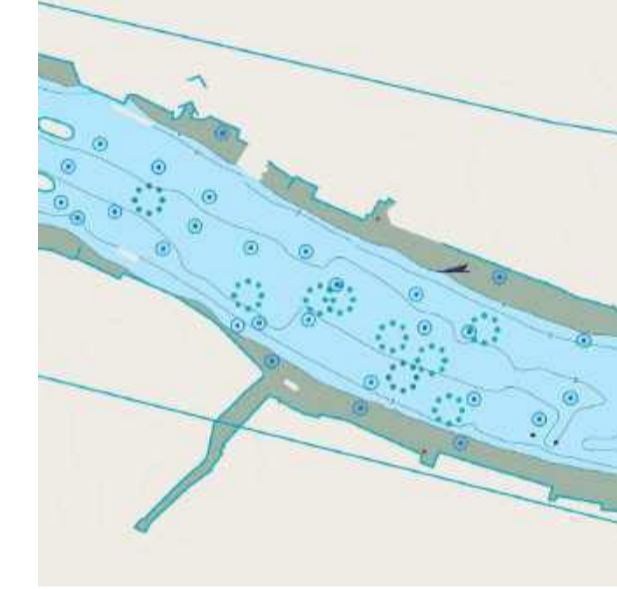
Employment density 1:10 000



Population density 1:10 000



Creative industries 1:10 000



Marine features 1:100



QUINCE trees - Tower Bridge Floating Gardens



Herbs eg. BORAGE - tower bridge community gardens and also native weed



native wild plants - NETTLES / calendula nasturtium



algae: cyanobacteria blooms



common eelgrass

7. Nature



snails



sole fish



moorhens / ducks



kingfisher



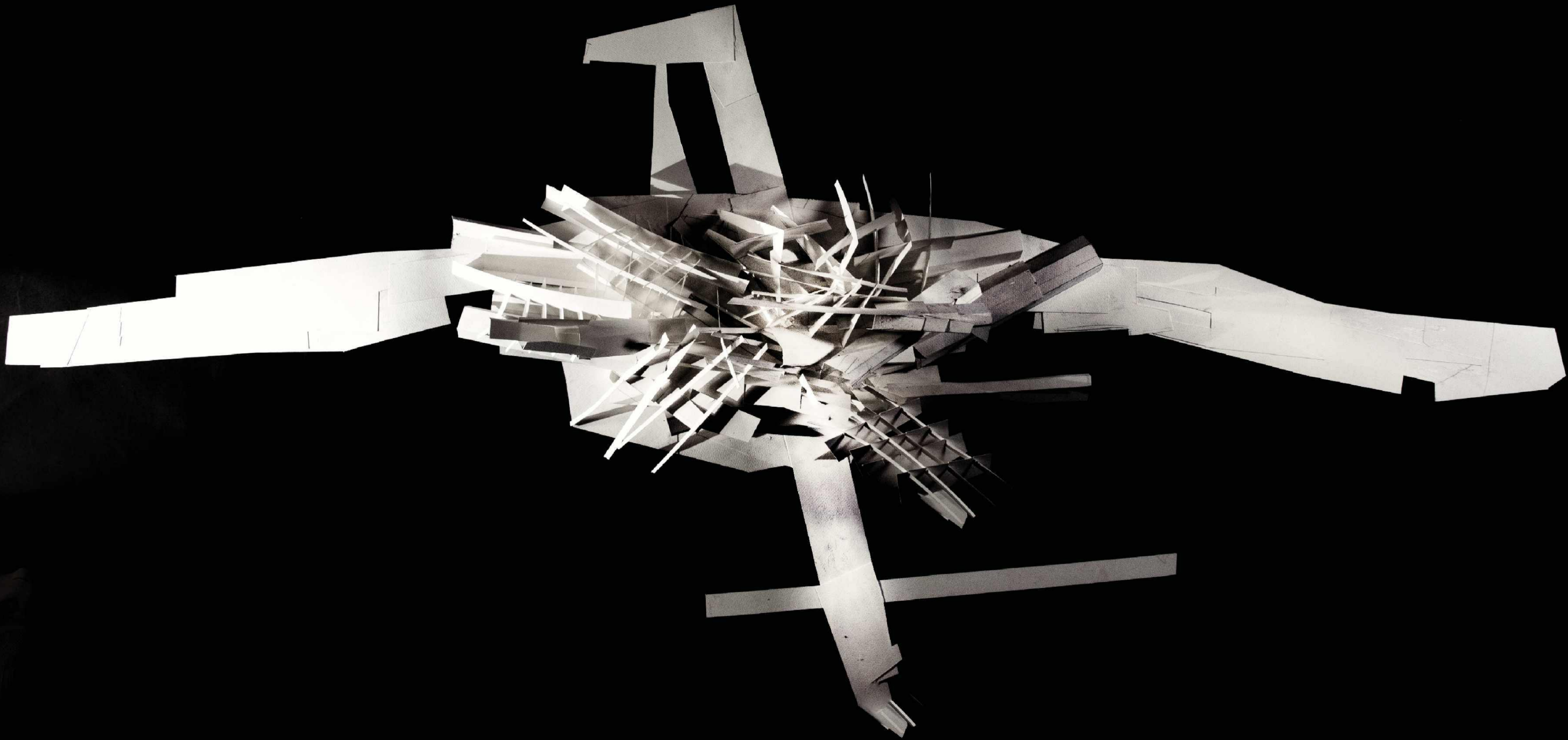
fox

8. Animals



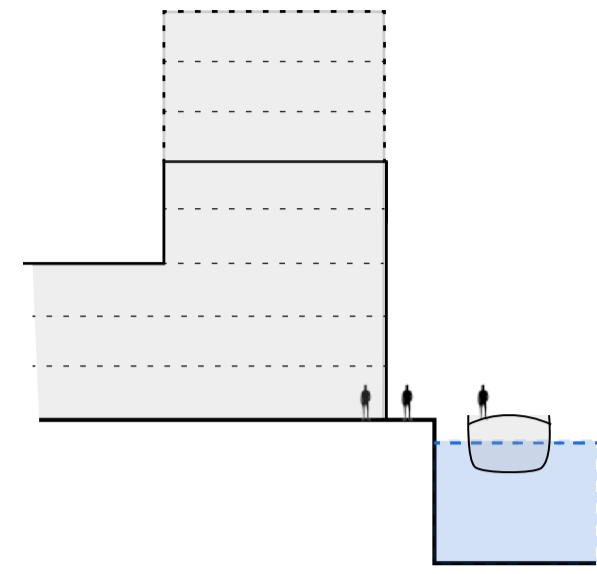
9. Buildings







Practical, Methodological and Legislative Constraints



The dockland elevations are usually a minimum of 3 storeys, with the water-facing side 5-8 storeys. Generally buildings have "blocky" masses facades are flat.

- Legend
- listed buildings
- fire station
- street art

Typical dockland elevations



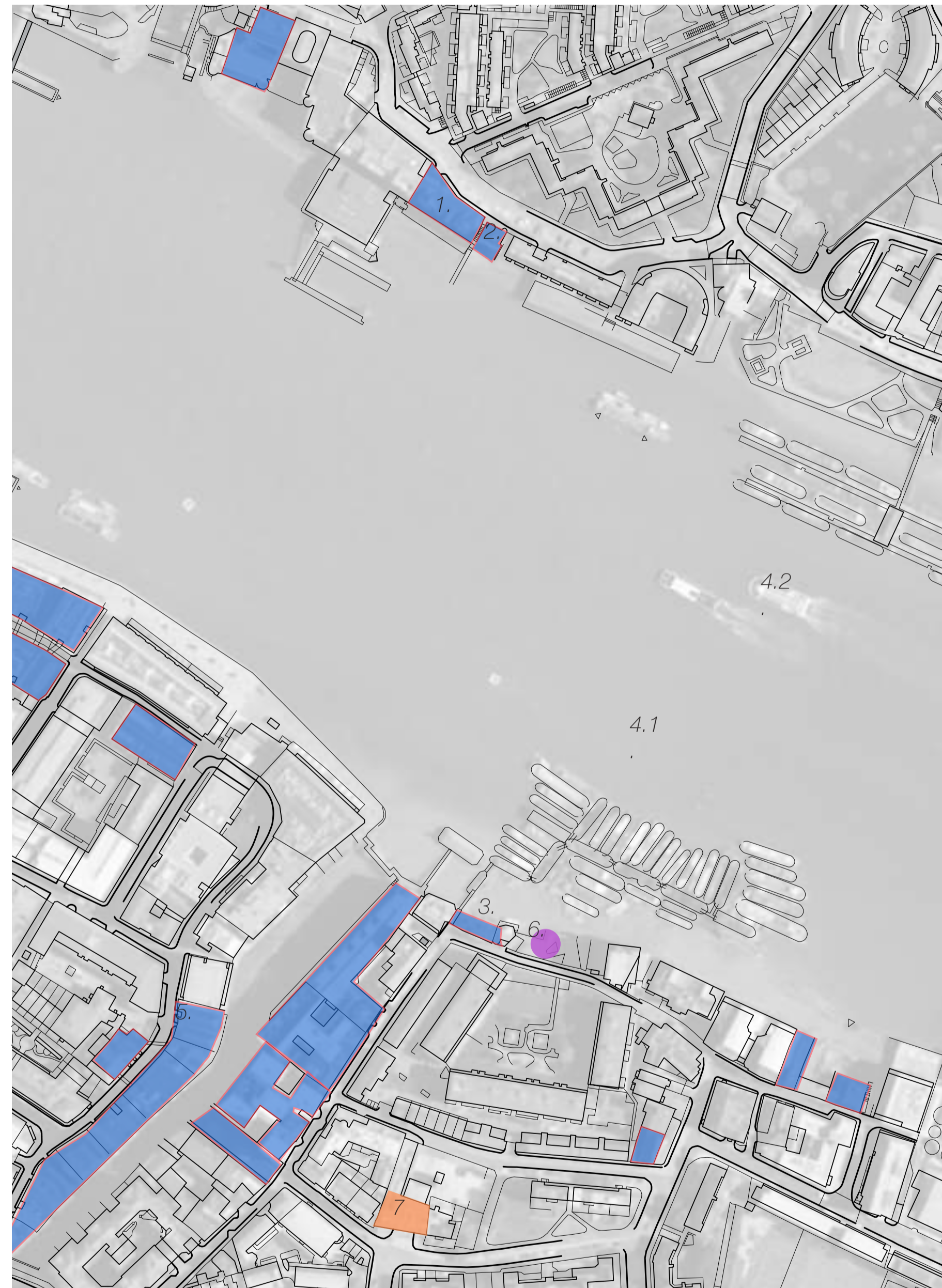
1. British and Foreign Wharves



2. Alderman Stairs and Gate Piers



3. China Wharf



Legislations map 1:100



5. Java Wharf



6. Banksy's Boy Fishing



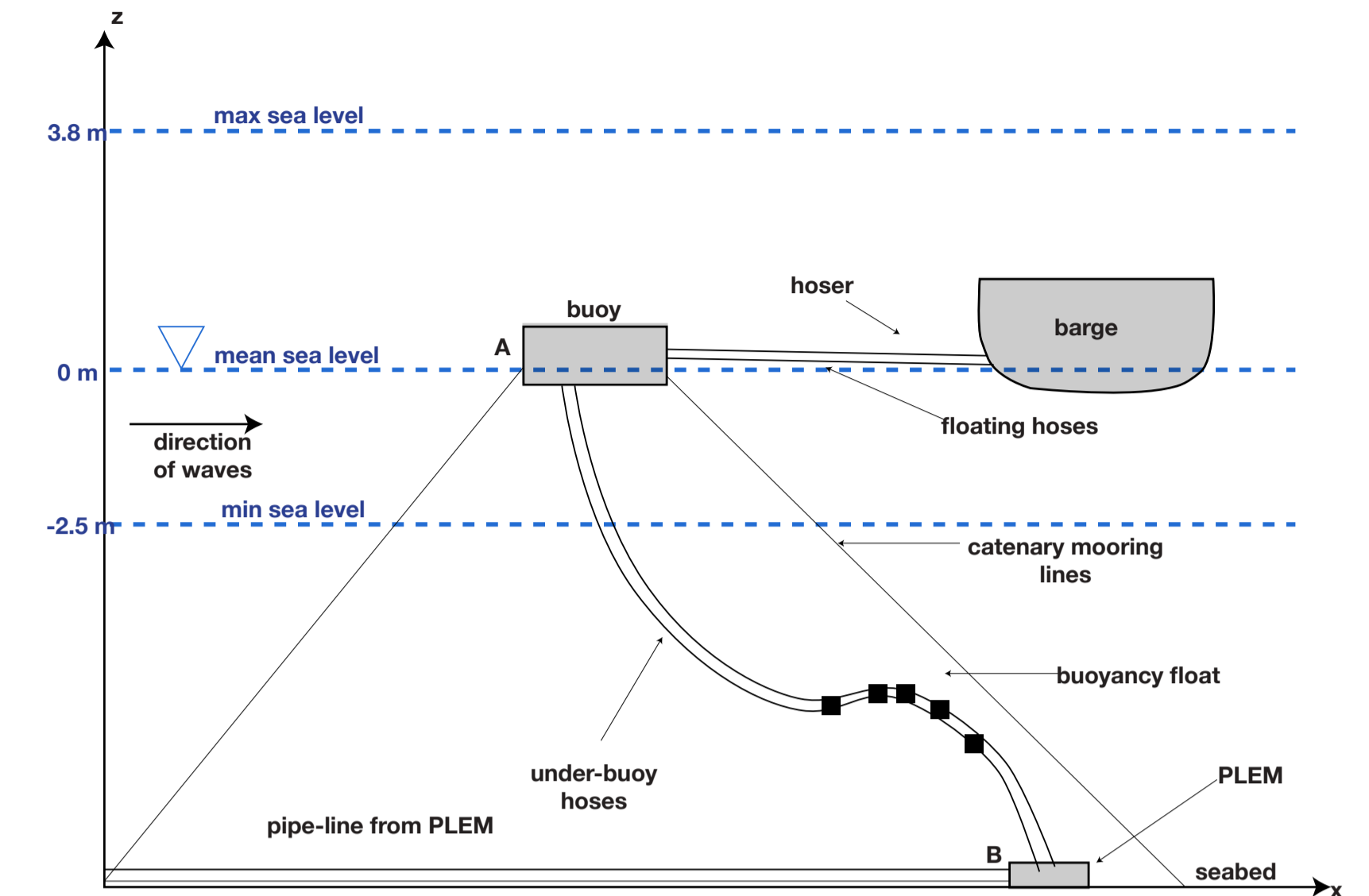
7. Dockhead Fire station



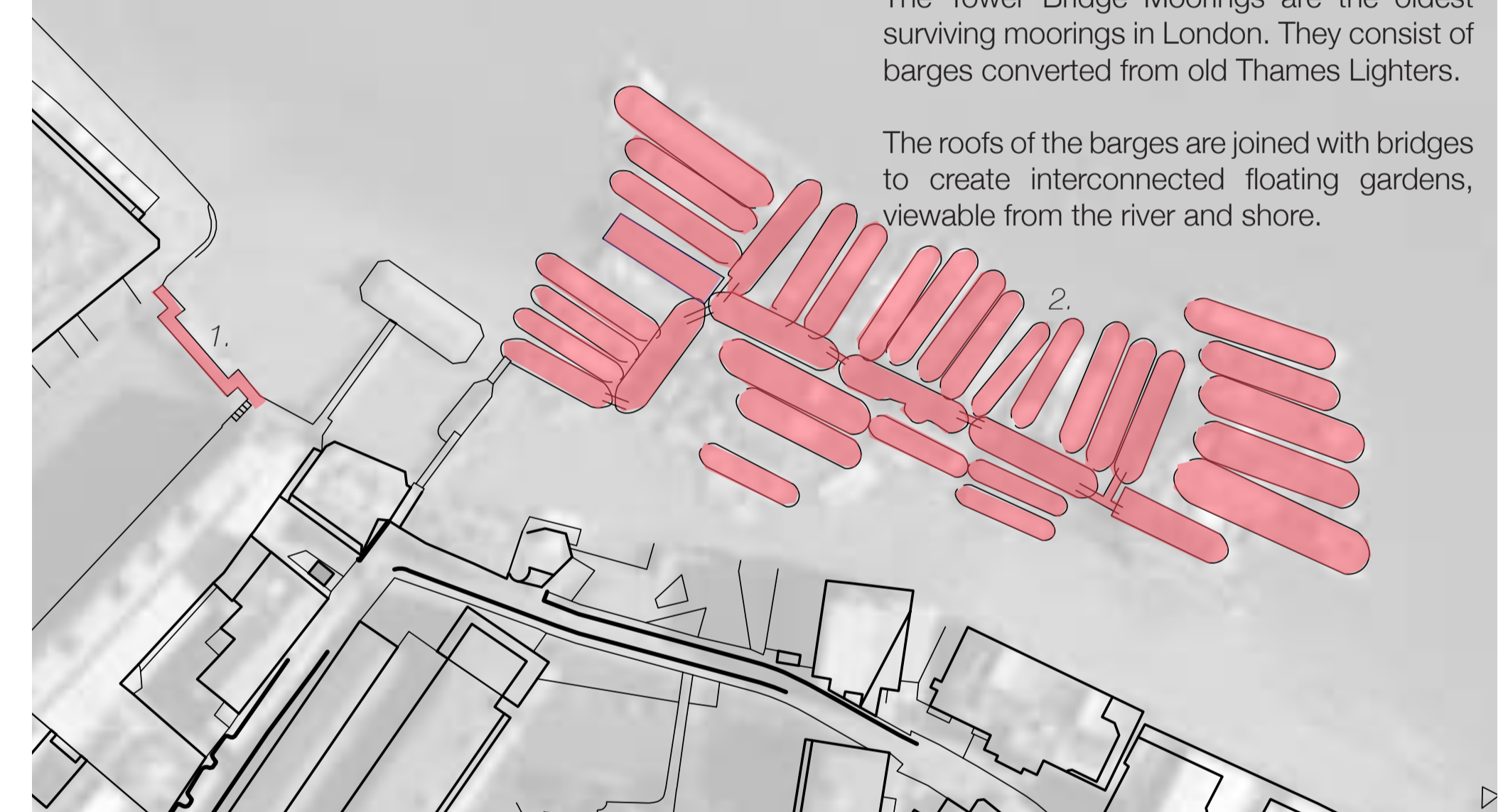
1. Saint Saviour's dock footbridge

In 1995, an foodbridge was constructed connecting the two sides of Saint Saviour's dock. However, due to several mechanical, structural and operational issues, the bridge had to be fixed in the closed position. This blocked the passage of large boats in the dock.

Now, the bridge is undergoing a refurbishment by CONWAY AECOM, in order to reopen the passage of boats, renew the sailboat rigging and repair the hydraulic swing system.



2. Tower Bridge Moorings & Floating Gardens



The Tower Bridge Moorings are the oldest surviving moorings in London. They consist of barges converted from old Thames Lighters.

The roofs of the barges are joined with bridges to create interconnected floating gardens, viewable from the river and shore.



2. Tower Bridge Moorings & Floating Gardens

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Floating Gardens

During the mid 1990s, one of the old lighters had started to develop its own ecology: a duck's nest was growing in some silt at the bottom of the barge. This inspired the beginning of the floating gardens.

In 2003, the Southwark Council threatened to evict the barges for having no planning permission but the case was appealed and won with a large number of the public arguing for the "valuable contribution to views down river" that they provided.



The Moorings used to be home to a communal barge event space called 'ArtsArk' which held many events such as film screenings, poetry evenings and bonfire parties.

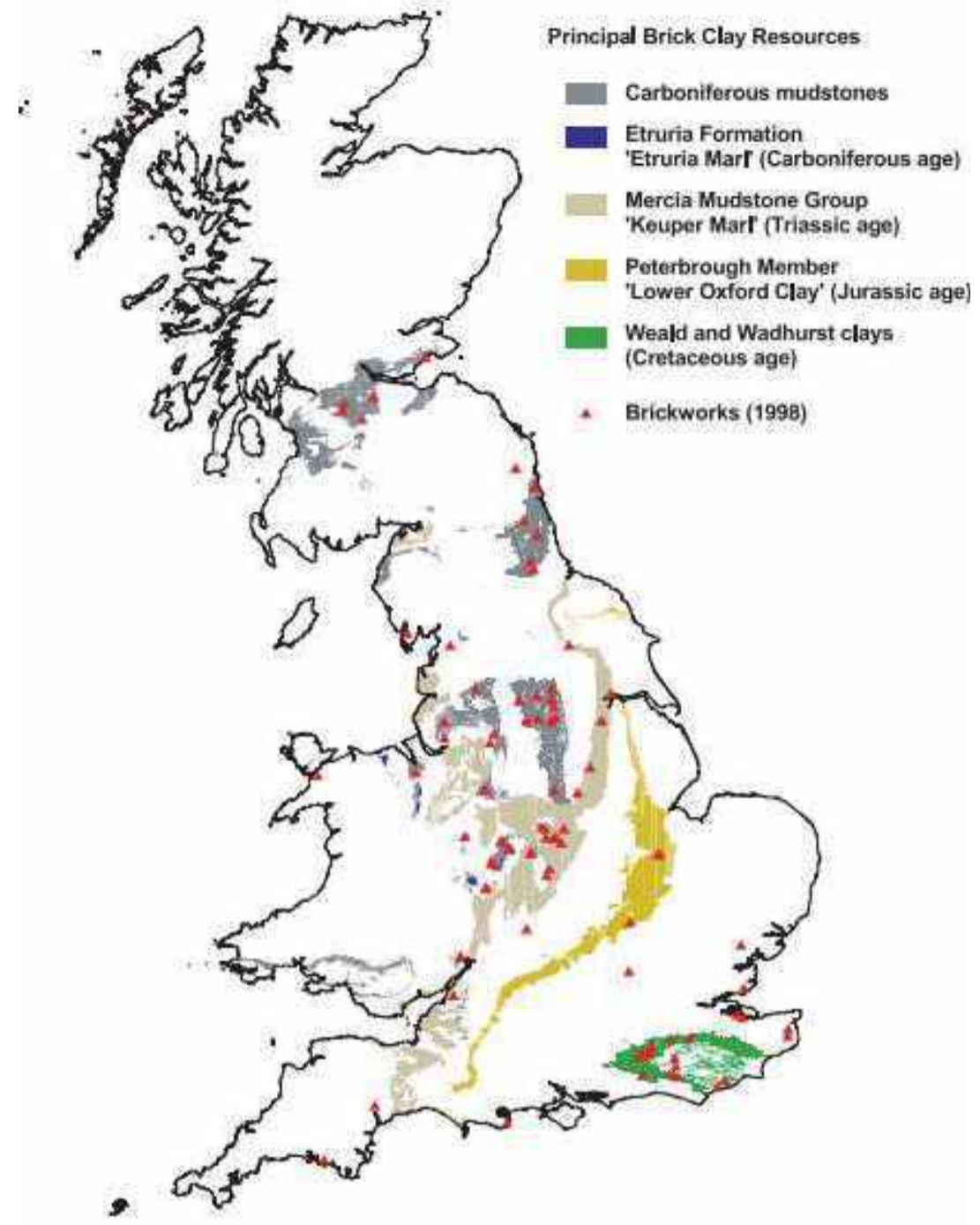
Today, ArtsArk has dissolved but the barge is still used for communal events.



4.1. Tower Bridge Moorings
4.2. Hermitage Community Moorings



Topographic, Natural and Geological Conditions

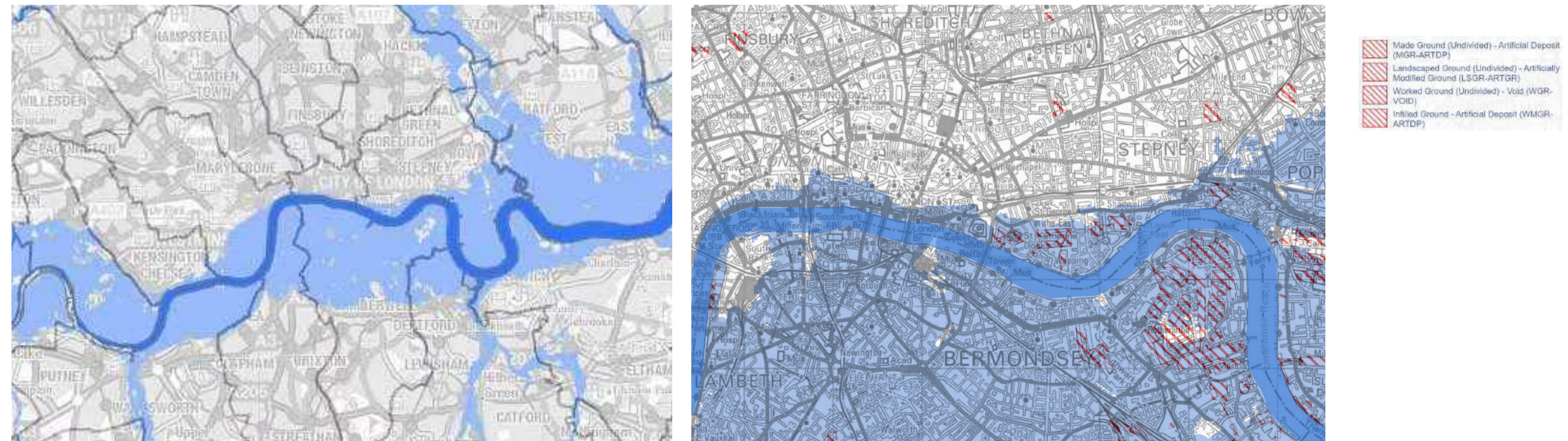


Bedrock map 1 1:375 000

Principal brick clay resources



Bedrock and superficial deposits 1 1:20 000



Sea level rise 1:200 000

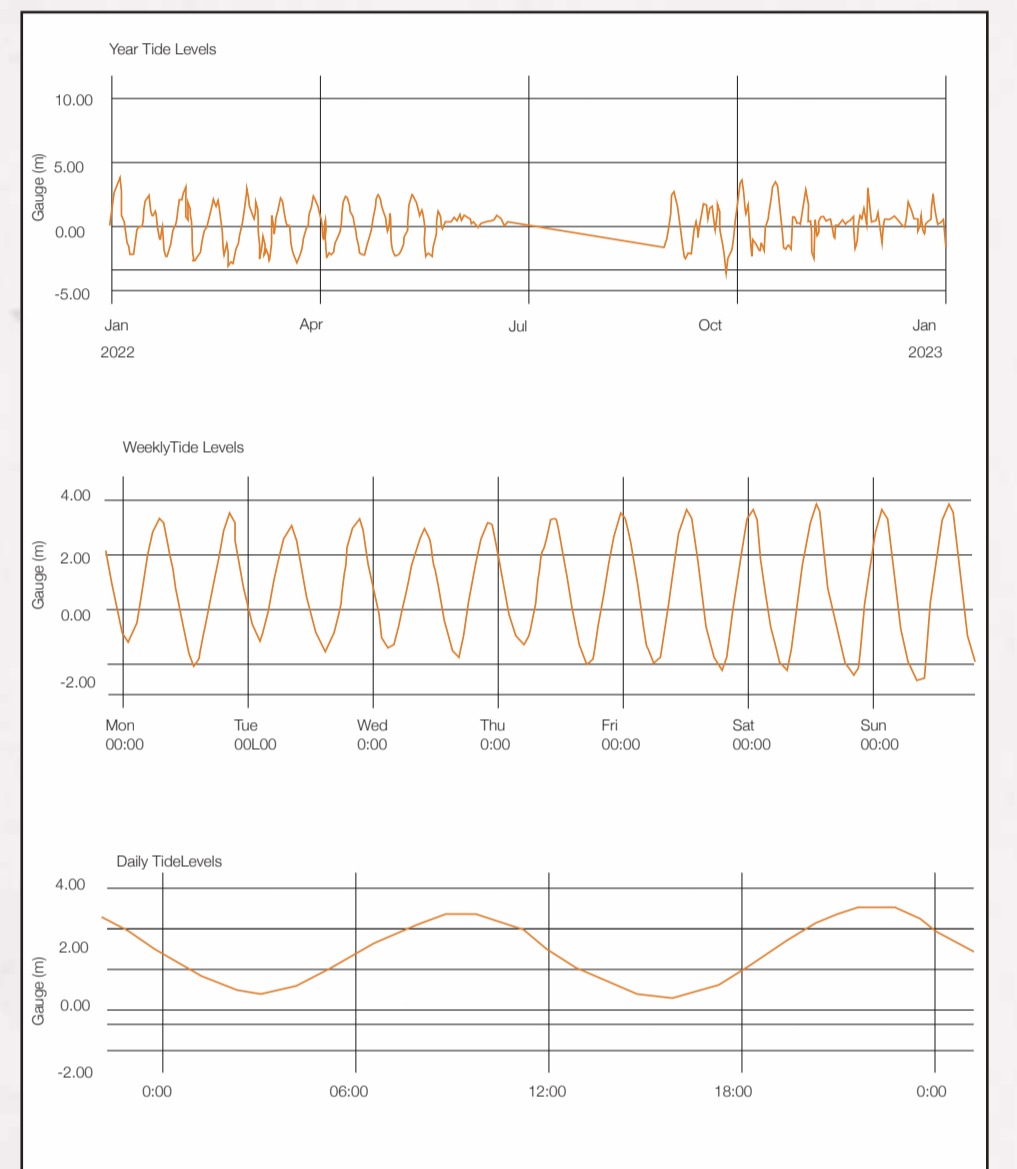
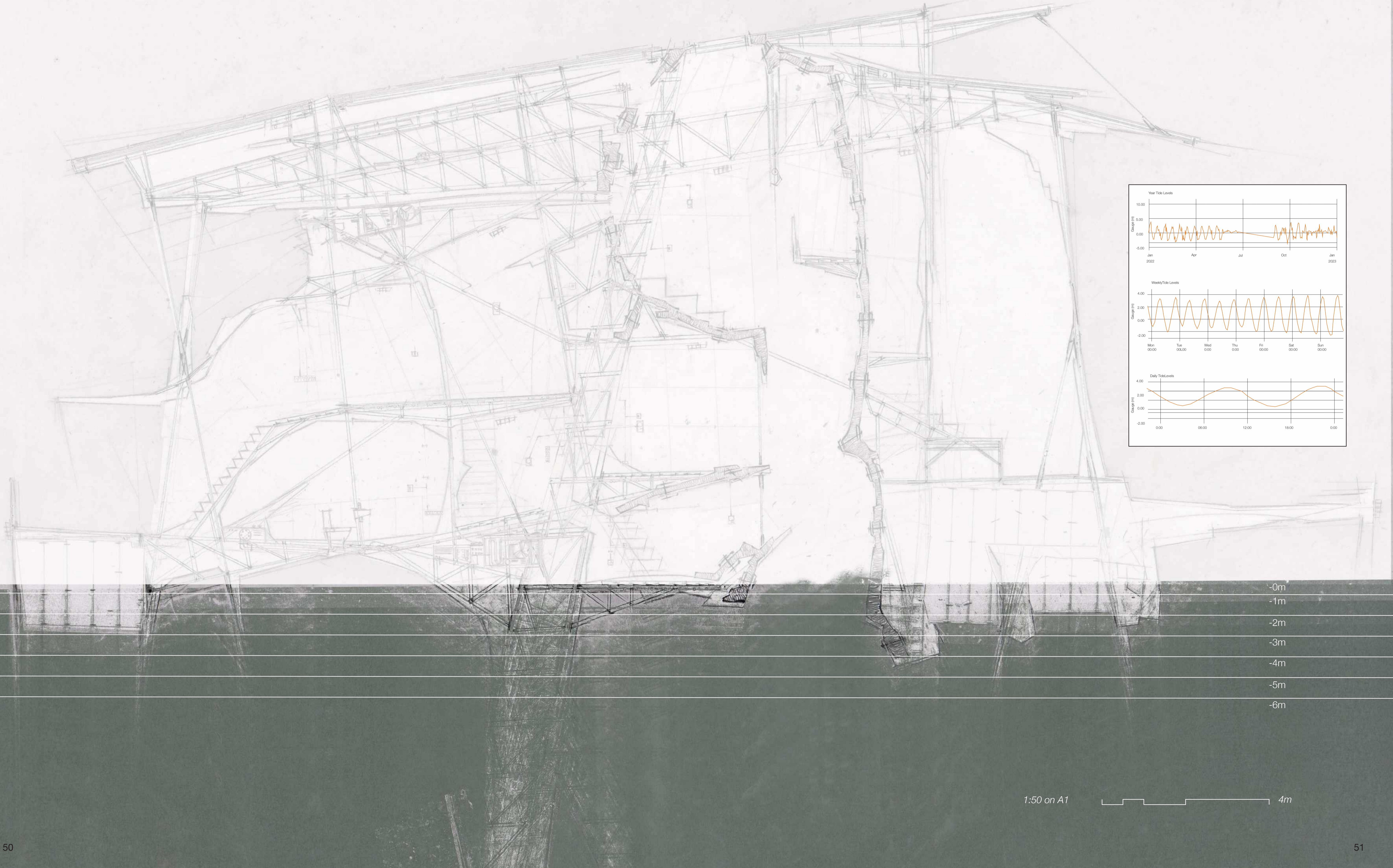
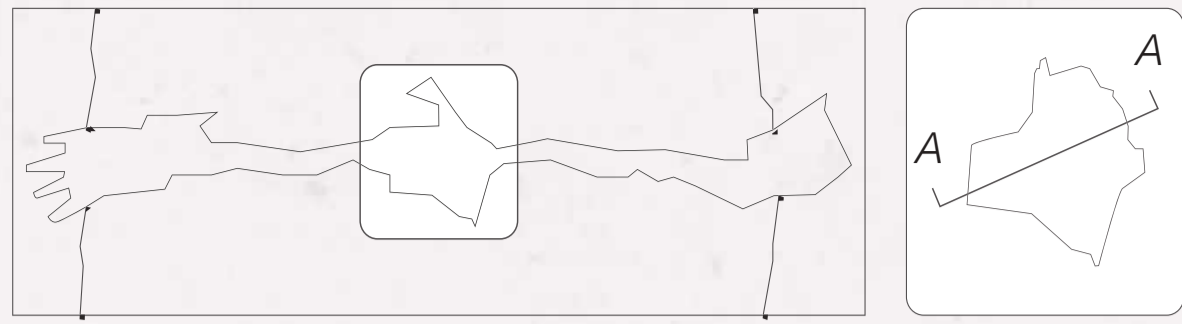
Artificial ground and sea level rise in 50 years 1:40 000

Contract Name		ST. THOMAS STREET		Borehole No. 1		Sheet 1 of 2	
Method of boring		Shell and Auger		Ground level 3.99 m OD			
Diameter		200 mm		Start 9.12.80		Finish 10.12.80	
Daily progress	Water levels	In-situ tests	Samples	Depth (m)	Reduced level (m O.D.)	Thickness (m)	Description of Strata
				0.10	3.89	0.10	Tarmac
				0.40	3.59	0.30	Concrete
				1.00	2.99	0.60	Bricks, gravel and rubble
9/12			U				Made
							Ground
						2.25	Firm brown silty clay with occasional shells and traces of carbonaceous matter
			U				British Geological Survey
				3.25	0.74	1.35	Firm grey silty clay with occasional shells and traces of decaying organic matter
11/12			J	4.60	-0.61	0.30	Soft grey silty sandy clay with pockets of decaying organic matter
			N = 25 B	4.90	-0.91		British Geological Survey
			N = 13 B			6.10	Medium dense coarse to fine subangular gravel with a little sand
10/12			N = 19 WB				British Geological Survey
			N = 4 B				British Geological Survey
Notes Surface tarmac and concrete broken out using compressor - 1 1/2 hours. Water level observation tubing installed to 11.00 m.							
Terresearch Limited		Report No. S.30/747		Appendix 1 Sheet 1			

Contract Name		ST. THOMAS STREET		Borehole No. 1		Sheet 2 of 2	
Method of boring		Shell and Auger		Ground level 3.99 m OD			
Diameter		200 mm		Start 9.12.80		Finish 10.12.80	
Daily progress	Water levels	In-situ tests	Samples	Depth (m)	Reduced level (m O.D.)	Thickness (m)	Description of Strata
10/12			J	11.00	-7.01	0.20	Stiff brown fissured silty clay with occasional small pockets and partings of brown fine sand and silt
			U	11.20	-7.21		British Geological Survey
						8.80	Stiff grey-brown fissured silty clay with occasional small pockets and partings of grey fine sand and silt claystone at some levels (London Clay)
			U				British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
							British Geological Survey
10/12			J	20.00	-16.01		Bottom of Borehole
Notes Bottom of Borehole							
Terresearch Limited		Report No. S.30/747		Appendix 1 Sheet 2			

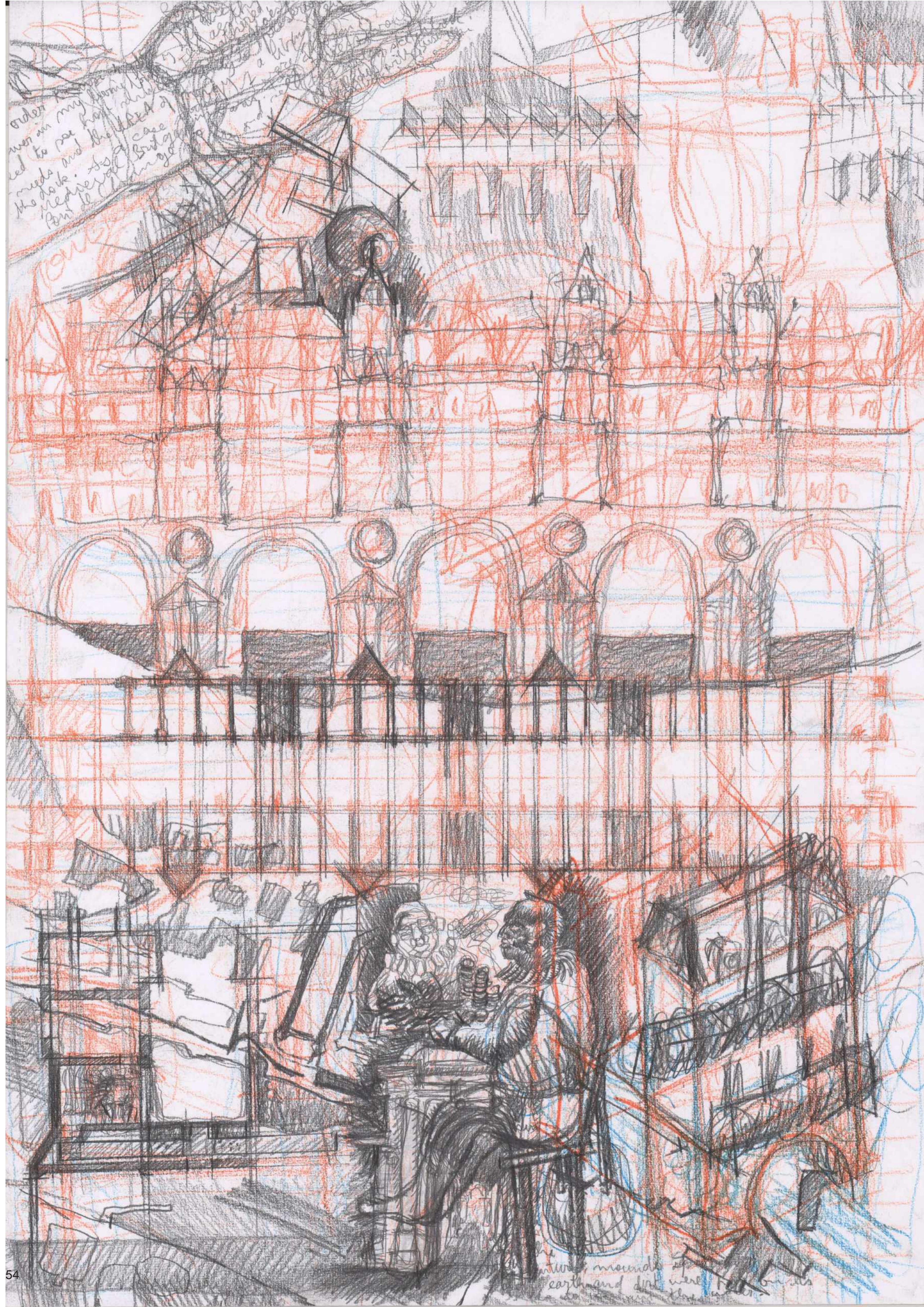
Borehole on site





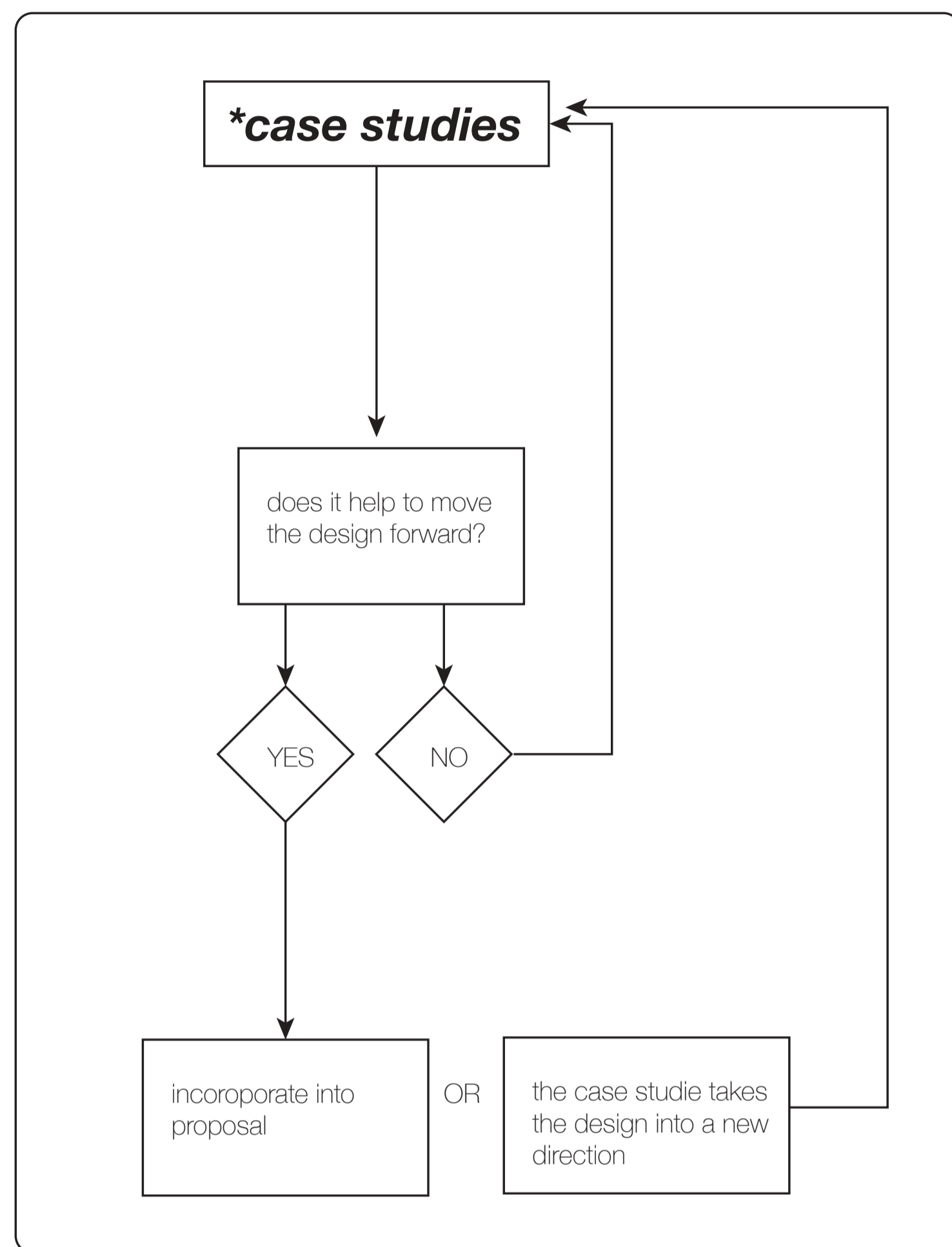
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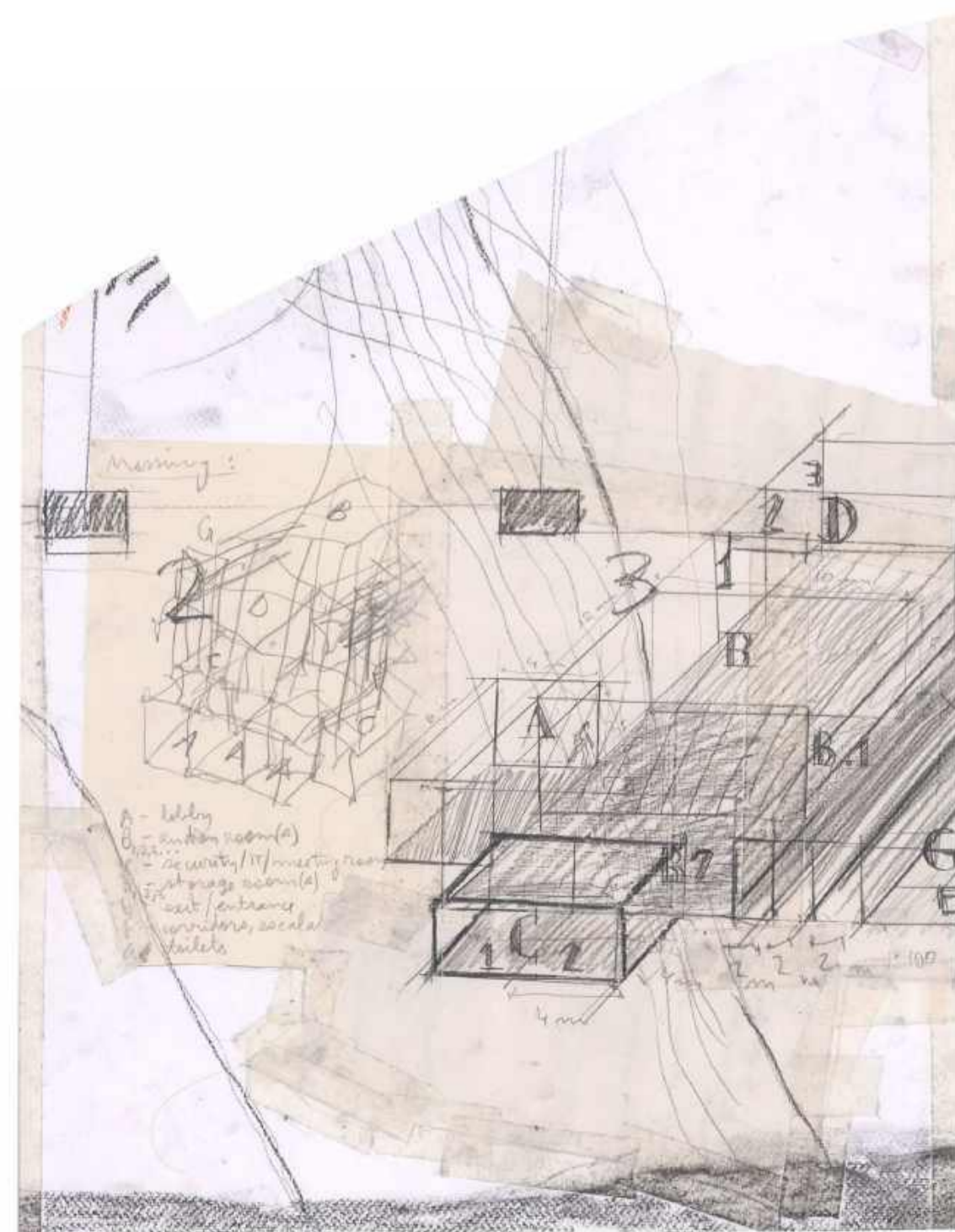


Section 3. Collaborative Urban Investigation Living Bridges

In designing a bridge that would serve as an auction house, I found it absolutely vital to research on past examples of bridges that have buildings for residential and business use. Looking at past examples of bridges that were created influenced me in my design and it is always helpful to look at problems I might not have thought of before. It was delightful to be able to link my hometown Paris into this overall project, especially my fondness over Parisian architecture. Specifically the architecture that was before Haussmann and that was destroyed, such as the Pont au Change, to which its history is quite fascinating, specifically how one of the main driving forces for the creation of this residential houses over the river were created to avoid land taxes. Of course, one of the most famous references that I used was the Ponte Vecchio in Italy, which I felt obliged to include for both its iconic look and to have certain people understand that the idea of constructing a floating bridge is not as crazy as it sounds. There have been past examples, and thus this bridge is taking inspiration from both past traditional examples whilst looking at innovating it and fitting it within the contemporary world. It is important to highlight that I drew inspiration from both historic landmarks – Ponte Vecchio, Maidenhead Railway Bridge – and current landmarks the likes of Tokyo Gate Bridge. I was particularly inspired by the artist Christo whose temporary art installation 'The Floating Piers' managed to shock, amaze and make people stop a little with their busy lives and ponder. Despite the fact that 'The Floating Piers' was a temporary landmark – and mine is designed to be permanent – the way the materials were industrially recycled is very important to me. It links to my idea that we should raise the level of sustainability in a world that is trying to cope with such pressing matters like climate change.

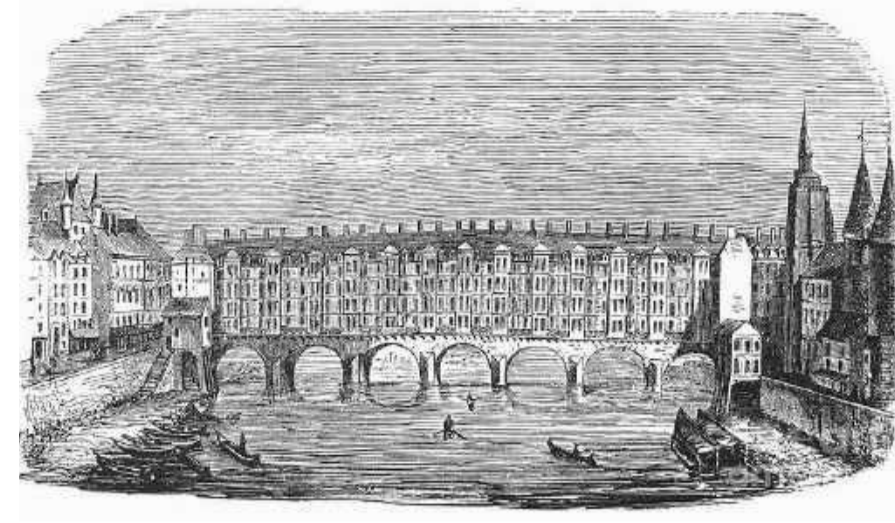


*even though it was done at the start, and was more of a formality to the brief, it helped to ground the project



Living Bridges and Infrastructures Case Studies

Case study 1: Pont au Change



Main feature: living

This bridge got its name from the money changers who built their houses and shops there in the 12th century. Although no houses exist now, it was common to have buildings on bridges in medieval Paris for two main reasons: to avoid land taxes and increase the stability.

Bridges might become a more viable living option in the future with rising sea levels and housing shortages. I could take inspiration from medieval Paris to increase the stability of my bridge.

Case study 2: Festa del Redentore Bridge



Main feature: floating

This bridge is temporarily constructed once a year for the Festa del Redentore during July which celebrates the end of the 16th century Plague in from 1577 and the construction of Palladio's church. It is a 330m long floating pontoon bridge which creates a walkway to the church which is usually only accessible by boat.

The floating mechanism could be inspiration for a pedestrian walkway feature of my bridge design, incase the Thames water level rises in the next 50 years.

Case study 3: West India Quay Bridge AL_A



Main feature: floating

his footbridge was built in 1996 over Canary Wharf, it is 94m long. It is built with X-shaped tubular steel struts bolted to pontoon floats, which are filled with polystyrene. The bridge also has wheels at either end, so in theory is fully moveable.

The floating structure could be inspiration for my bridge design to allow adaptability to the Thames water level rise.



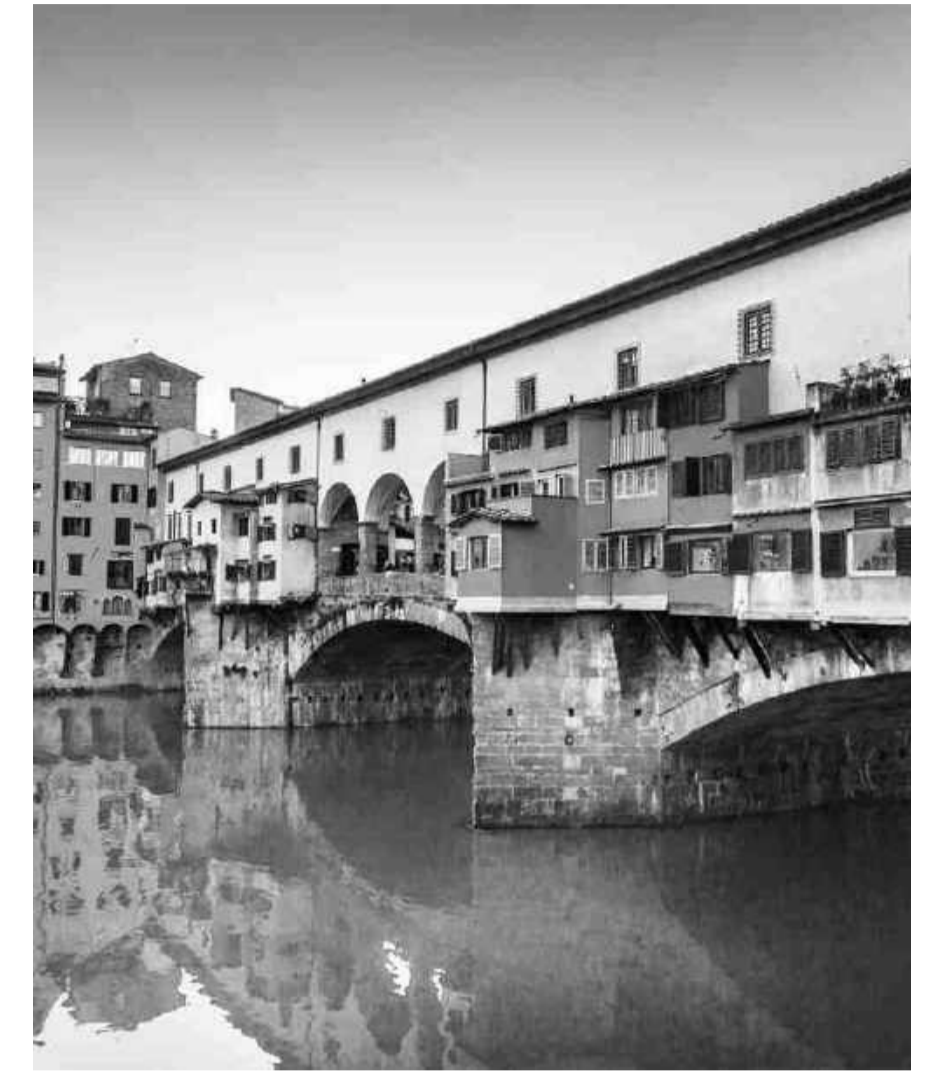
Case study 4: Ponte Vecchio Bridge



Main feature: living

This was believed to have been built in the Roman times, but notably recorded from medieval times. It was common place to build shops on bridges in Florence then. It wasv originally full of butchers, tanners and farmers but in 1595 they were excluded and replaced with mostly jewellers. Today, these prevail along with art dealers and souvenir shops. An important feature of the bridge is the large piazza at the center.

It's rich inhabited history attracts many tourists and artists to the bridge, which could inspire the living art community of my bridge.



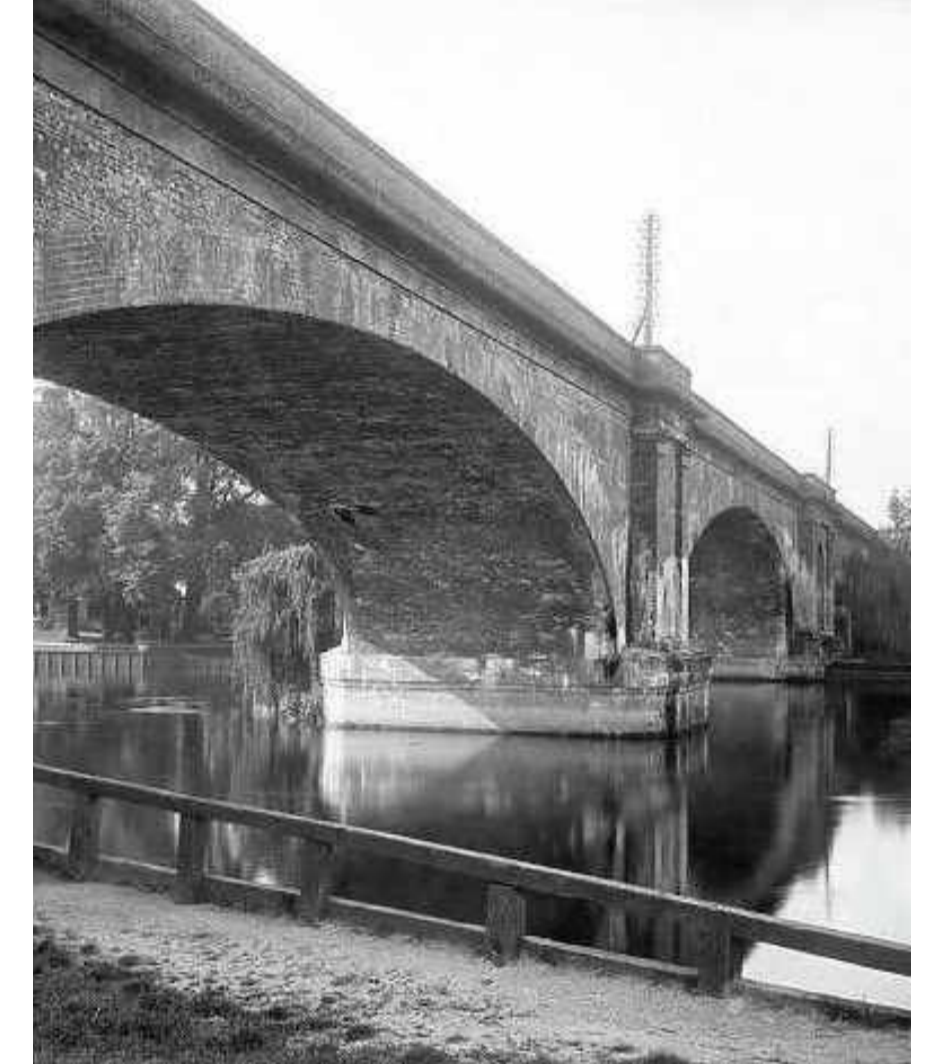
Case study 5: Maidenhead Railway Bridge



Main feature: masonry

Masonry bbridges are extremely durable and can last for a thousand years. This bridge was designed by Brunel and opened in 1839 for the Great Werstern Railway line to connect London to Bristol. It was the flattest and widest brick arch in the world at the time. Many didn't believe that the shallow arch could withstand the load but they were proved wrong.

Masonry bridge structures feature a main supporting arch so in order to use ceramics structurally, the form will be quite restricting. However, ceramics can be- used more for environmental qualities



Case study 6: Tokyo Gate Bridge

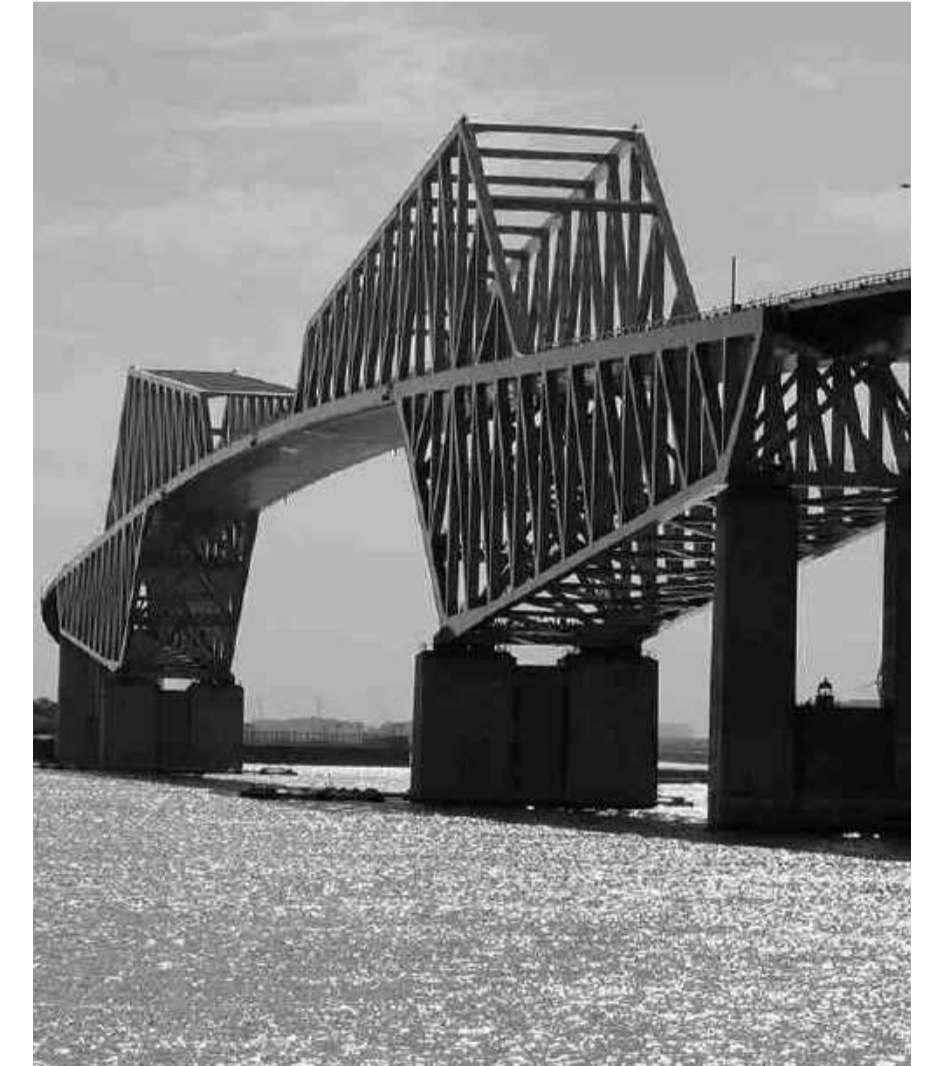


Main feature: cantilever

This bridge was built in 2012 to carry a heavy flow of traffic across East Tokyo. It had height restrictions due to air traffic and vessel crossings so the structural solution used was a steel truss cantilever system. It has an overall length of 2,618m.

It is also a leading example of the application of Internet of Things technology in Japan where over 50 sensors across the bridge are used to collect and analyse environmental data in order to mitigate future climate crises, eg. earthquakes

The cantilever system of the bridge could be used to inspire the structure of my bridge....



UNCHECKED
 It is likely that not all case studies were relevant.
 20th June 2023 London, UK
 @proverby

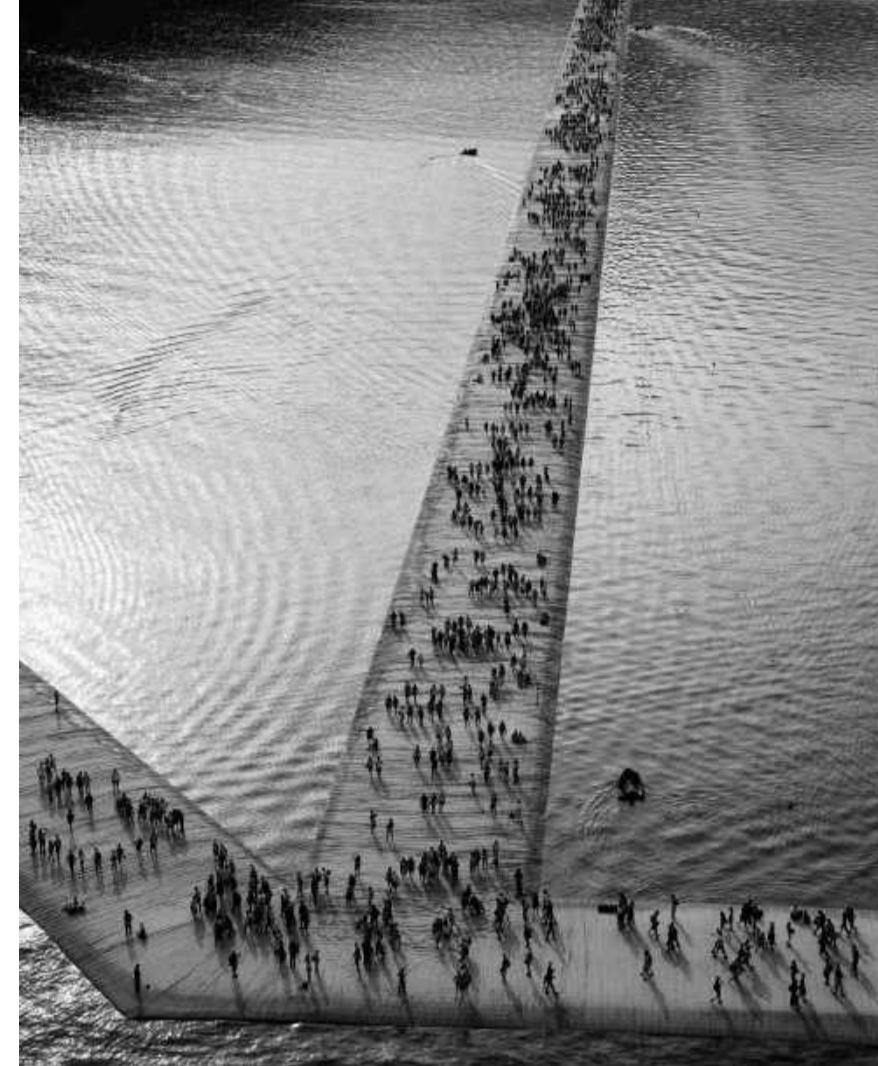
Case study 7: The Floating Piers by Christo



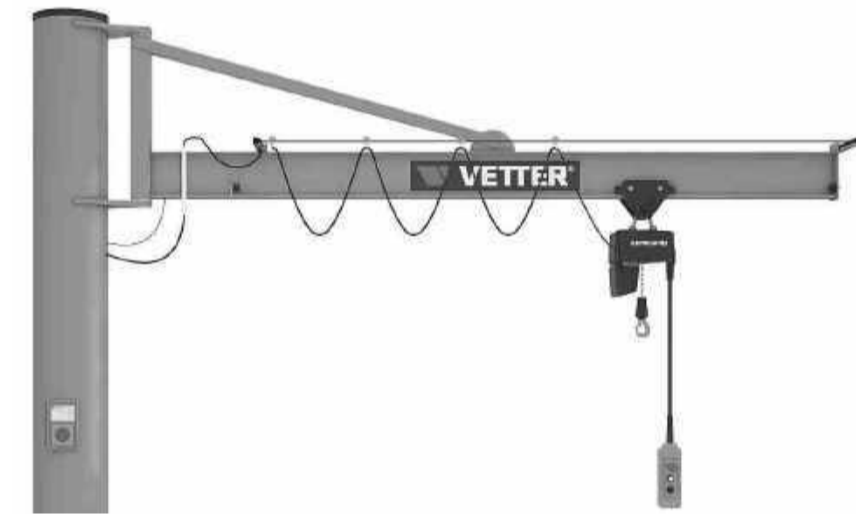
Main feature: floating

This floating walkway was temporarily installed in Lake Iseo, Italy, from June to July 2016, it was constructed using a modular floating dock system of polyethylene cubes (shown above). It was covered in 100,000 square meters of bright yellow fabric. The piers were 16 meters wide and 3 km long. After the 16 day installation, all parts were taken down and industrially recycled.

The temporary nature could inspire my project - to create walkways that go through the living spaces but can adapt and move to moving water levels.



Case study 8: JIB crane system



Main feature: pulley system

JIB cranes are one of the most simple types of cranes used. They consist of 3 main components: an arm/boom/jib, a trolley and a lifting device. Floor-mounted jib cranes have a vertical column to support the arm. The lifting device could use an electric chain hoist, a pneumatic chain hoist or a manual chain hoist. The jib arm can rotate up to 300 degrees or be positioned as stationary.



Case study 9: Barge boat



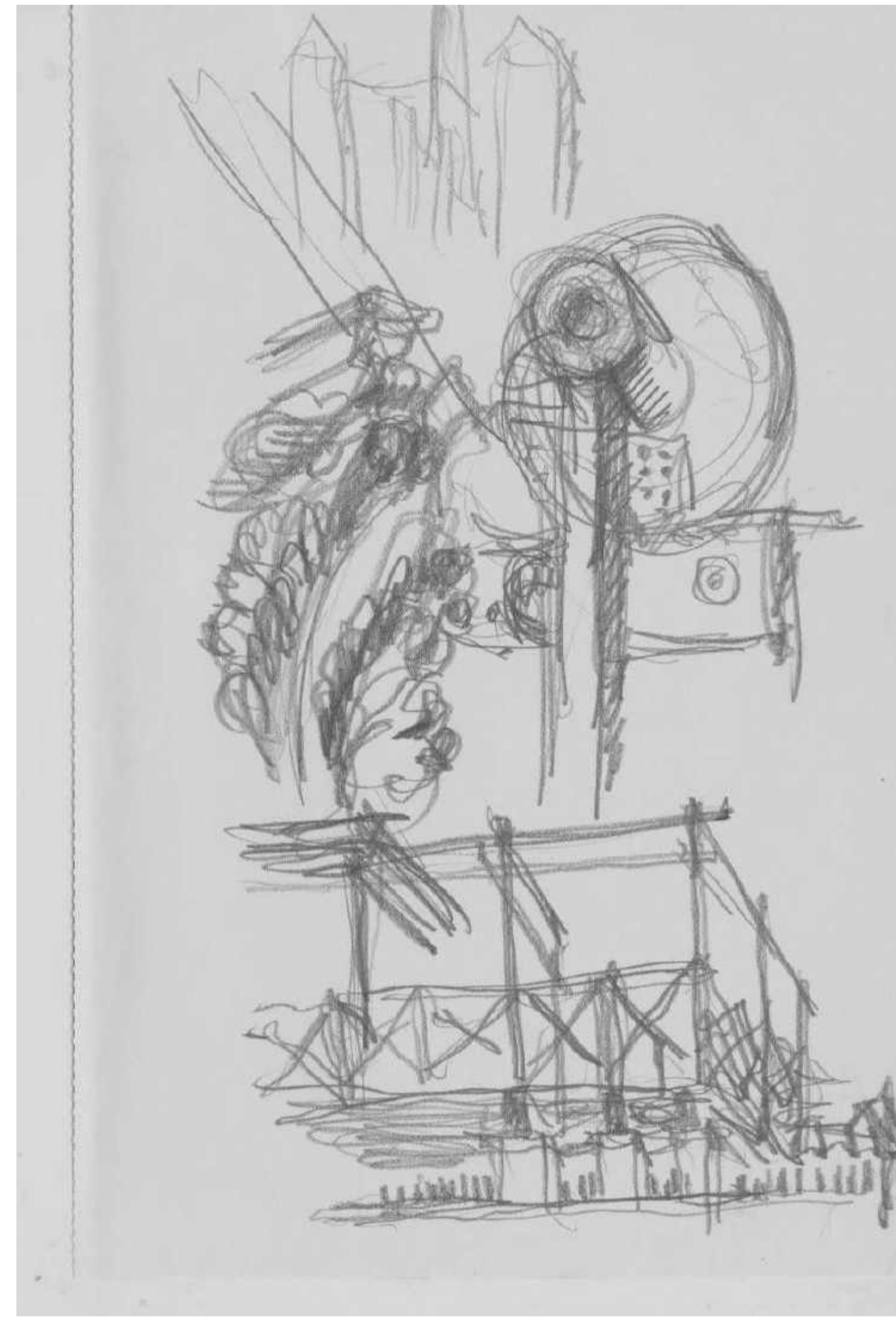
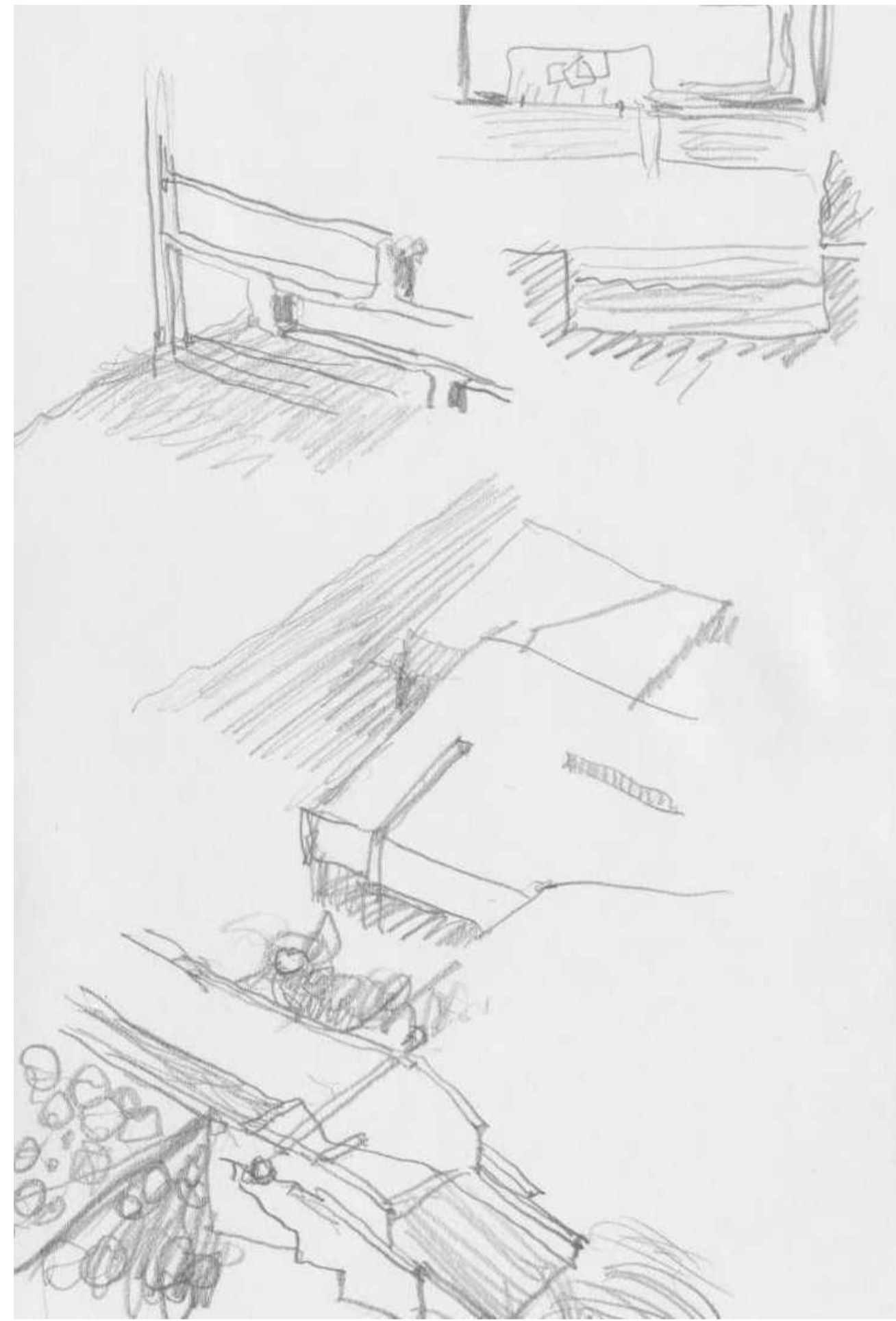
Main feature: floating

The narrow barge boat surround the entrances of the site and host a thriving artistic community.



First Bridge Concept

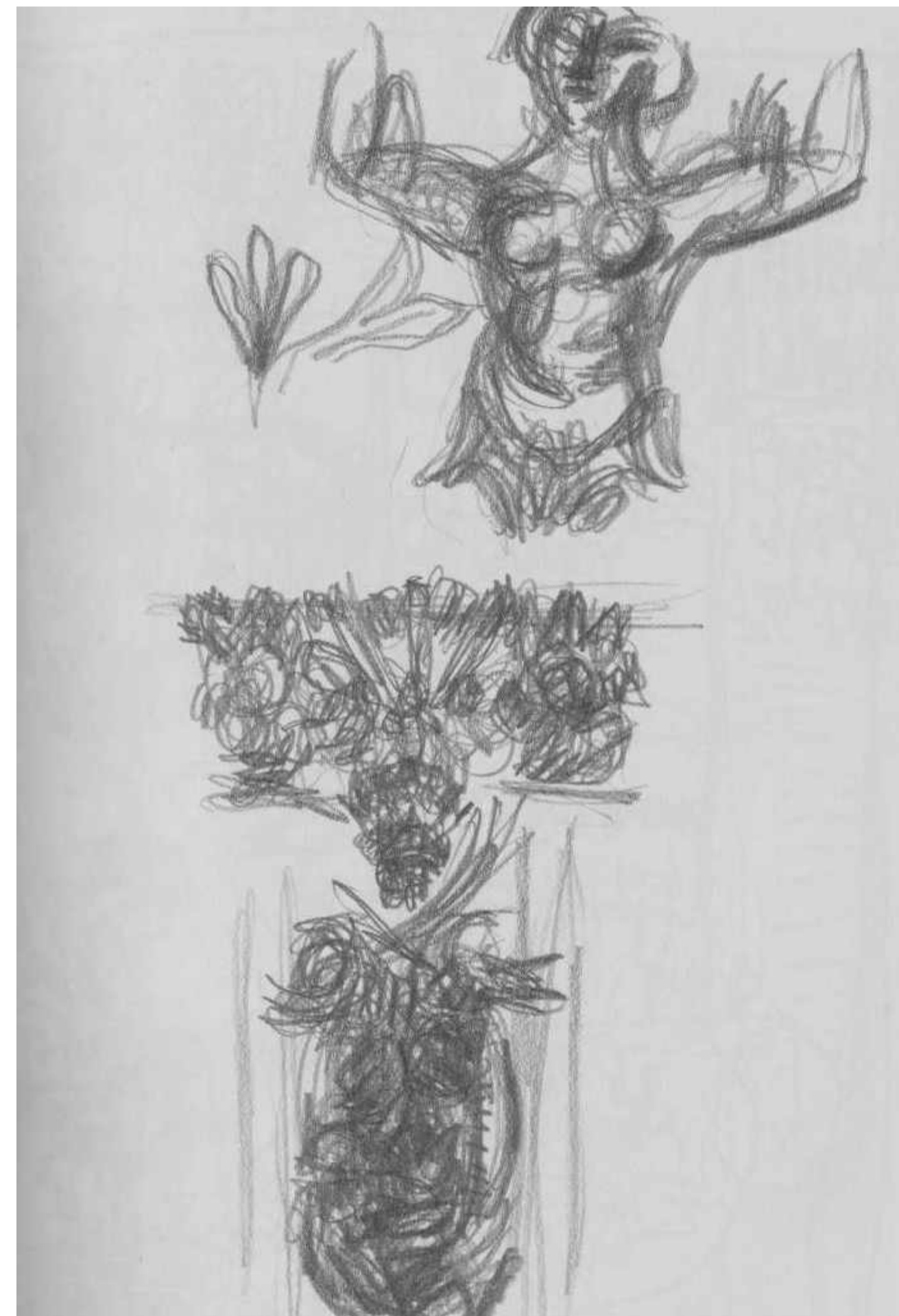
Venice Sketches of Bridge Details

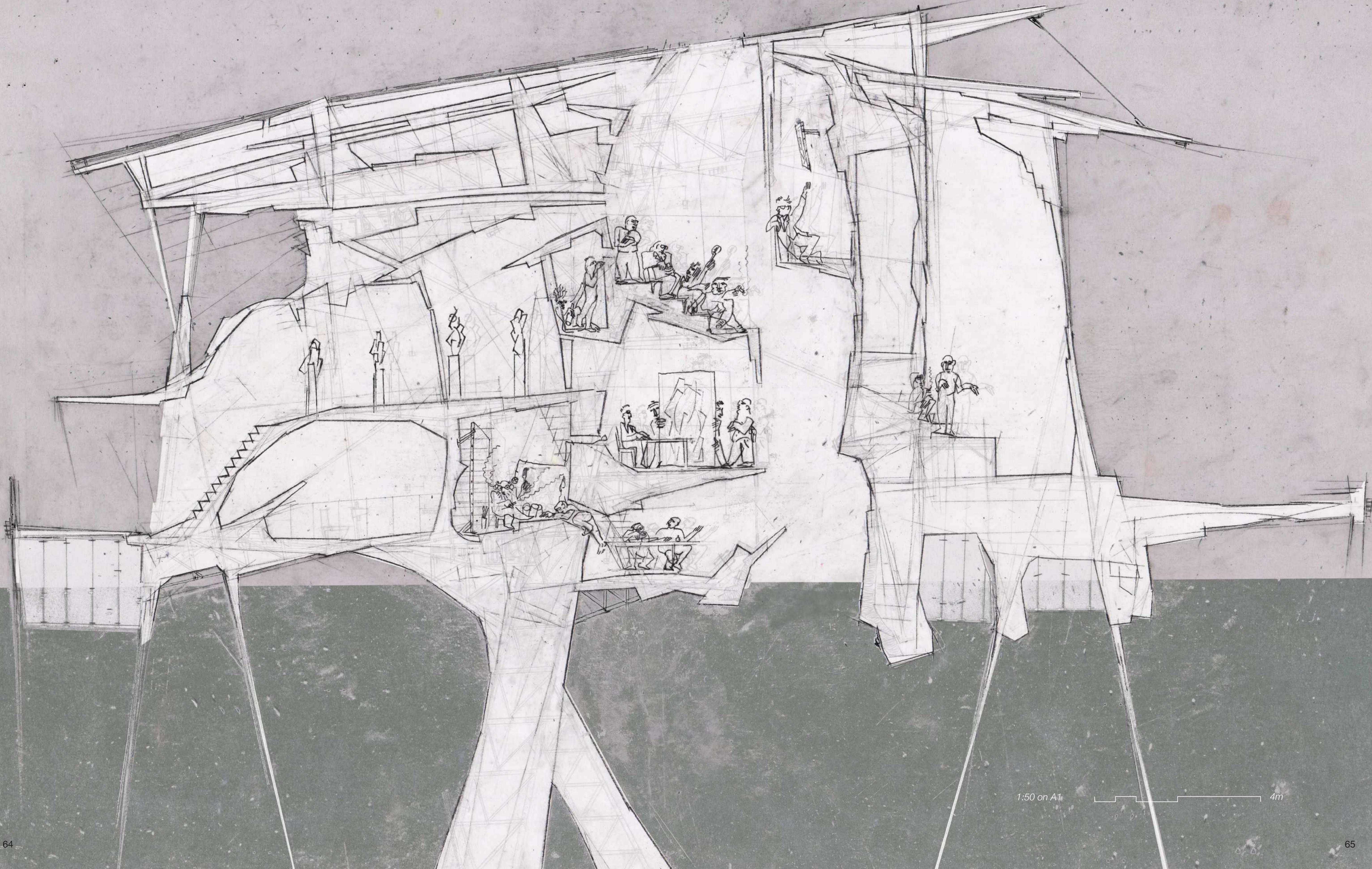
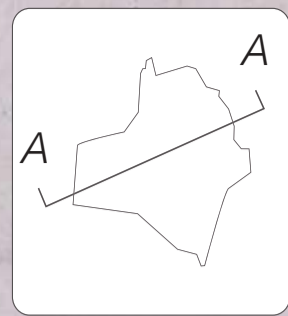
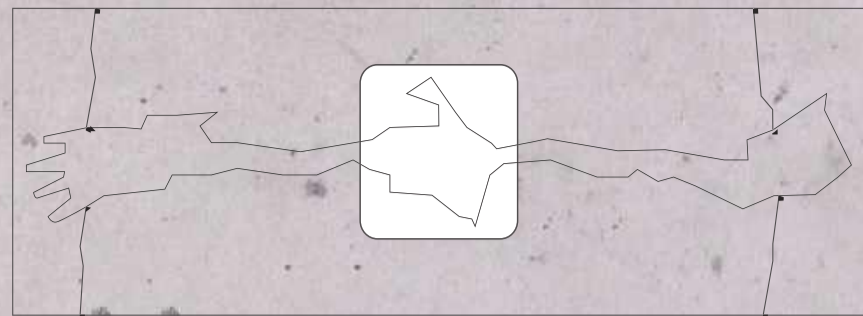


Mosime 22 Venice



Venice Mosime 22



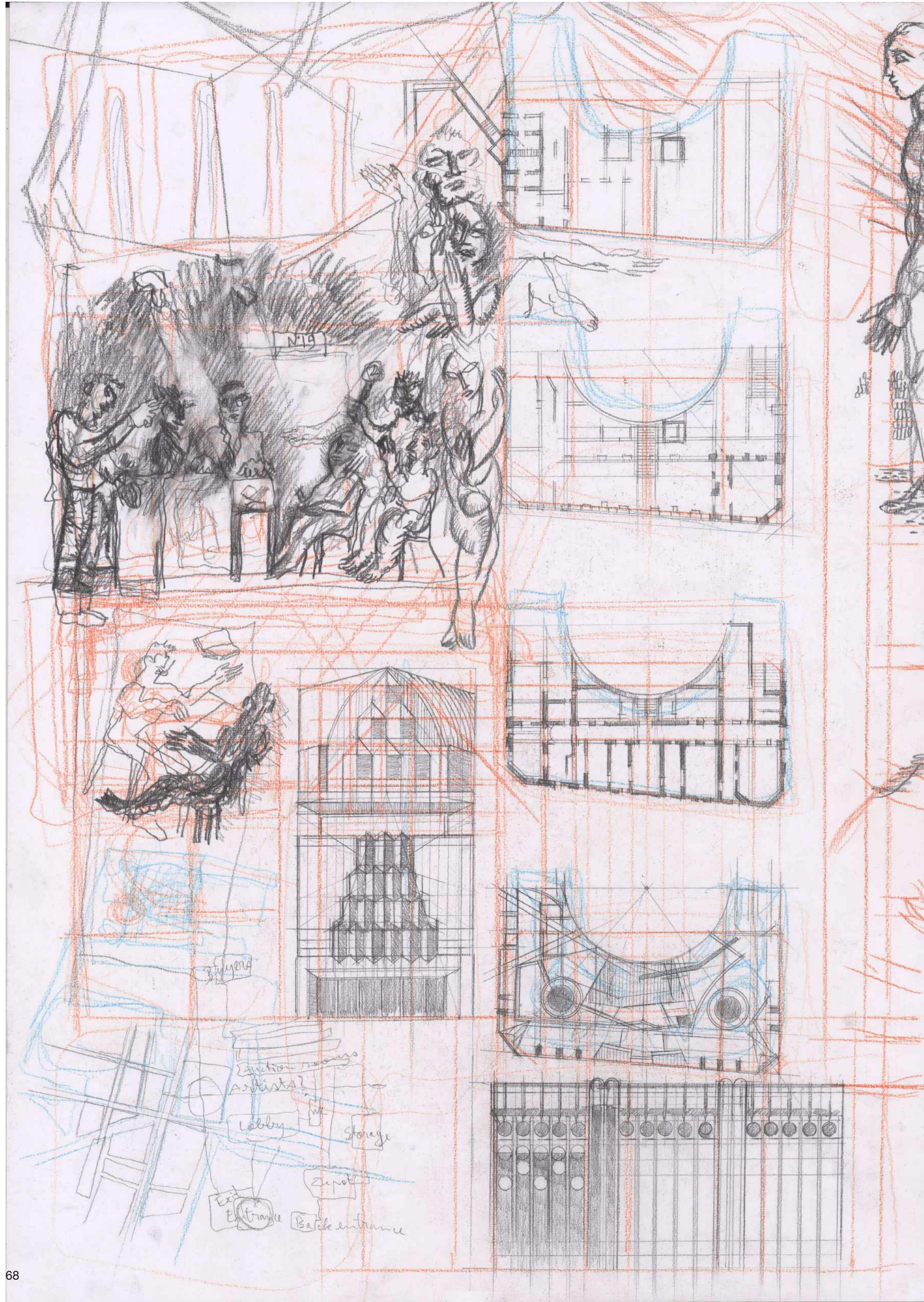


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4m





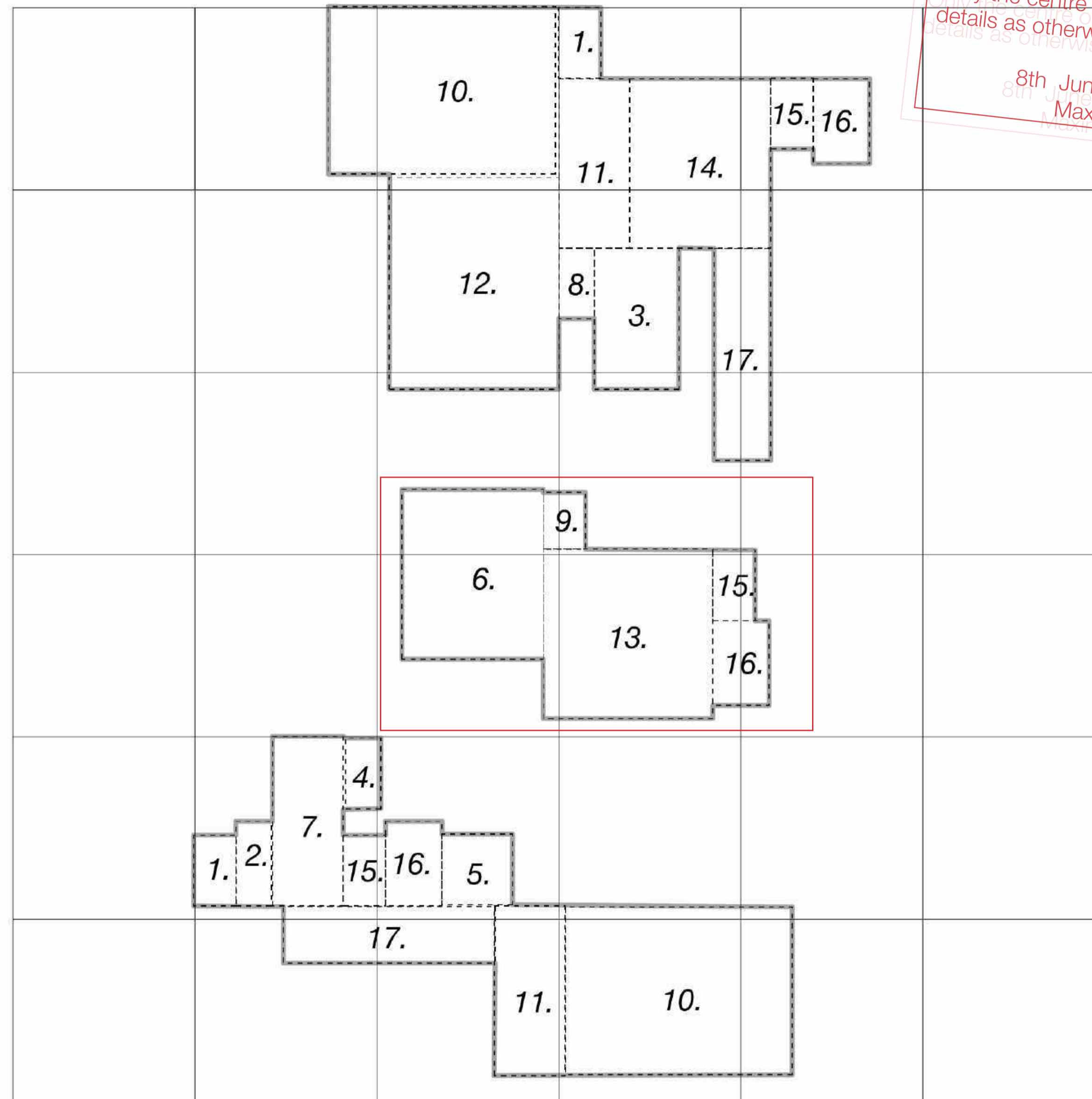
Section 4. Organisation of Adjacencies: Programme

In order to create an auction house, it must function as an auction house. In order for that to be achieved, I looked at already existing auction houses in the world – Drouot – as well as taking inspiration from art spaces, specifically those that I have personally visited, such as the White Cube Museum in Bermondsey, London, and private art studios in Paris.

In the process I have made a diagram to reiterate the central message of this work, through creating a visual understanding of the way the art business world functions. In understanding this system, we can then start to think about possibilities to foster a healthy, balanced and fair relationship between the artist, the dealer and the buyer. The diagram put down two examples, two ways in which we could live. One is within a buyer-dominated market, and the other is an artist-dominated market. I am inspired by the Cubitt Art Co-op in London which seeks to create a market in which the artist has a fairer say in what happens to his artwork which he, after all, remains the art's creator. All of this was key in influencing me for the overall architectural design of the auction house on the floating bridge. Through a series of initial sketches (see Appendix), an intuitive approach was taken to create a paper model of the bridge. I have sought to create a thought-provoking, feasible and beautiful building which would reflect the central ethos that the auction house is trying to emulate: a place where the artist's work is sold with fairness as artists and curators get a fair share of the business and the art spaces.

The uneven shapes, reflects on the decomposing piers on the river banks, the rusted metal chains and boat fragments.

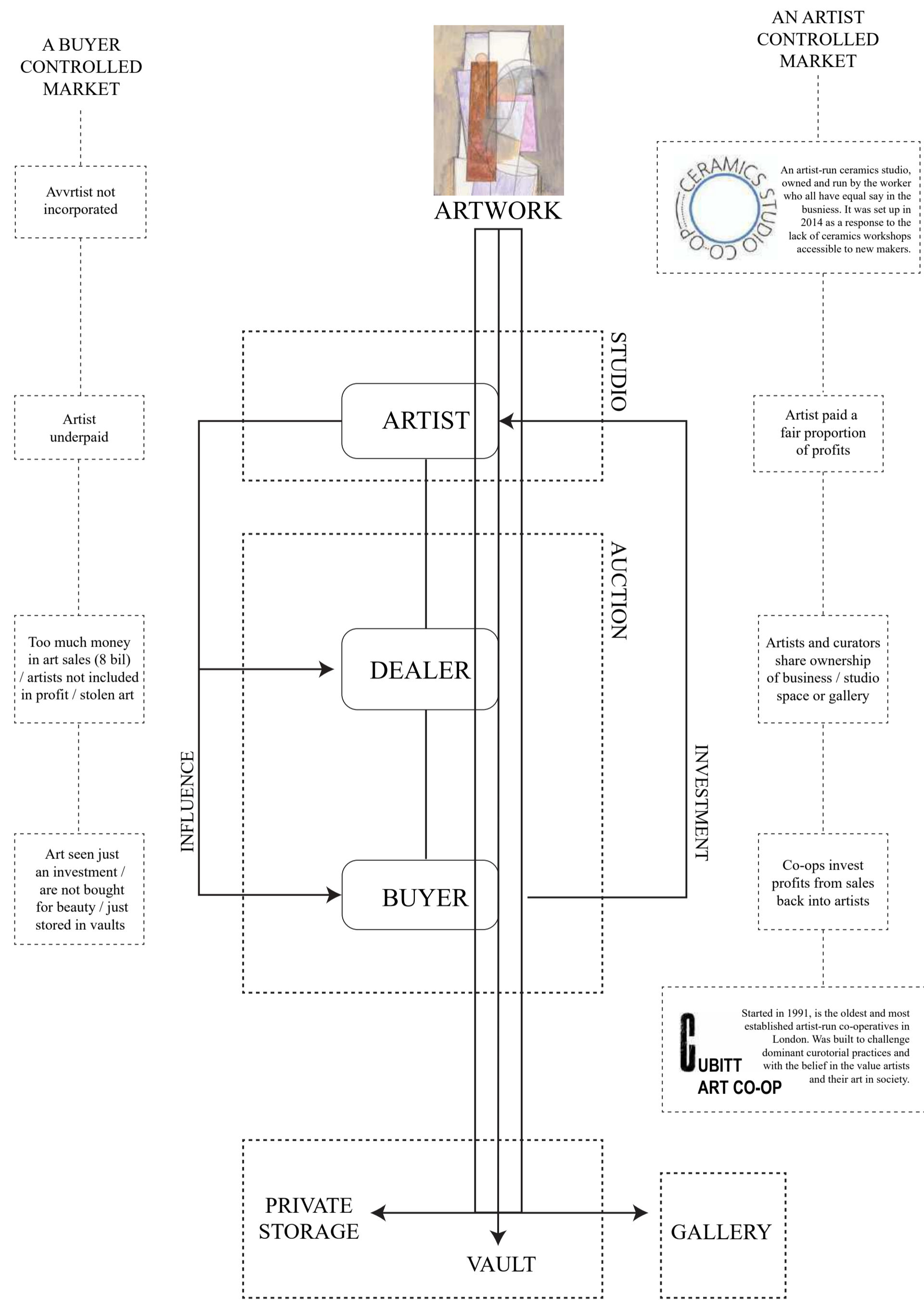
ADJACENCY DIAGRAM



CURRATOR'S NOTE
 Only the centre of the bridge was designed in details as otherwise the proposal was too big.
 8th June 2023 London, UK
 Maxime Ostroverhy

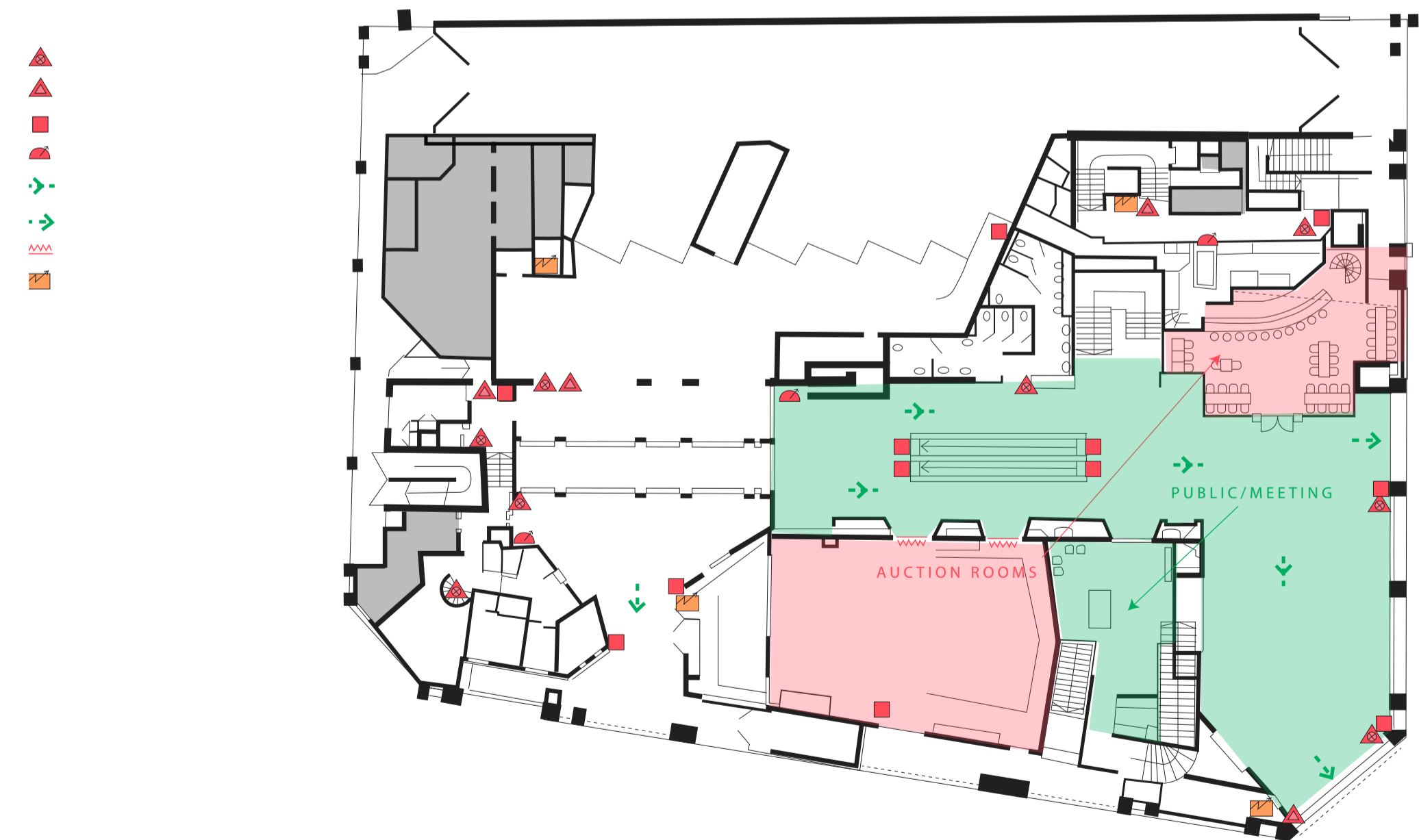
- | | |
|-------------------|-----------------------|
| 1-entrance | 10-offices |
| 2-bookshop | 11-boat entry |
| 3-north galleries | 12-storage |
| 4-auditorium | 13-AUCTION ROOM |
| 5-south galleries | 14-art restauration |
| 6-central gallery | 15-toilets |
| 7-viewing rooms | 16-boiler room |
| 8-archive | 17-maintenance |
| 9-meeting room | 18-dam for hydropower |





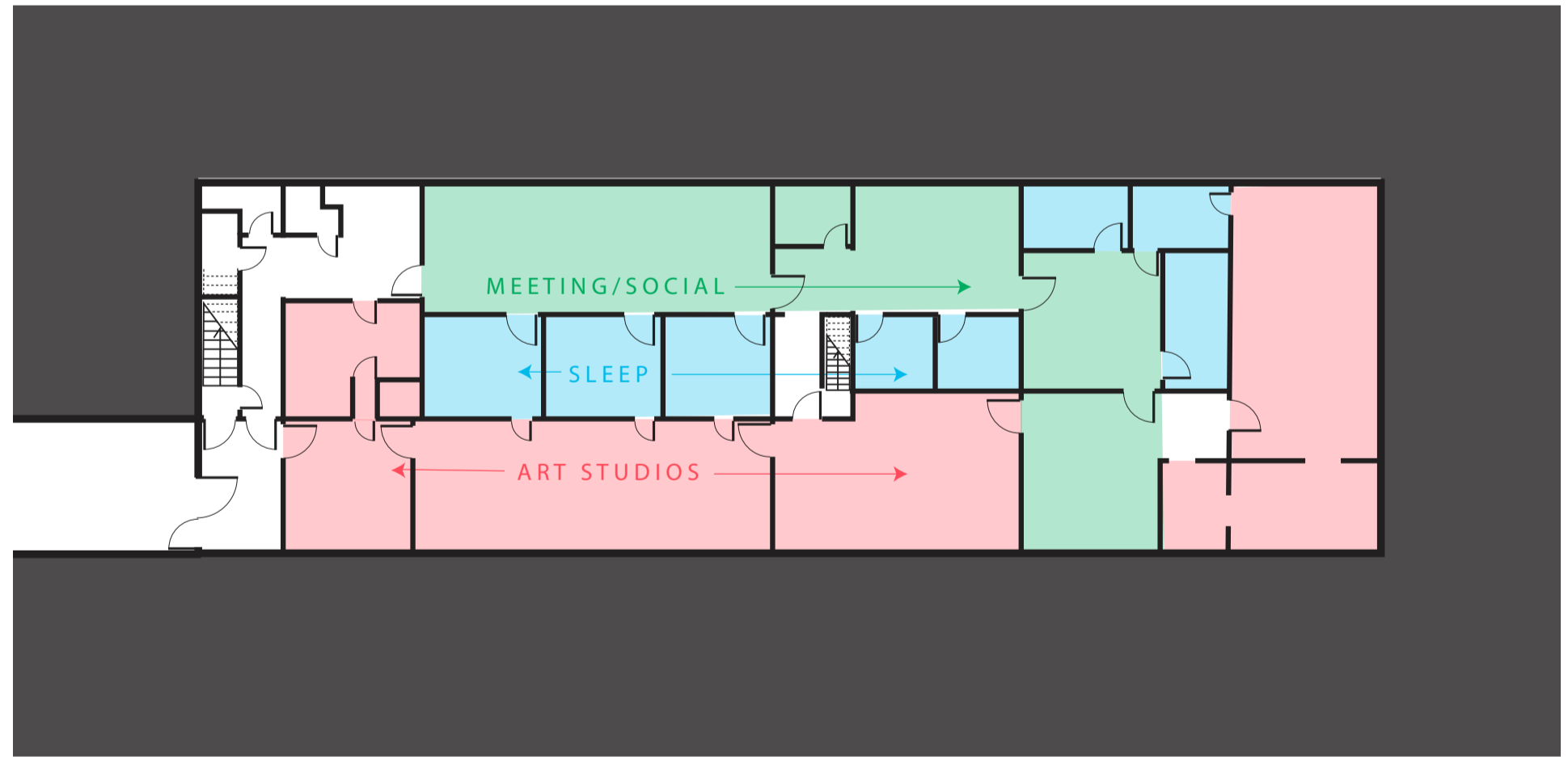
Drouot, Paris

Drouot auction house was established in 1852 as part of the hotel, it is one of the oldest and biggest public auction houses that exists today. It houses 18 salerooms which hold auctions of pieces from a wide range of disciplines including archeology, islamic art and wines. These are always accessible to everyone.



The Territory, Paris

This is an art community space in the south of Paris, where art assistants can co-live in exchange for help on various projects. There are various flexible art studio and gallery spaces, with shared equipment and amenities which the residents can use for many different creative projects. It was built into a courtyard surrounded by Haussmanian apartment buildings, so relies on the polycarbonate roof for its sunlight.

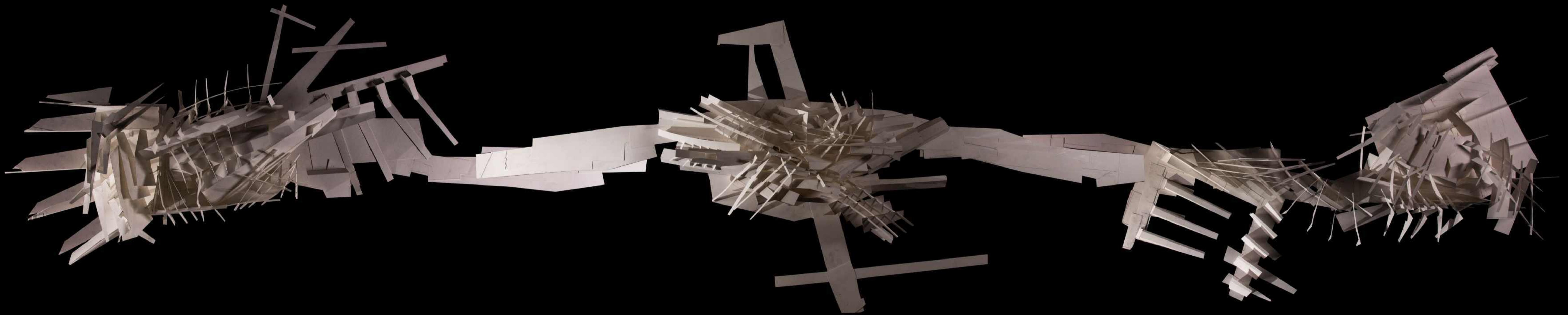


White Cube, Bermondsey

The White Cube in Bermondsey was opened in 2011, sitting inside a former 1970s warehouse. At the time it was Europe's largest commercial art gallery, since then it has provided an early exhibition venue for many now internationally acclaimed artists, such as Tracey Emin, Gilbert & George and Antony Gormley. It holds multiple exhibition spaces, a warehouse, private viewing rooms, a bookshop and auditorium. The industrial character of the building was enhanced and maintained by retaining the existing structure and with the use of concrete and steel.

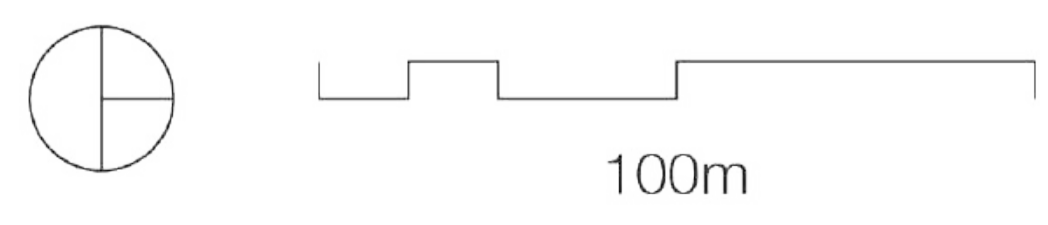
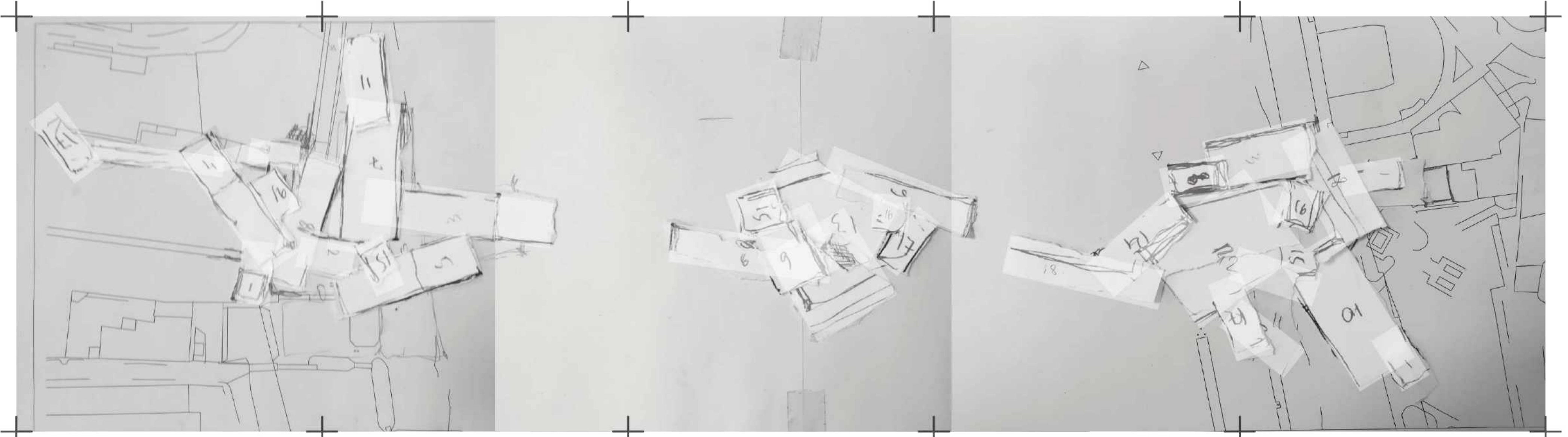
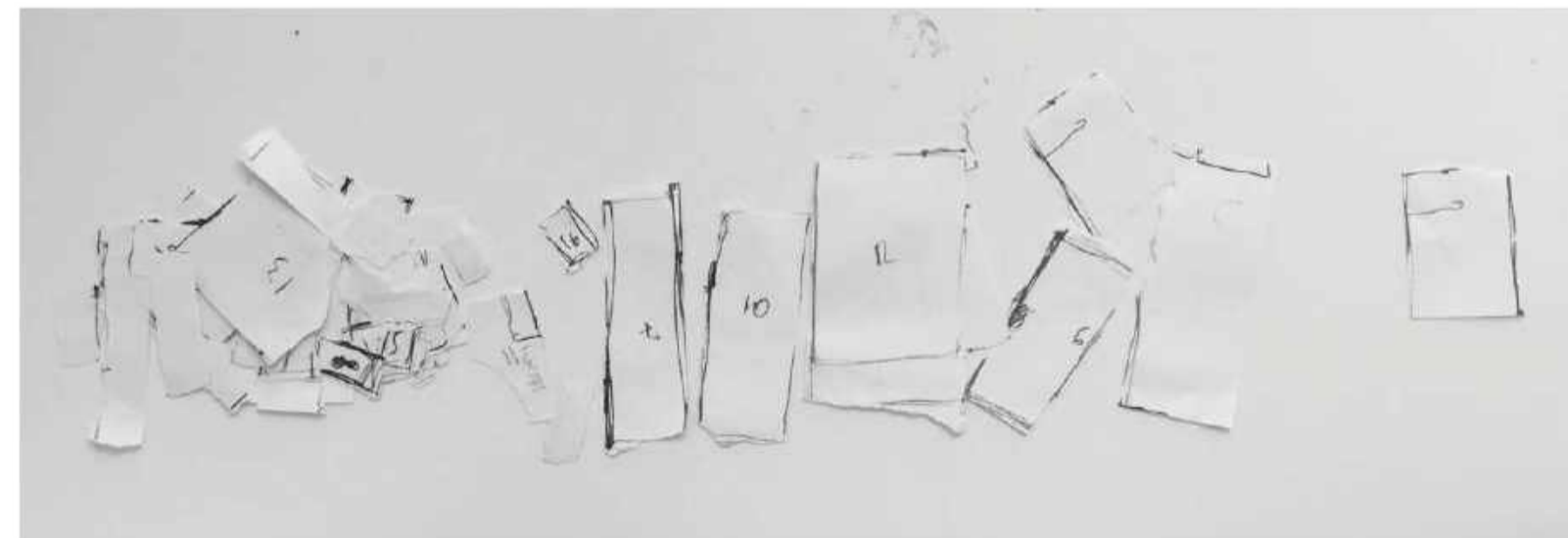
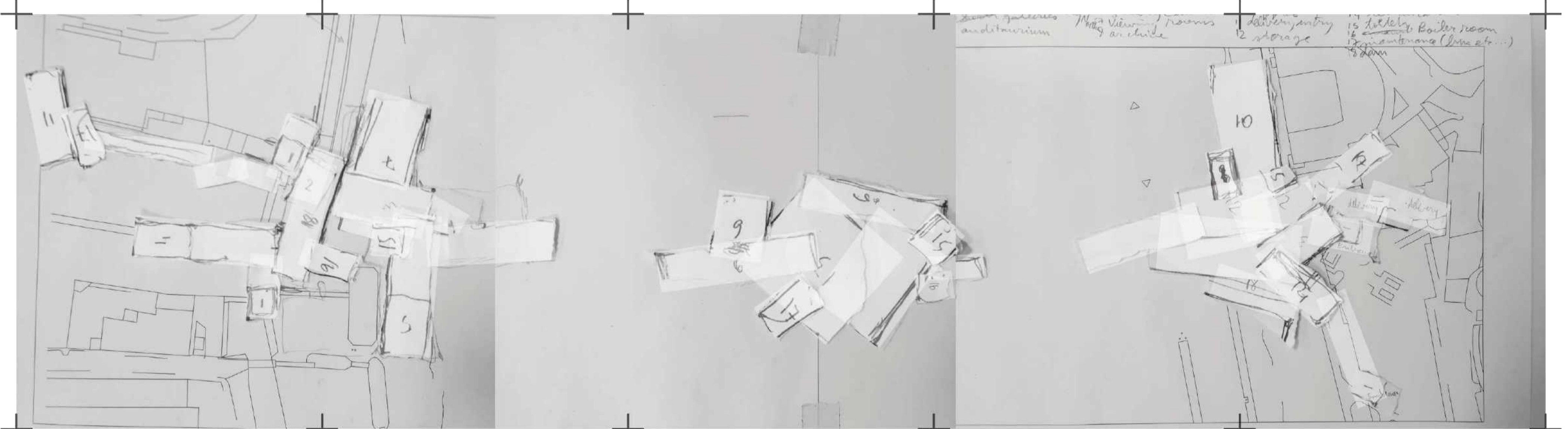
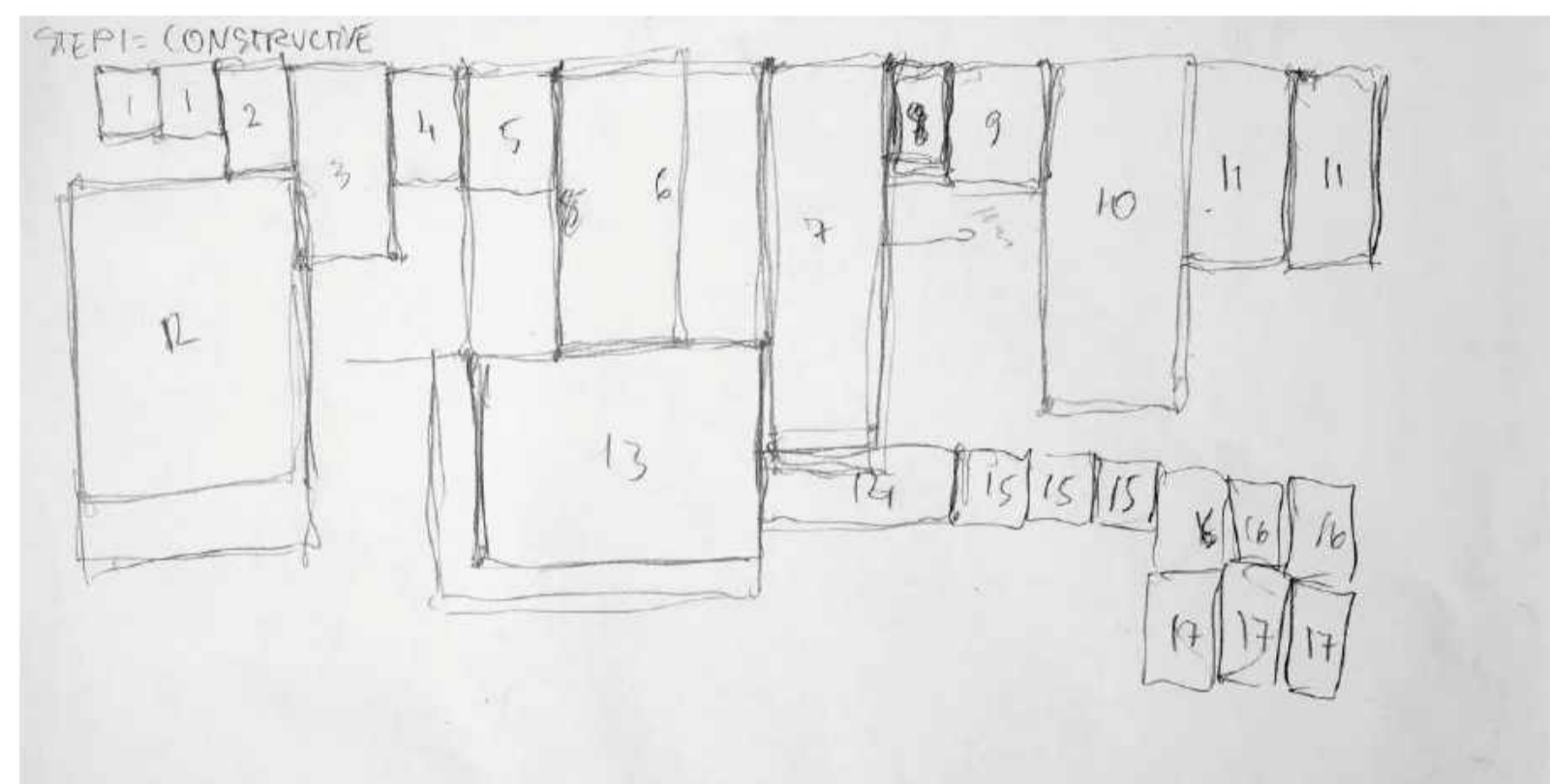
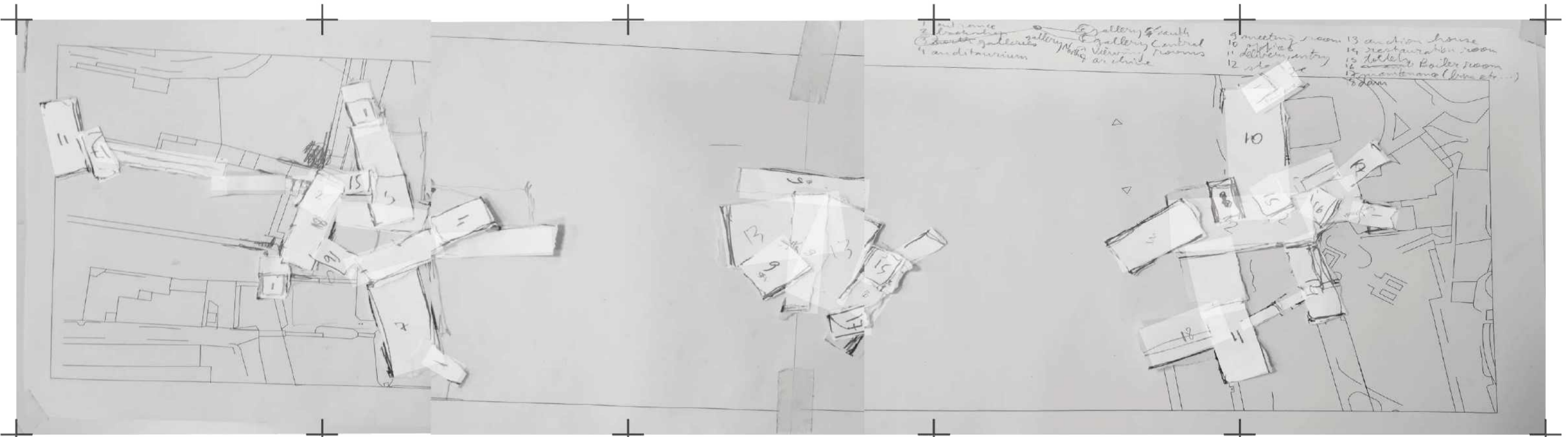
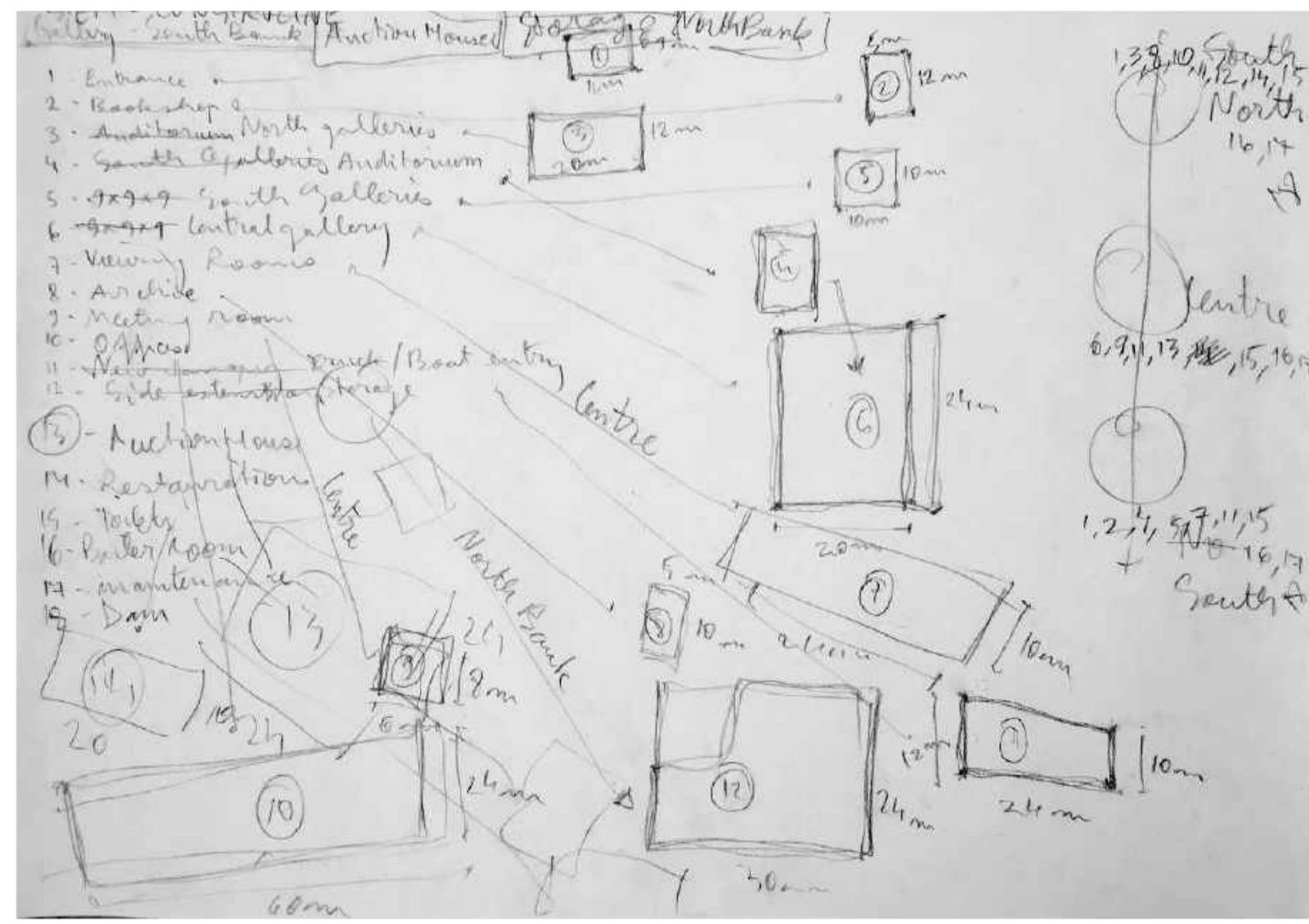




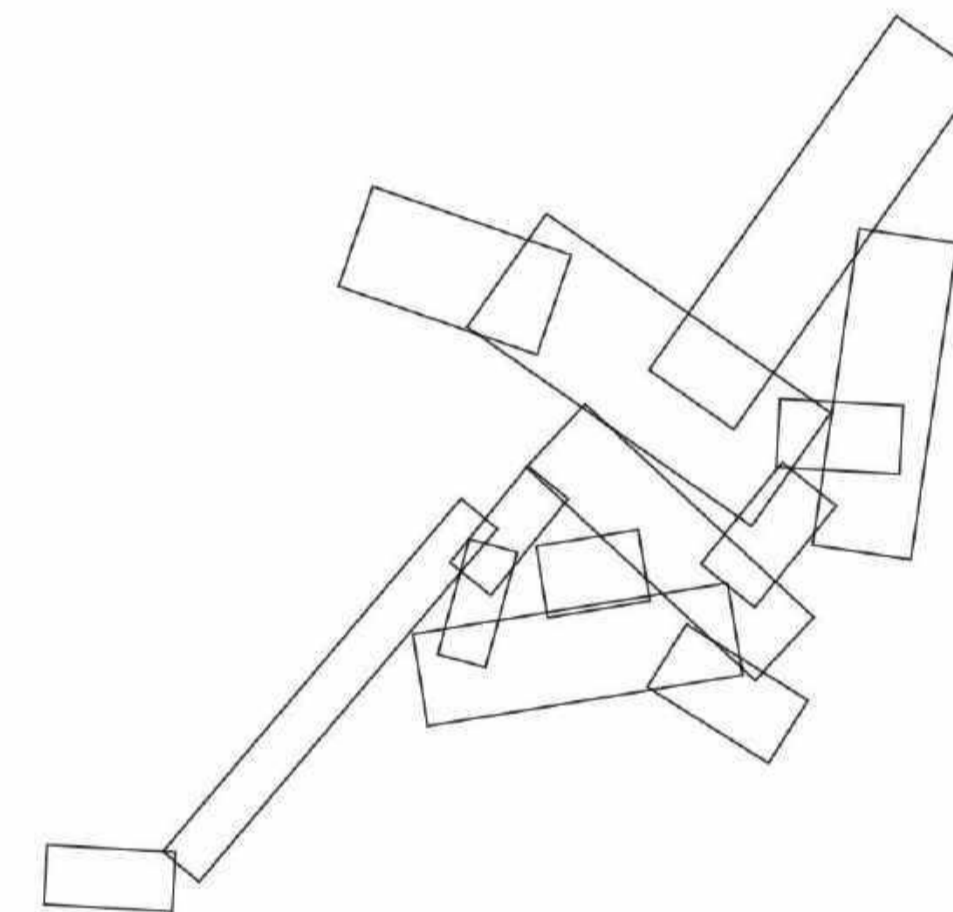
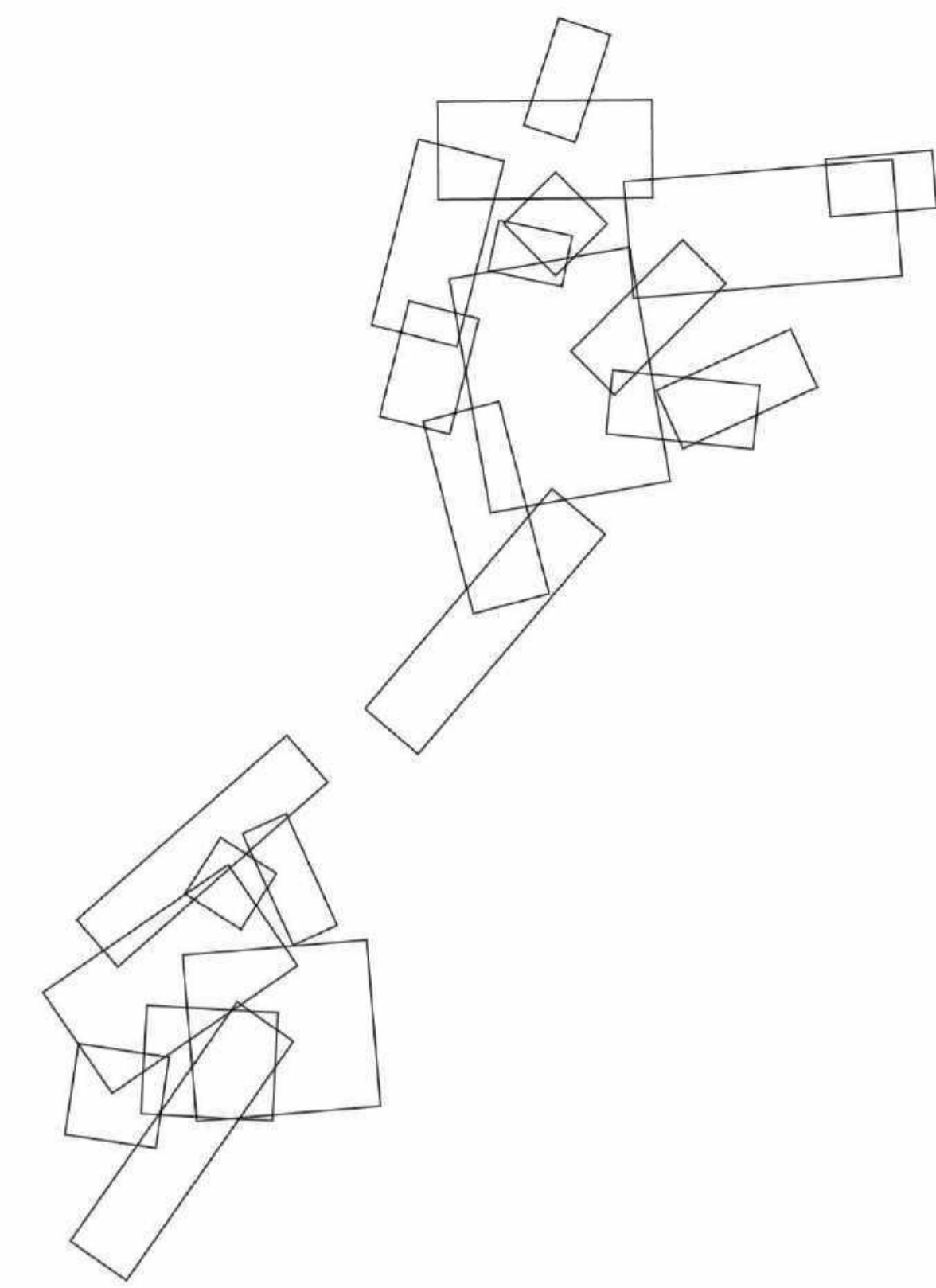
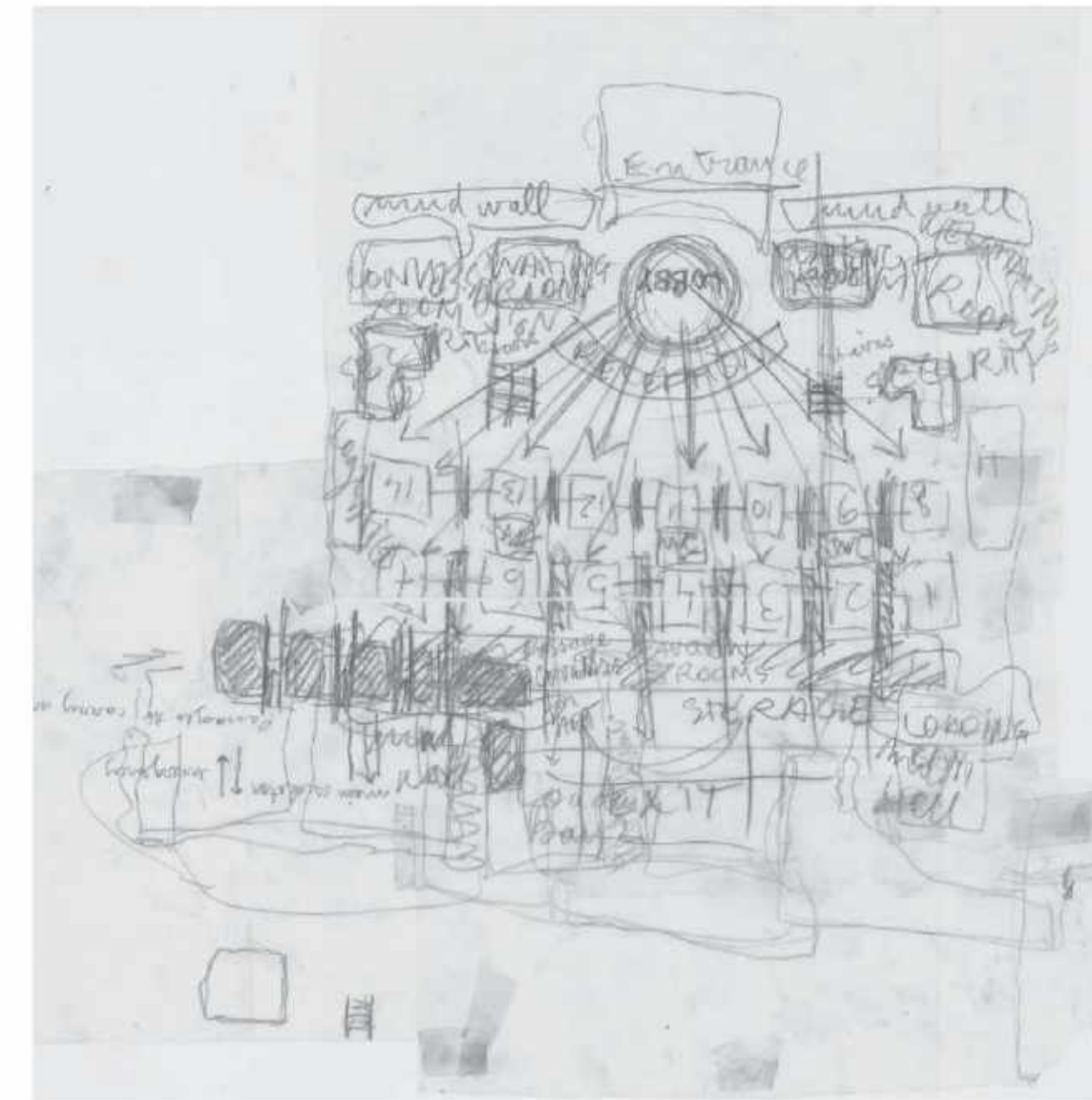
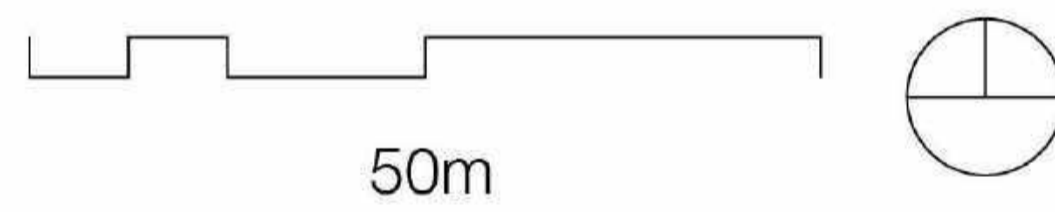
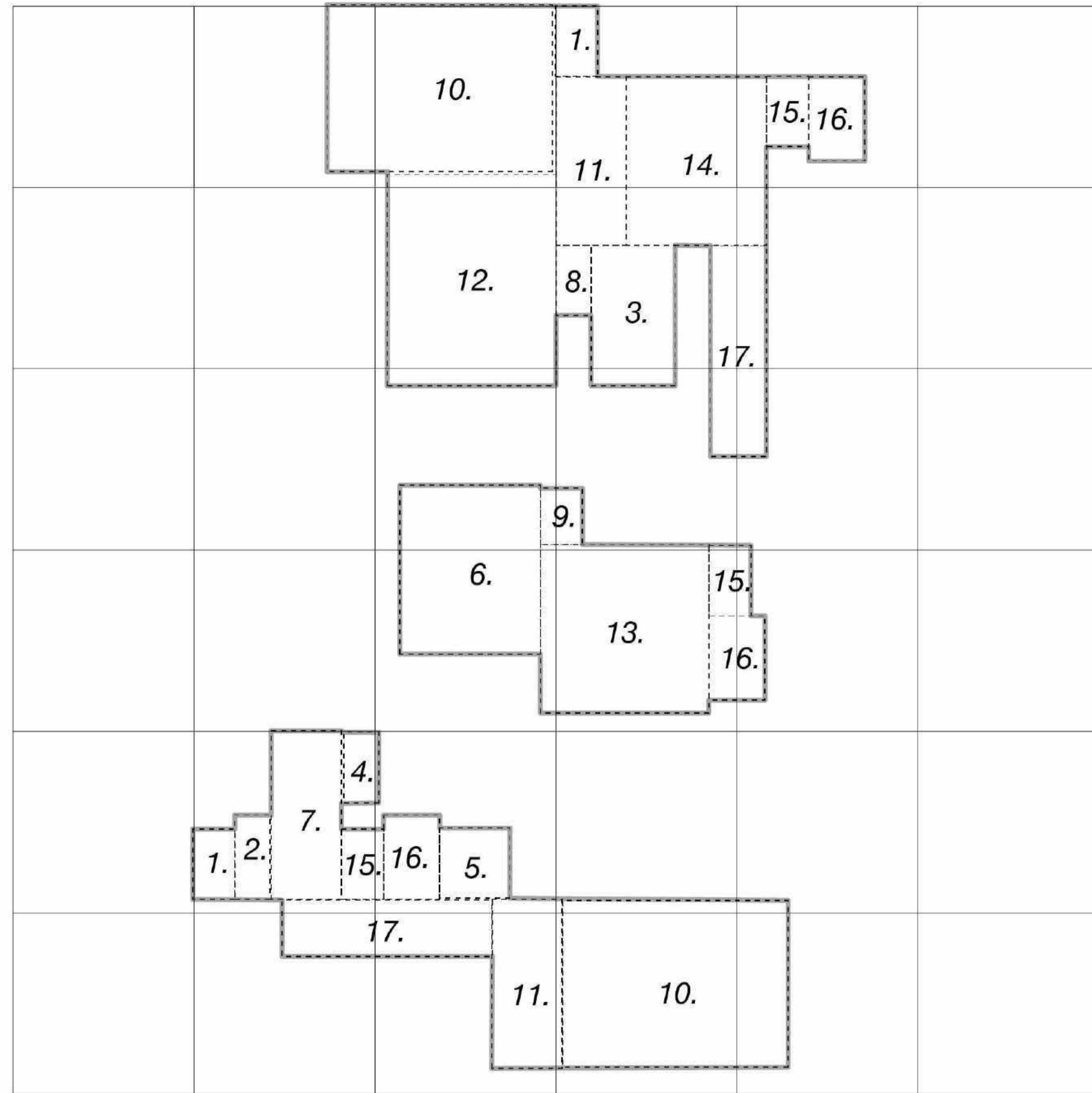




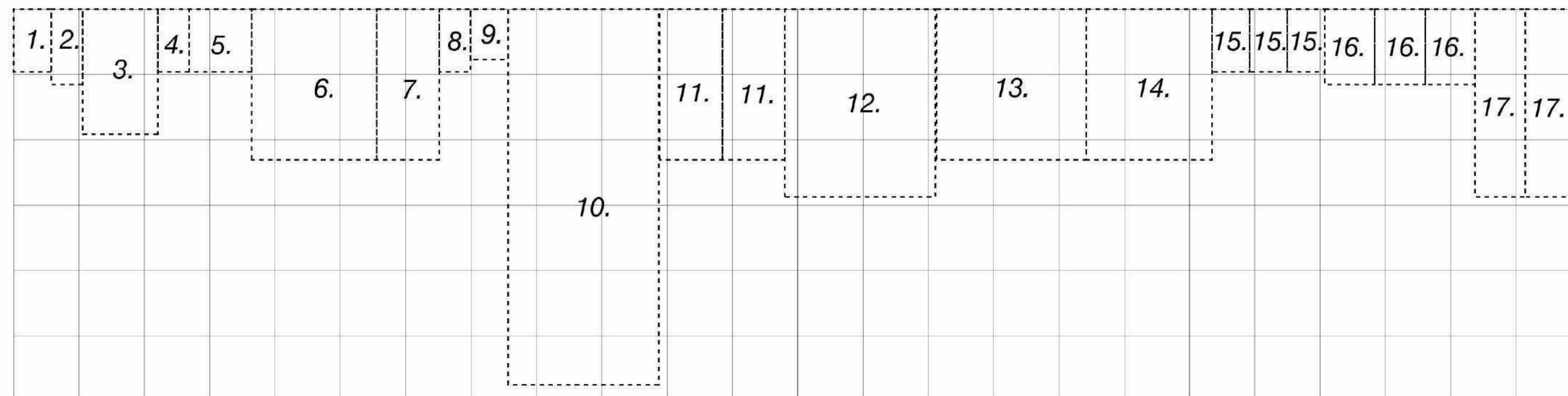
Massing



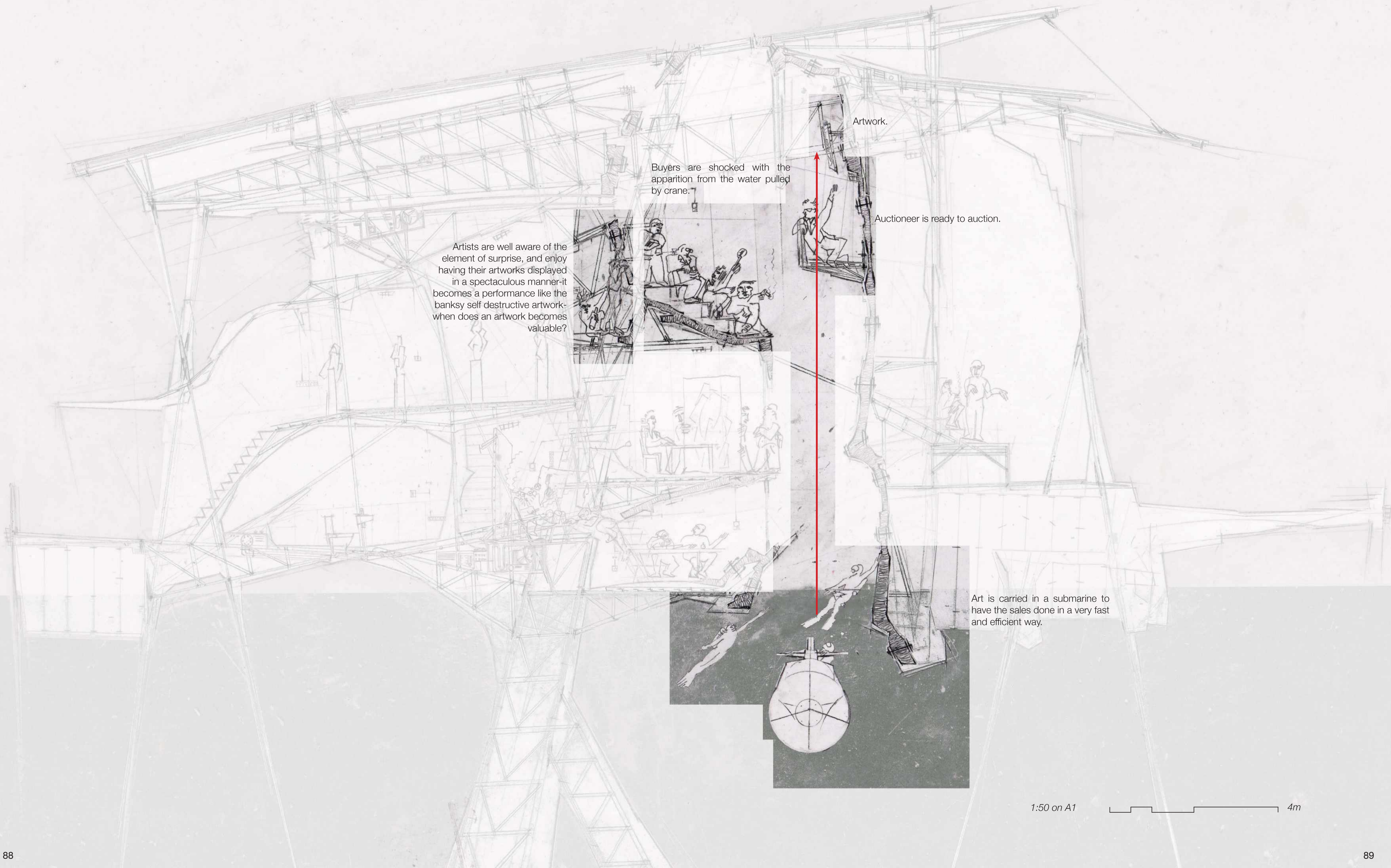
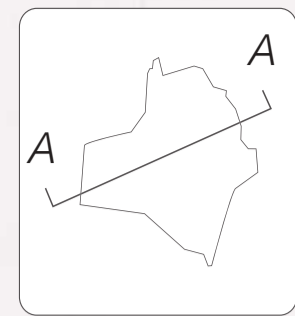
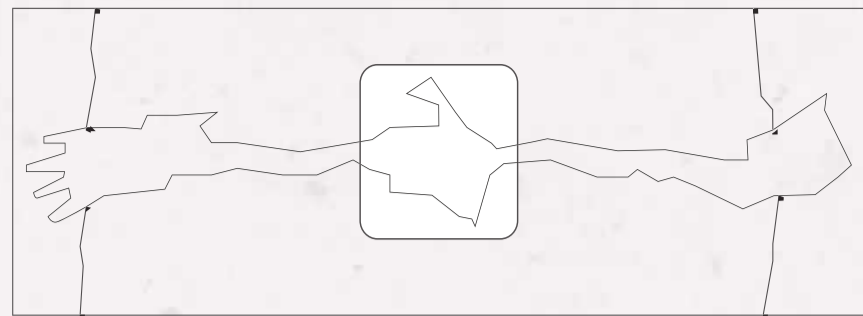
Adjacency diagram



- 1-entrance
- 2-bookshop
- 3-north galleries
- 4-auditorium
- 5-south galleries
- 6-central gallery
- 7-viewing rooms
- 8-archive
- 9-meeting room
- 10-offices
- 11-boat entry
- 12-storage
- 13-AUCTION ROOM
- 14-art restauration
- 15-toilets
- 16-boiler room
- 17-maintenance
- 18-dam for hydropower



10m
10m



Artists are well aware of the element of surprise, and enjoy having their artworks displayed in a spectacular manner-it becomes a performance like the banksy self destructive artwork-when does an artwork becomes valuable?

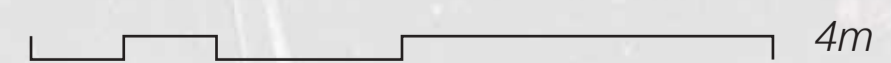
Buyers are shocked with the apparition from the water pulled by crane.

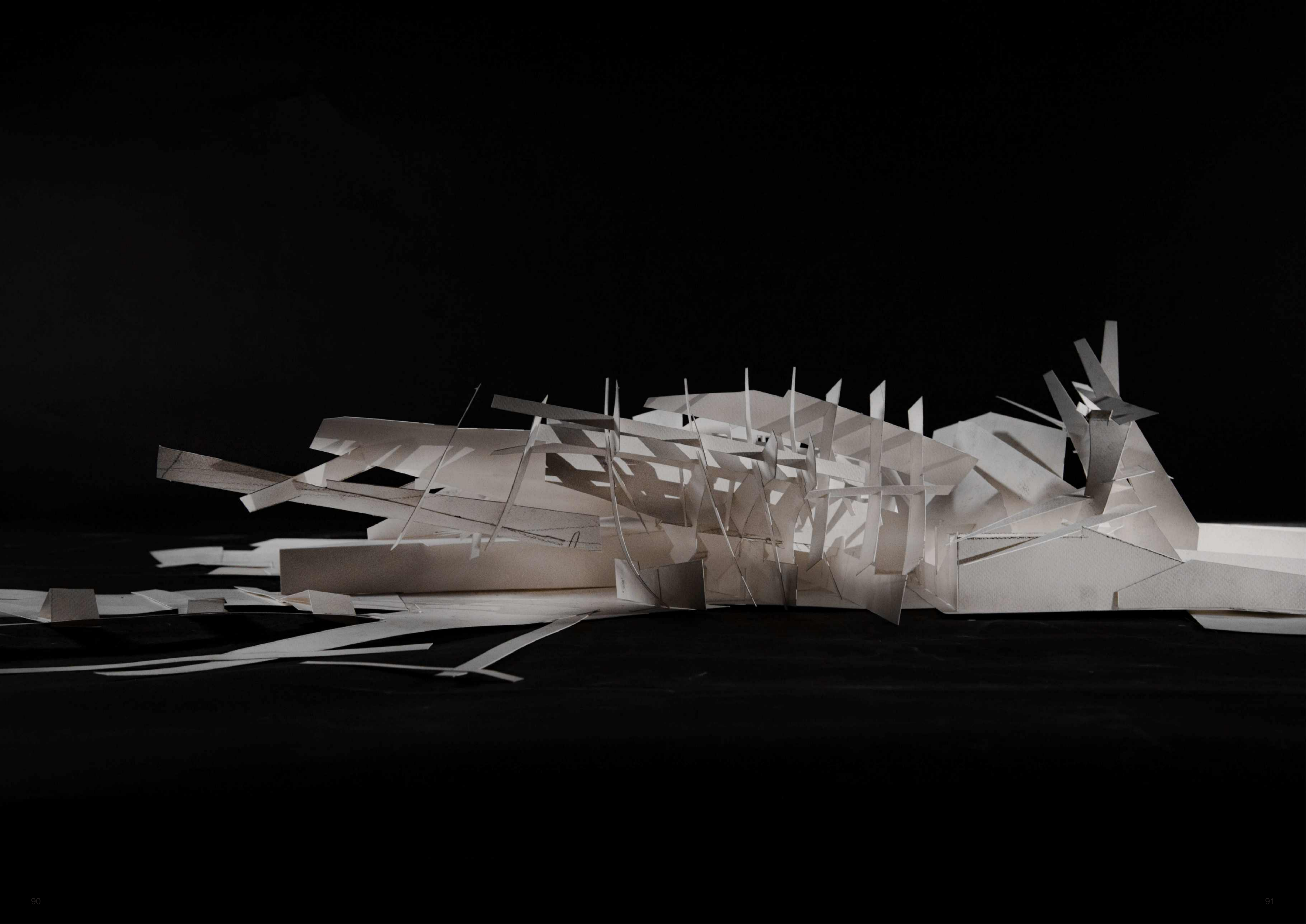
Artwork.

Auctioneer is ready to auction.

Art is carried in a submarine to have the sales done in a very fast and efficient way.

1:50 on A1





Mudlarking

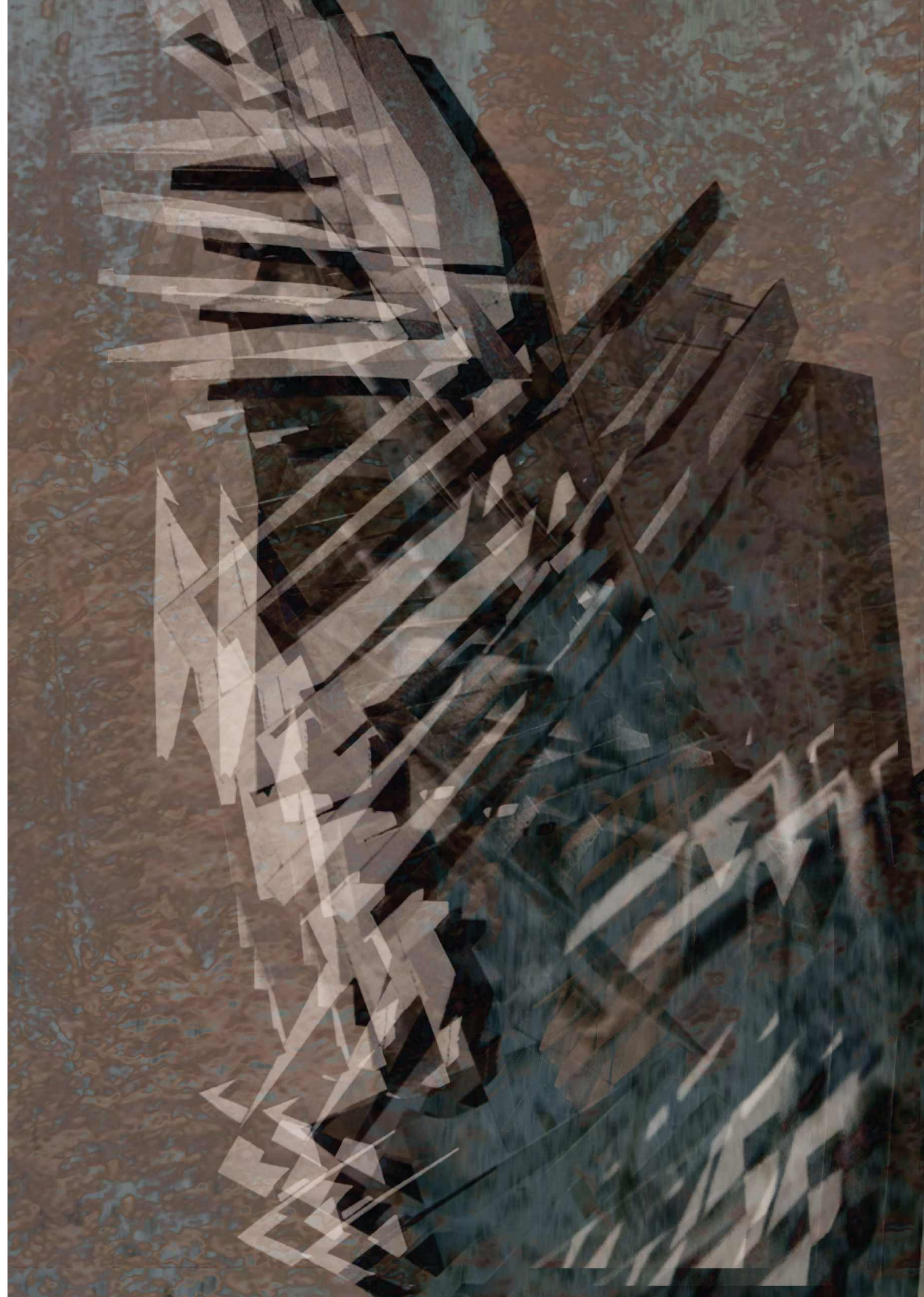
The action of scavenging in the river mud in search of valuable items that can then be sold (at the auction house). Was commonly used in the 18th and 19th cent-reappeared in the year 2050 as most of the river banks got covered in mud.

Thames Artefacts

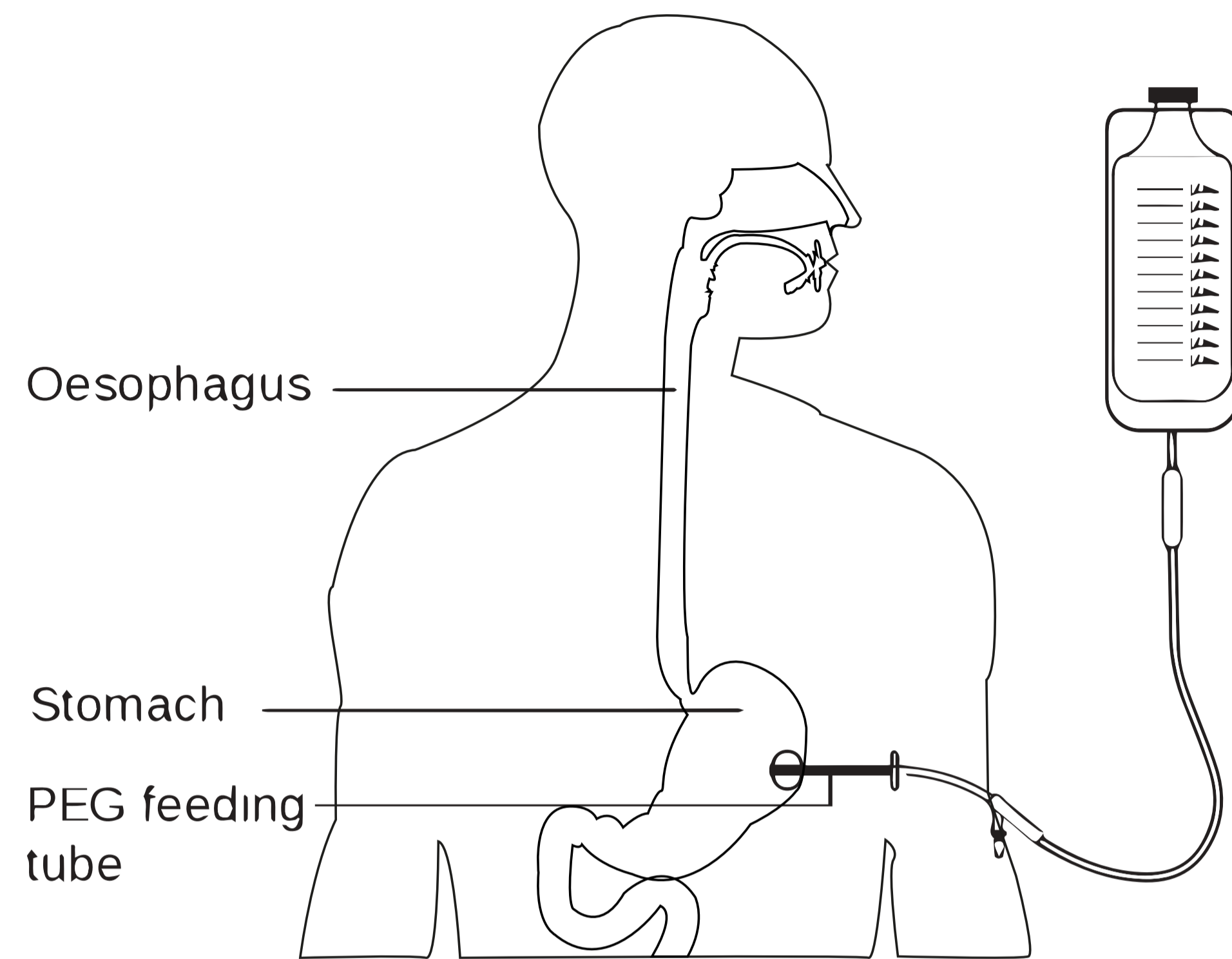
Feeding Tube

A small architectural project that can then be upscaled and metamorphose into a building proposal.

2050:
London river banks are flooded
People are used to feeding tubes
and only eat liquid food.



PEG: Maintaining Life



The PEG tube bypasses the oesophagus by inserting directly into the stomach.

Indications

PEG feeding tubes are recommended for patients who have difficulty swallowing. This could be as a result of:
 head and neck cancer
 neurological disorders
 stroke

It is also sometimes used for those who are severely malnourished.

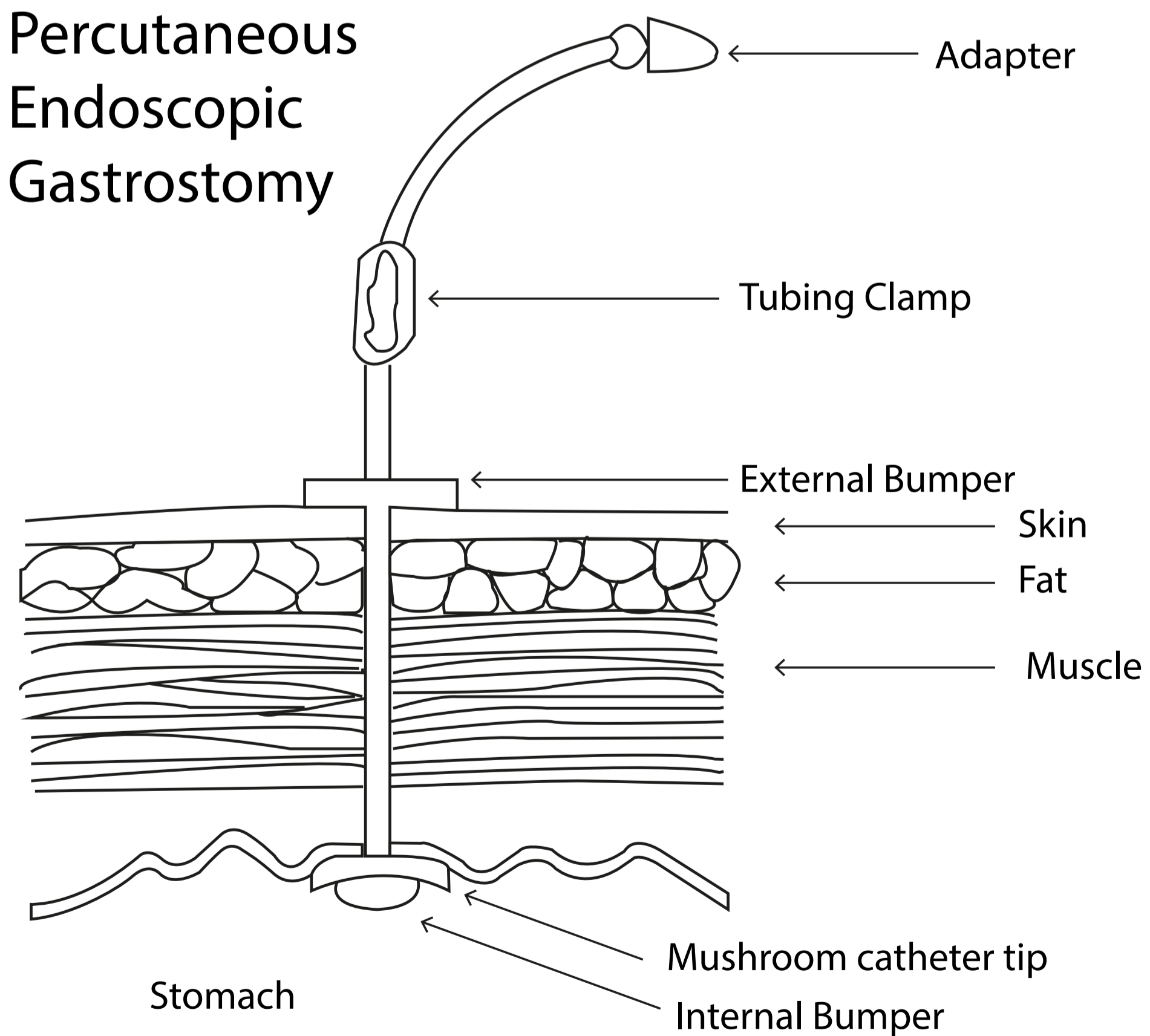
Procedure

A surgeon or gastroenterologist inserts an endoscope through the mouth down to the stomach. The surgeon then makes a small incision in the abdominal wall at the appropriate location, and threads a guidewire through the endoscope and out of the incision. The endoscope is carefully removed, and then the internal portion of the PEG tube is threaded over the guidewire. The guidewire is removed and the PEG tube is externally fixed.

Benefits

As compared with other types of feeding tubes, the PEG feeding tube is more comfortable for the patient, reduces the risk of aspiration pneumonia, and allows for greater mobility. Though the tube must be replaced every year, it can be a long-term solution for those who cannot feed orally.

Percutaneous Endoscopic Gastrostomy



The PEG tube needs to have both external and internal bumpers to secure the tube in place.

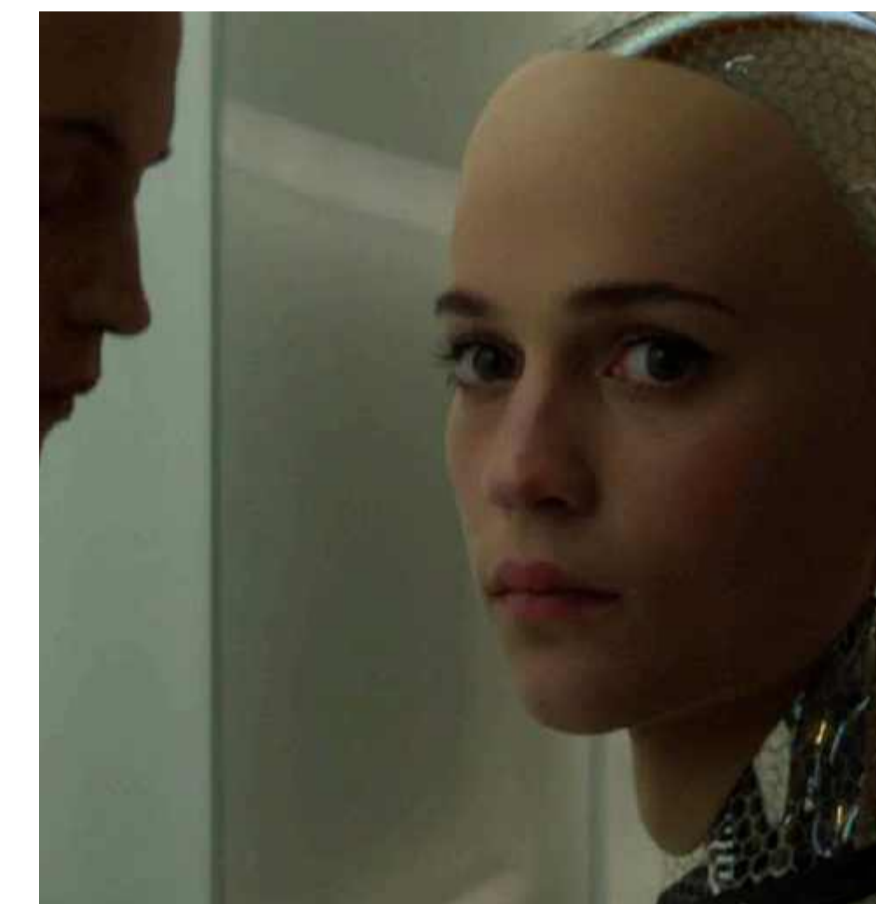
Conceptual Research



Pre columbian gold jewelry



Sterilac body and machine



Ex-Machina



Joint ceramics transplant



Bio ceramics



Roman bronze amulet

Mudlarking: Thames Artefacts



1.

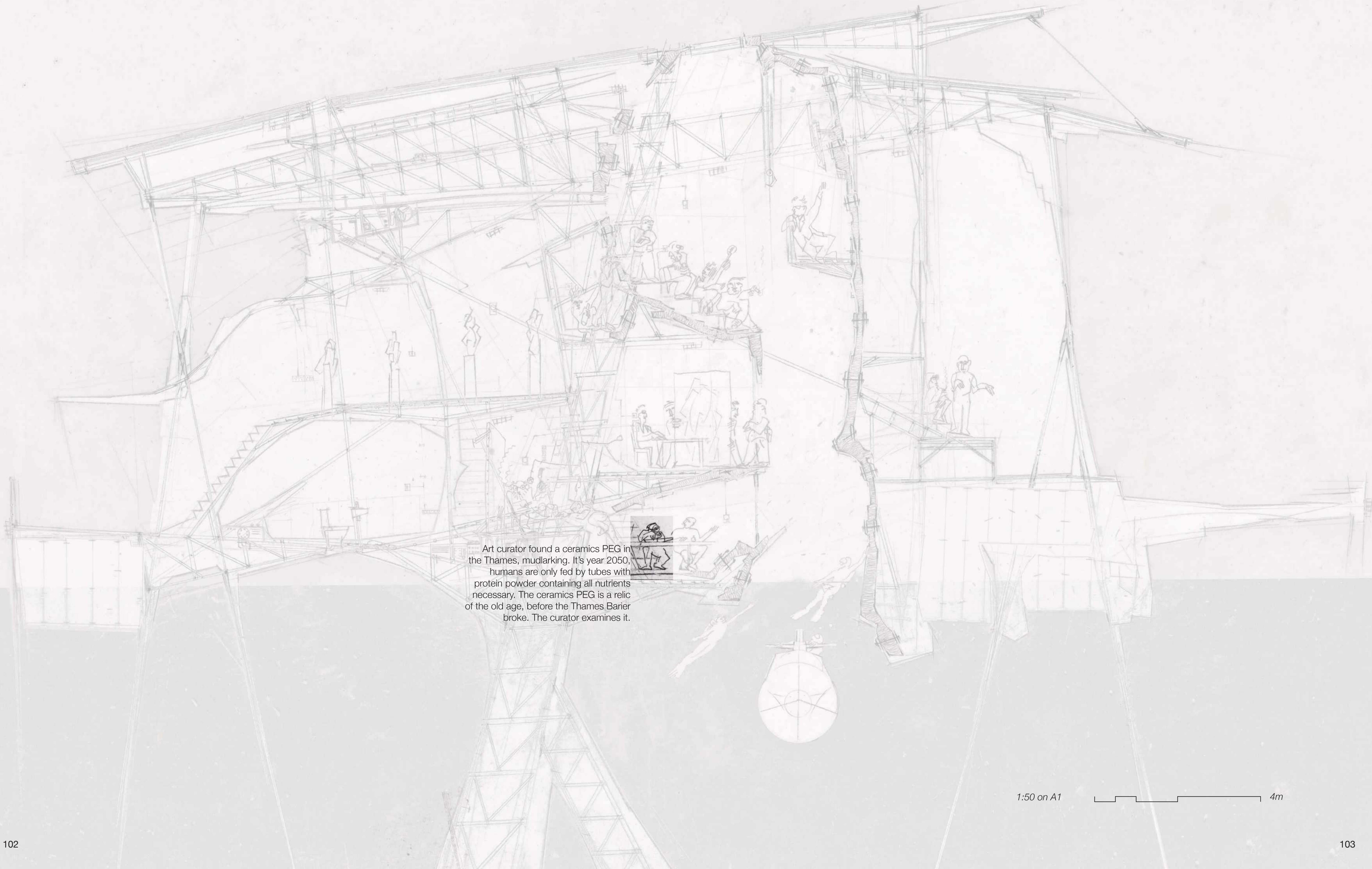
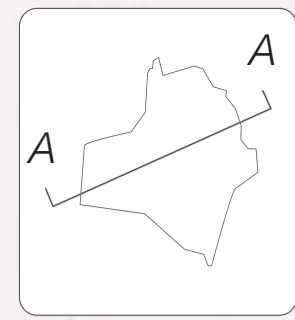
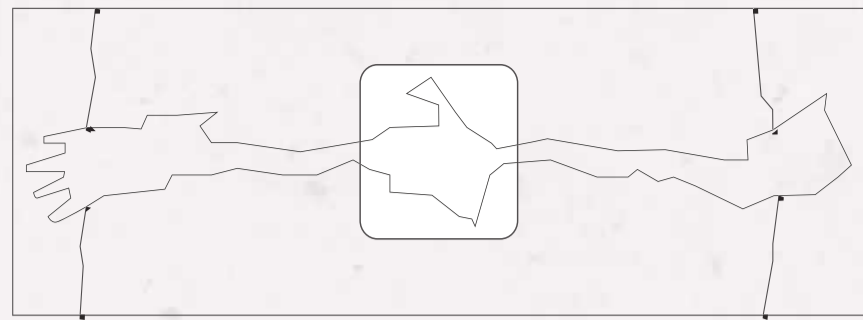
3.

2.

4.

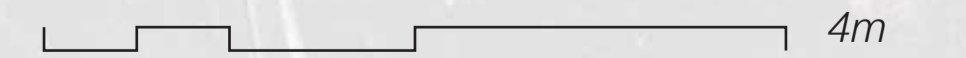
- 1-clay pipe
- 2-ceramic broken pieces
- 3-glass medicine bottle
- 4-Steel coin

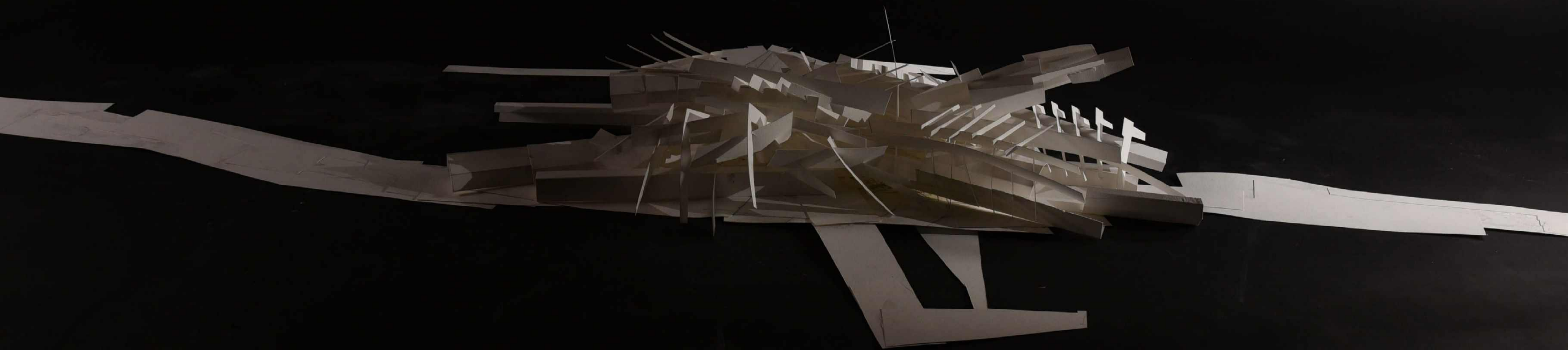




Art curator found a ceramics PEG in the Thames, mudlarking. It's year 2050, humans are only fed by tubes with protein powder containing all nutrients necessary. The ceramics PEG is a relic of the old age, before the Thames Barrier broke. The curator examines it.

1:50 on A1

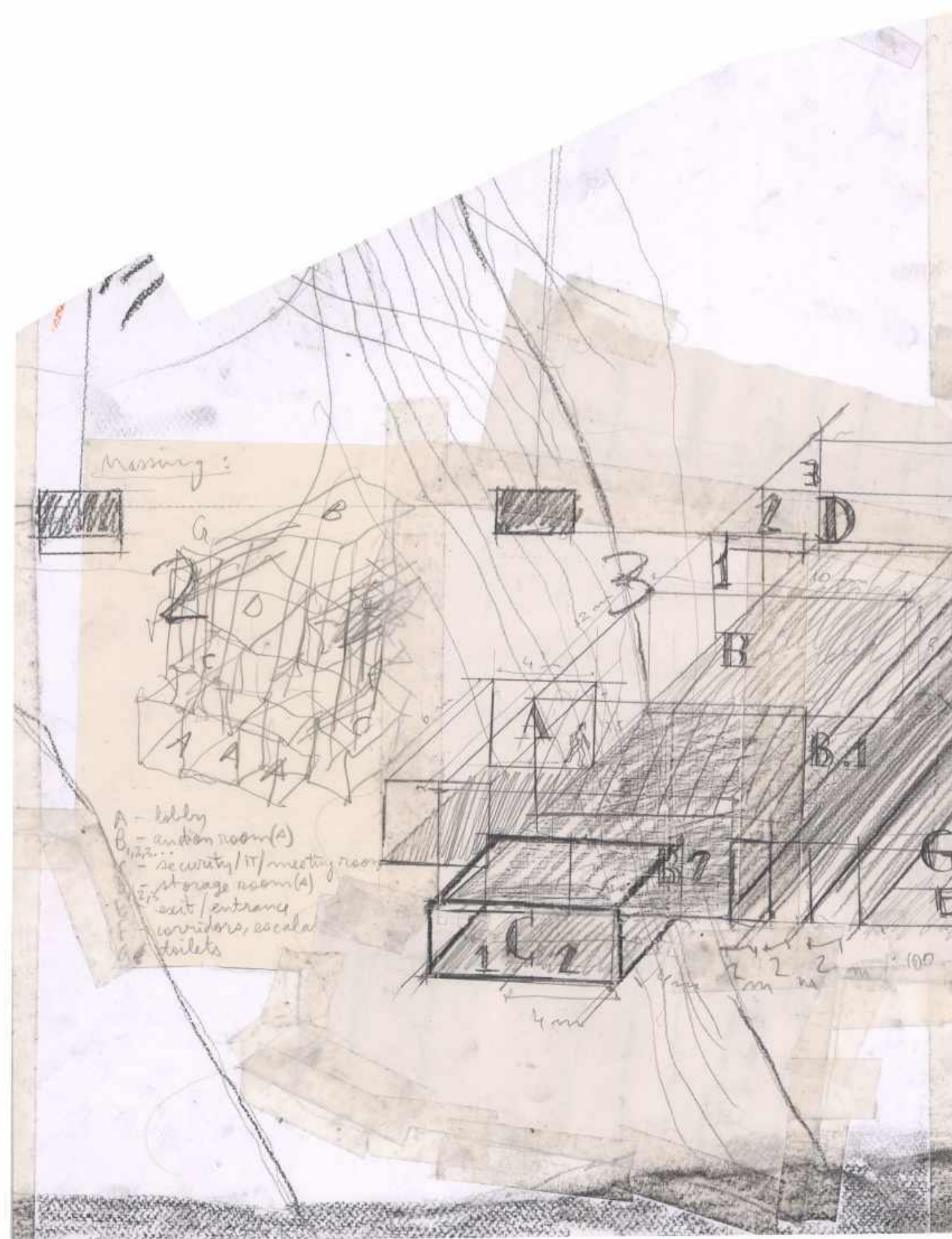
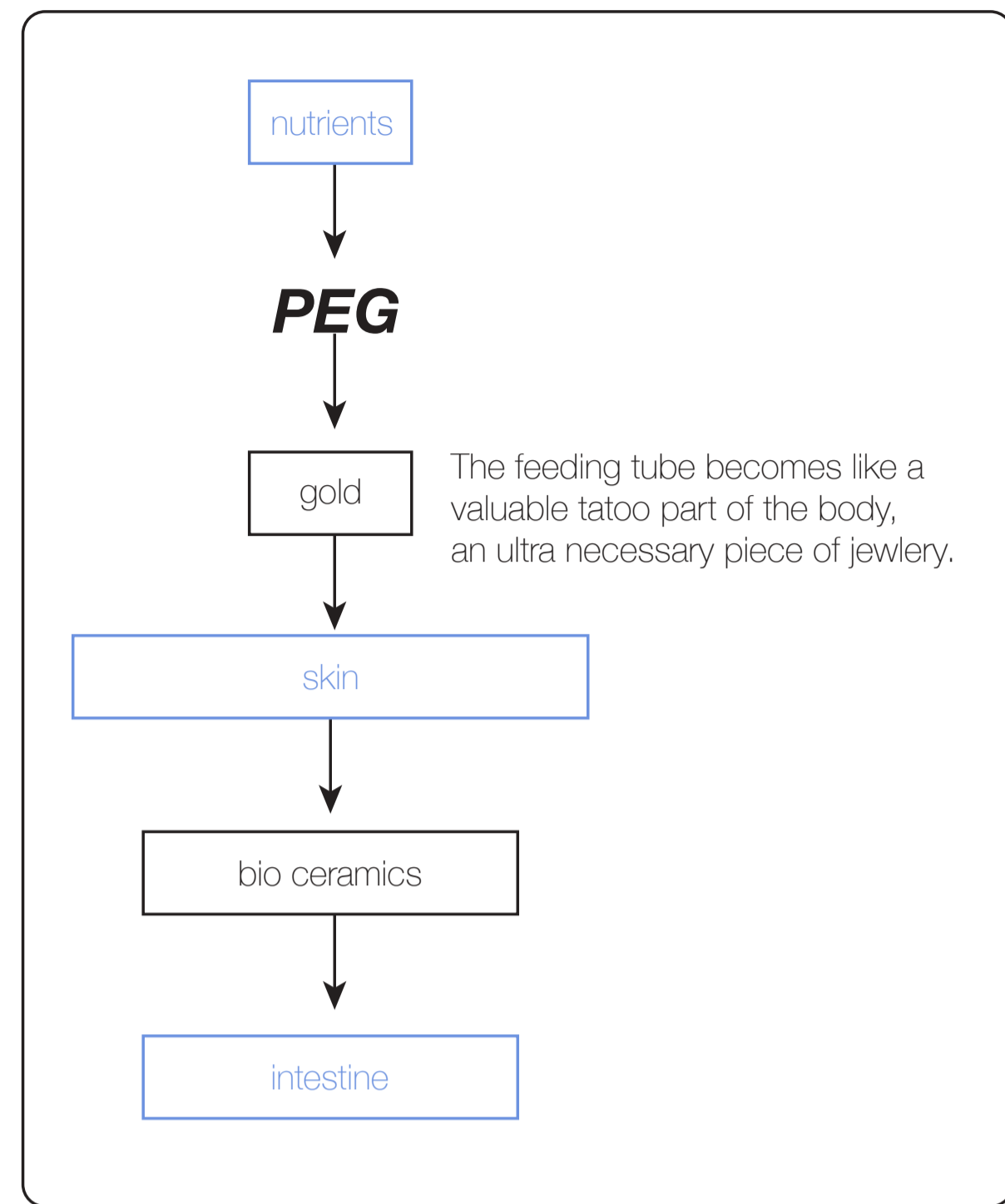




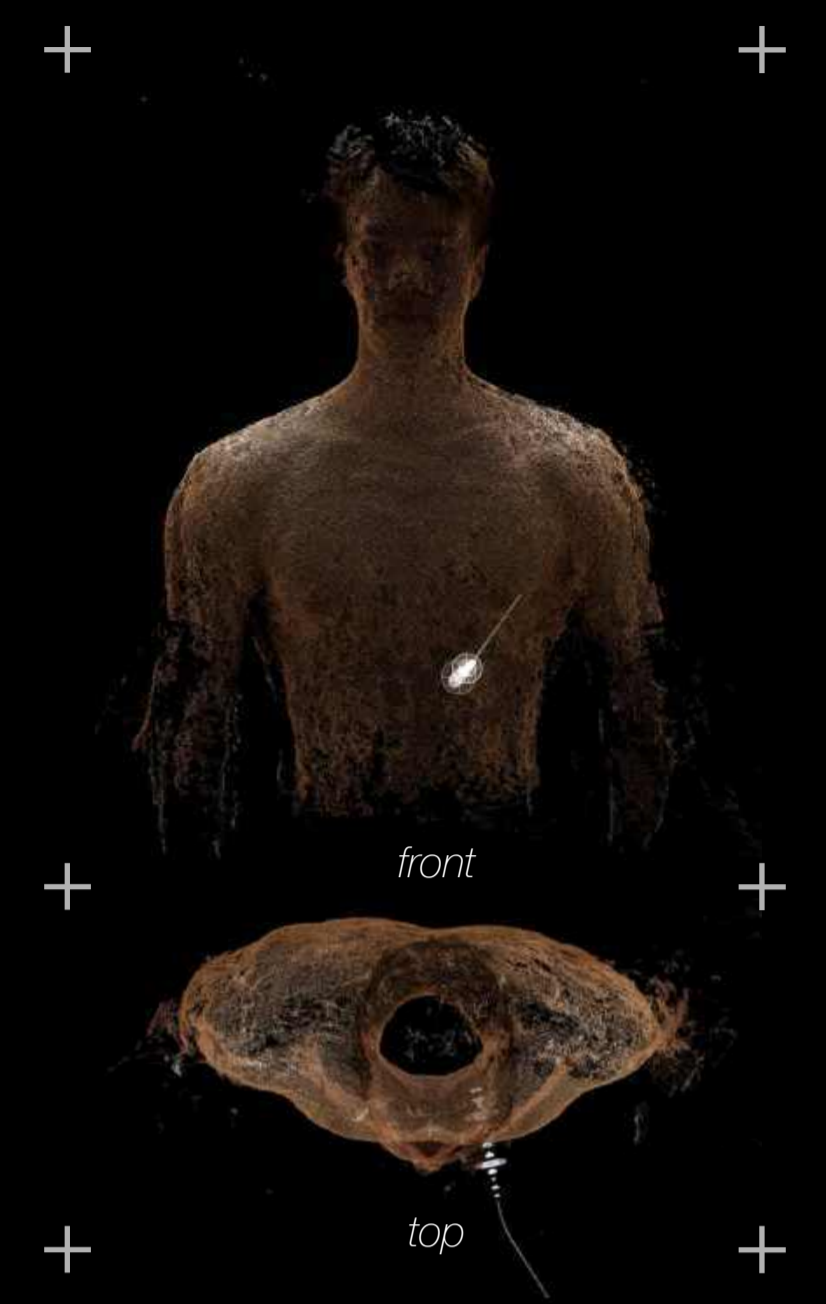


Section 6. Experiment and Concept Design: PEG

I had a lot of experiments with the design of PEG, and kept looking for different ways to make the design both practical and aesthetical. The goal remained the same, how to transform a medical device such as PEG into something beautiful and desirable. A design created to be solely practical can make us forget that sometimes humans desire a comforting touch that can be best bought by looking at the beautiful. In a world that is increasingly becoming more technological, it is vital that we do not lose ourselves and that we are able to retain our identities. Inspired by my upbringing within the Steiner education which aimed at students developing a creative self through a variety of disciplinary subjects – such as gardening, dancing, singing, drawing and wood-sculpting – I have remained attached to the idea that we should have beautiful designs. Beauty appeases worries, and in transforming a medical device into something that fosters reassurance, and not fear, it can help appease a patient's fear.







ABDOMINAL AREA V2 with FEEDING TUBE

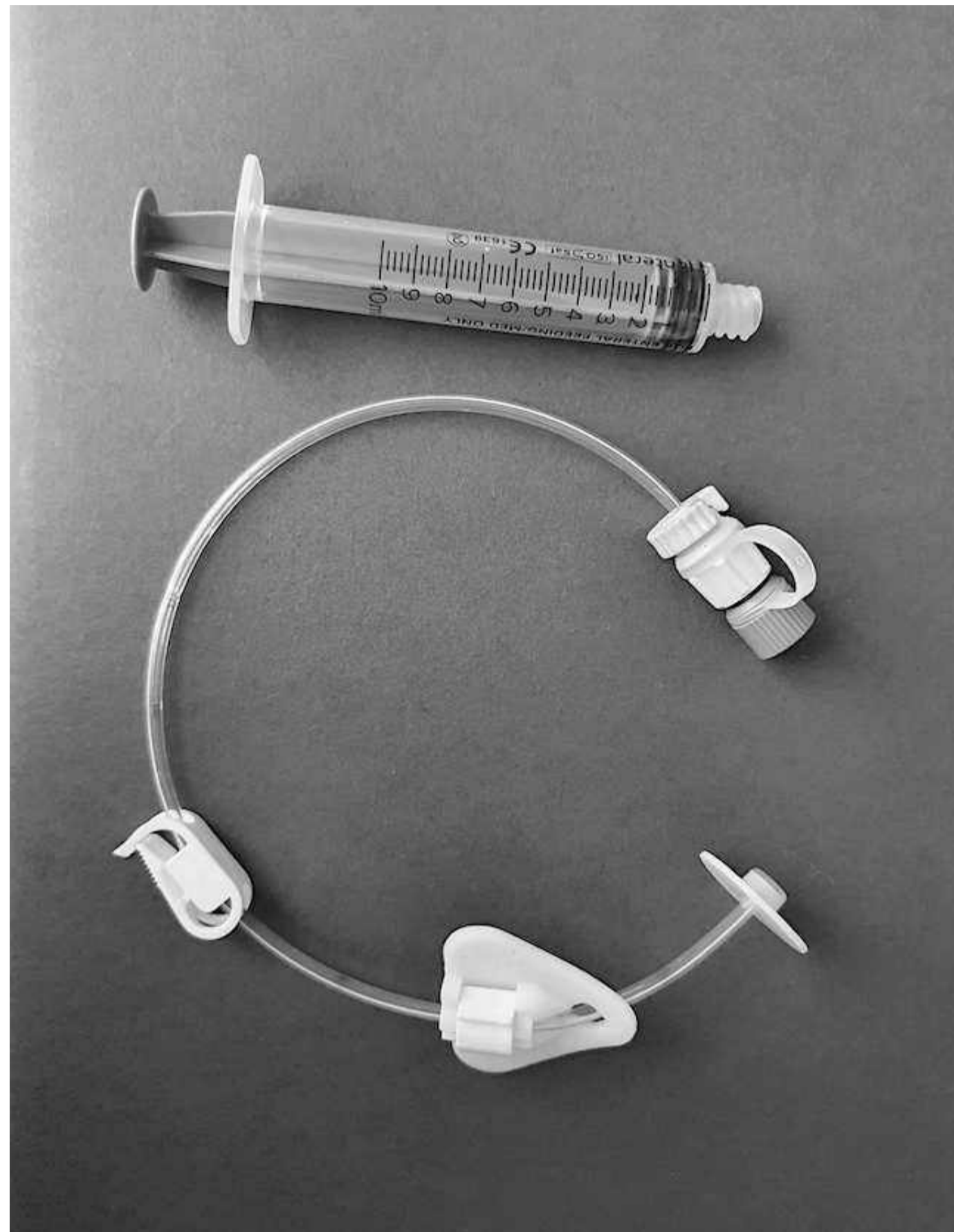


PEG Analysis and Redesign

STANDARD PEG VERSION

Material Sustainability

Standard PEG tubes are produced using a mix of medical-grade polyurethane (PU) and silicone. The extraction and the processing of petroleum used to manufacture PU is both carbon-intensive and can lead to habitat destruction. Similarly, silicon needs to be extracted from sand deposits and processed into silicone via energy-intensive chemical reactions. These environmental effects are compounded by the fact that PEG tubes have to be replaced every 6-12 months. It is difficult to recycle medical-grade PU and silicone because of contamination risks.

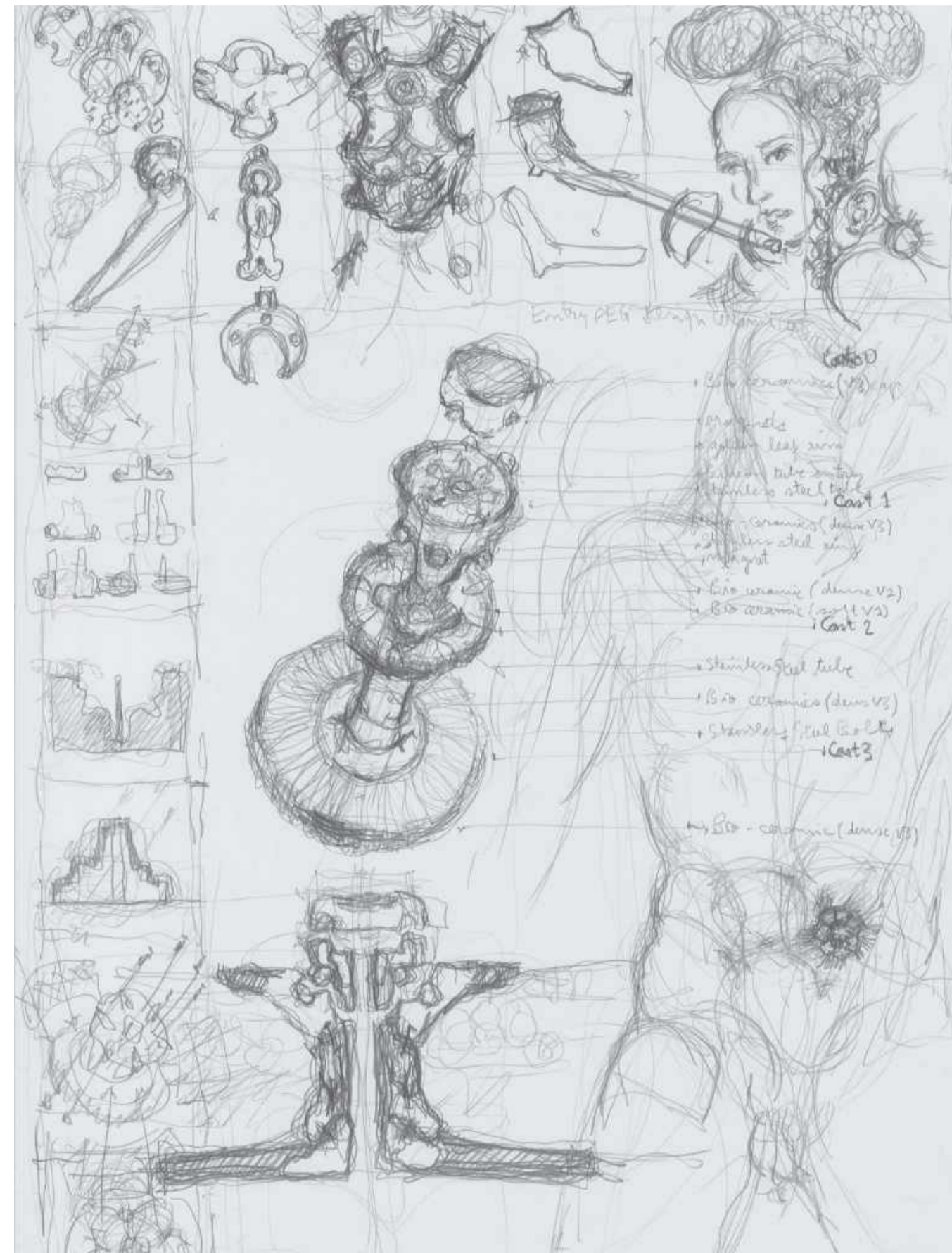


Previous PEG design, using polyurethane and silicone.

Aesthetics

"The truth is a feeding tube makes you self-conscious when you go out."
- Tayarra Smith, via feedingtubeaware.com.au

Many who wear feeding tubes face psychological distress from external judgement. Unfortunately, tube feeding in public continues to be viewed with emotions ranging from curiosity to disgust by observers.



PEG redesign, using ceramics

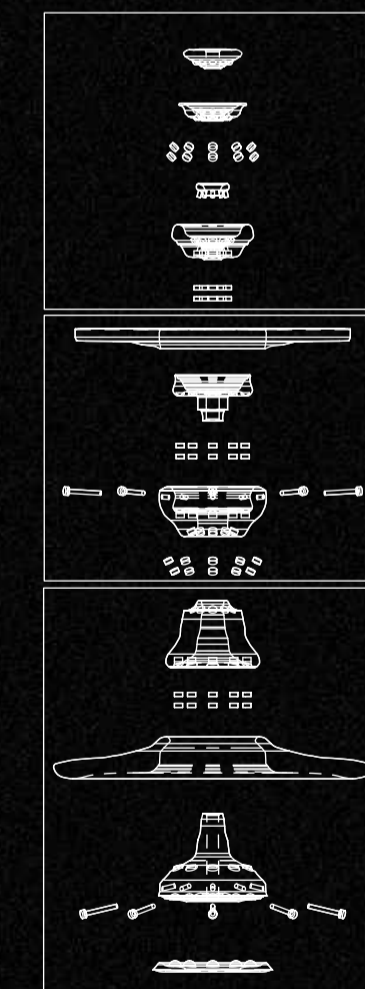
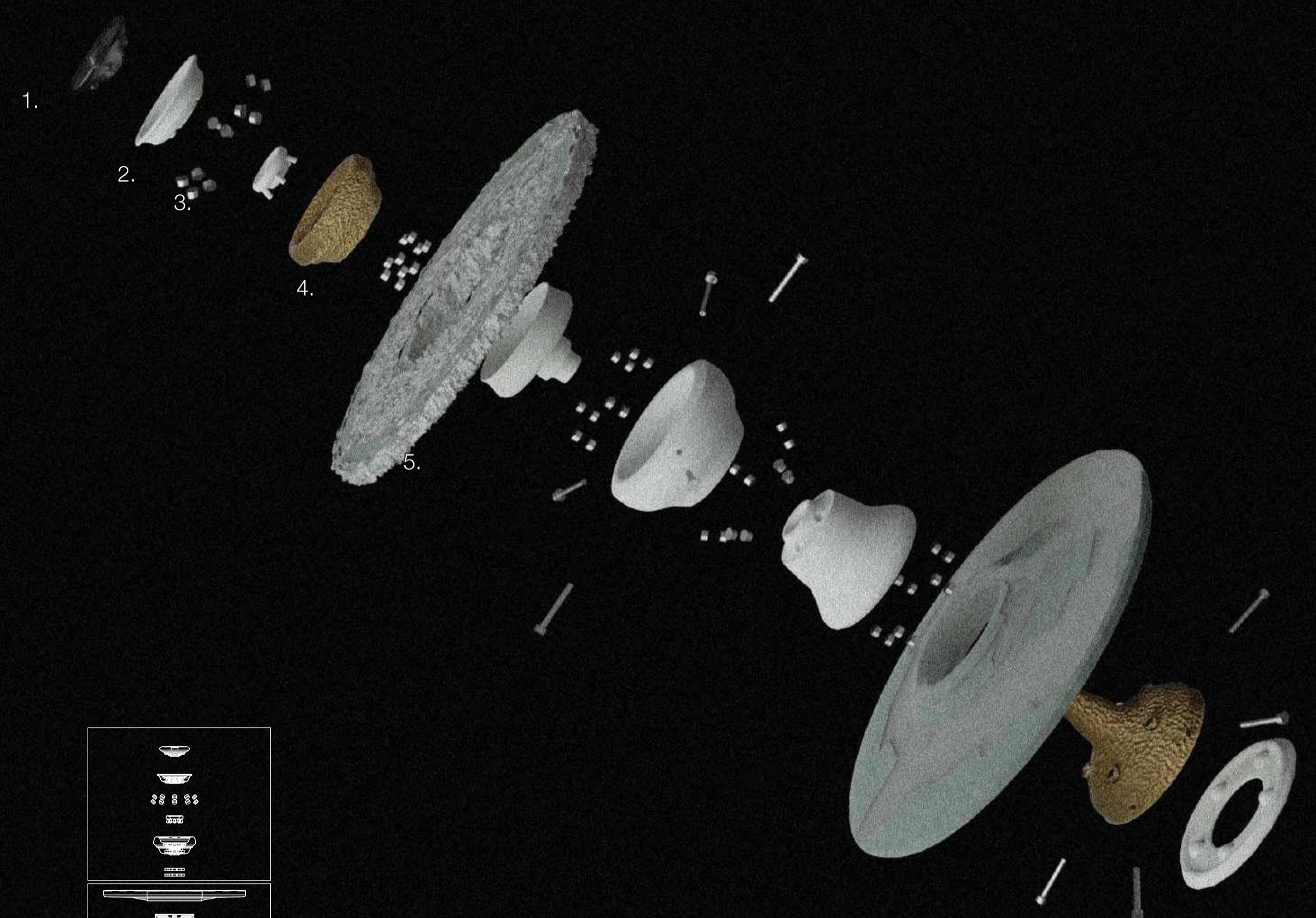
SUSTAINABLE PEG VERSION

Material Sustainability

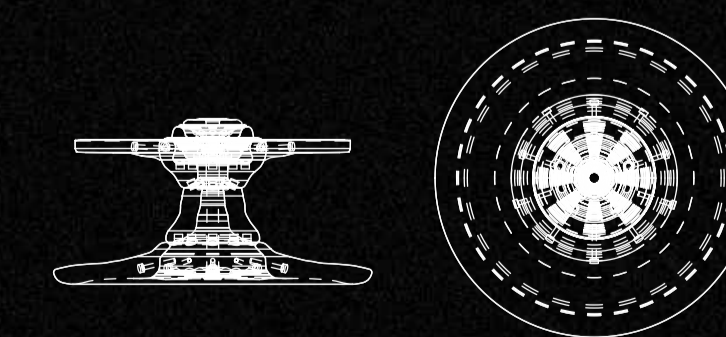
Bio-ceramics were selected as a sustainable alternative to the use of petroleum-based products like polyurethane. Materials used in the PEG tube must be bio-compatible and easily sterilizable. Fortunately, bio-ceramics are stable in tissue and pose a low risk of infection. They are already widely used in dentistry, tissue engineering, and drug delivery. Some forms of bio-ceramics, such as alumina and zirconia are non-resorbable and can remain in the body for years. At the end of their life-time, bio-ceramics can be ground, milled, and processed for use as a powder in industrial applications.

Aesthetics

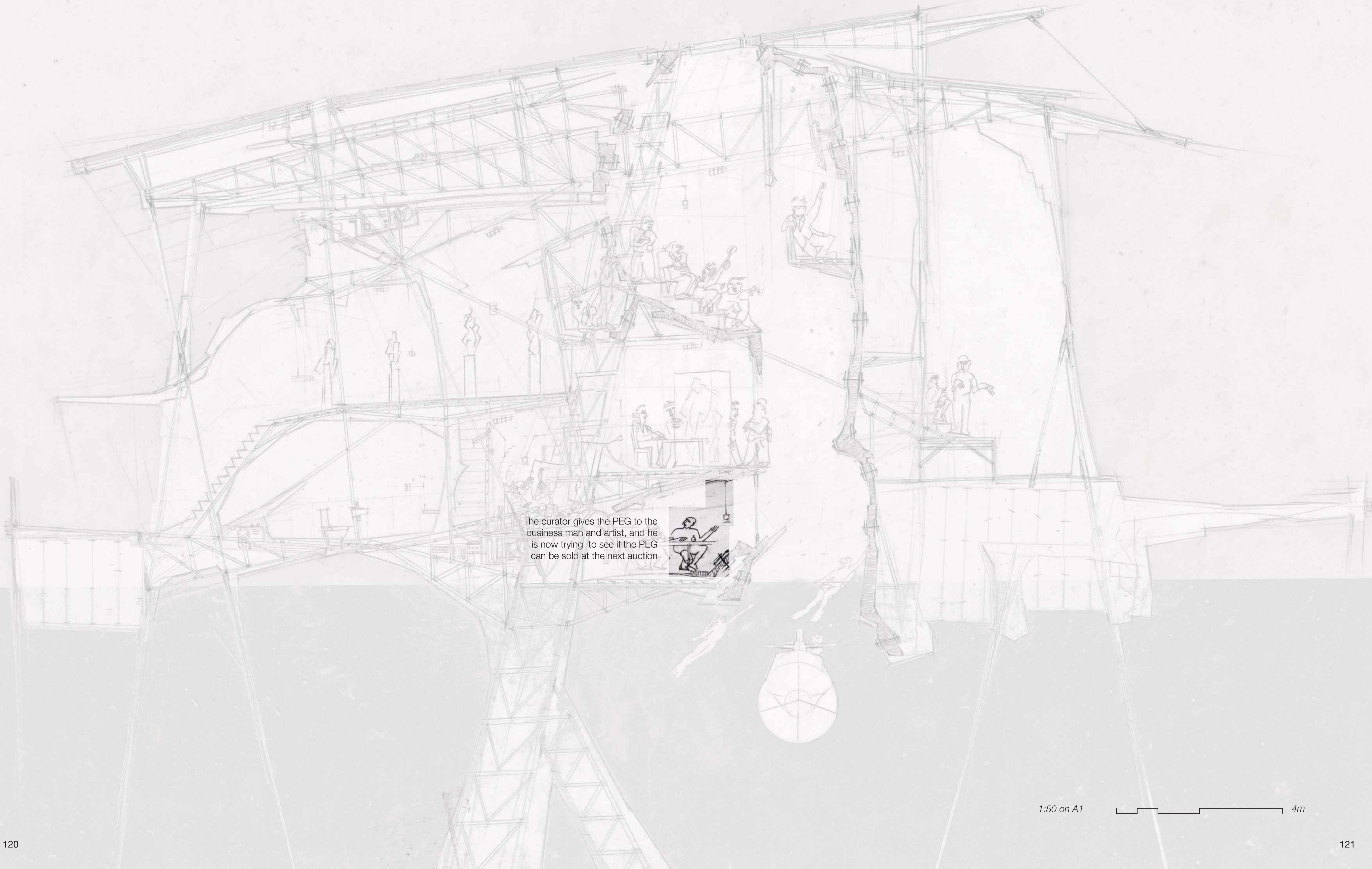
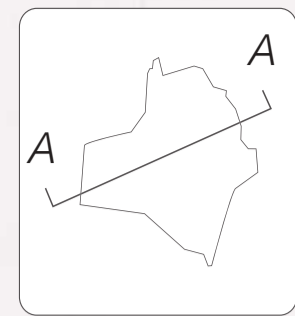
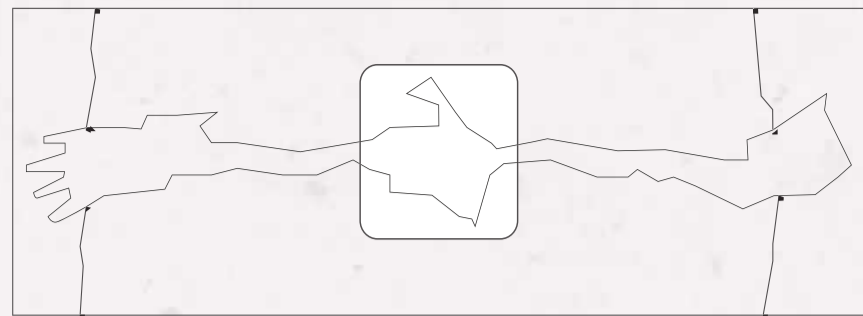
Bio-ceramics can be aesthetically customized to the preference of the patient. For example, the external fixations of the PEG tube can be matched to skin tone -- or alternatively created with specific color to provide joy to the patient. These additions could reinvent the feeding tube as a natural extension of the body, and provide patients with agency that help them to gain confidence with their feeding tube.



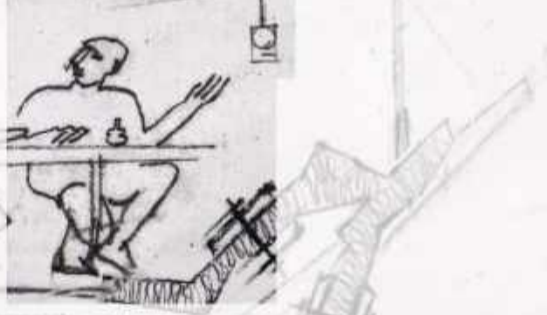
- 1-silicon replaceable entry point
- 2-hardened bio ceramics
- 3-stainless steel joints
- 4-gold (antioxidant)
- 5-bio ceramics



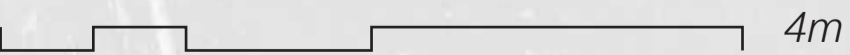




The curator gives the PEG to the business man and artist, and he is now trying to see if the PEG can be sold at the next auction.



1:50 on A1



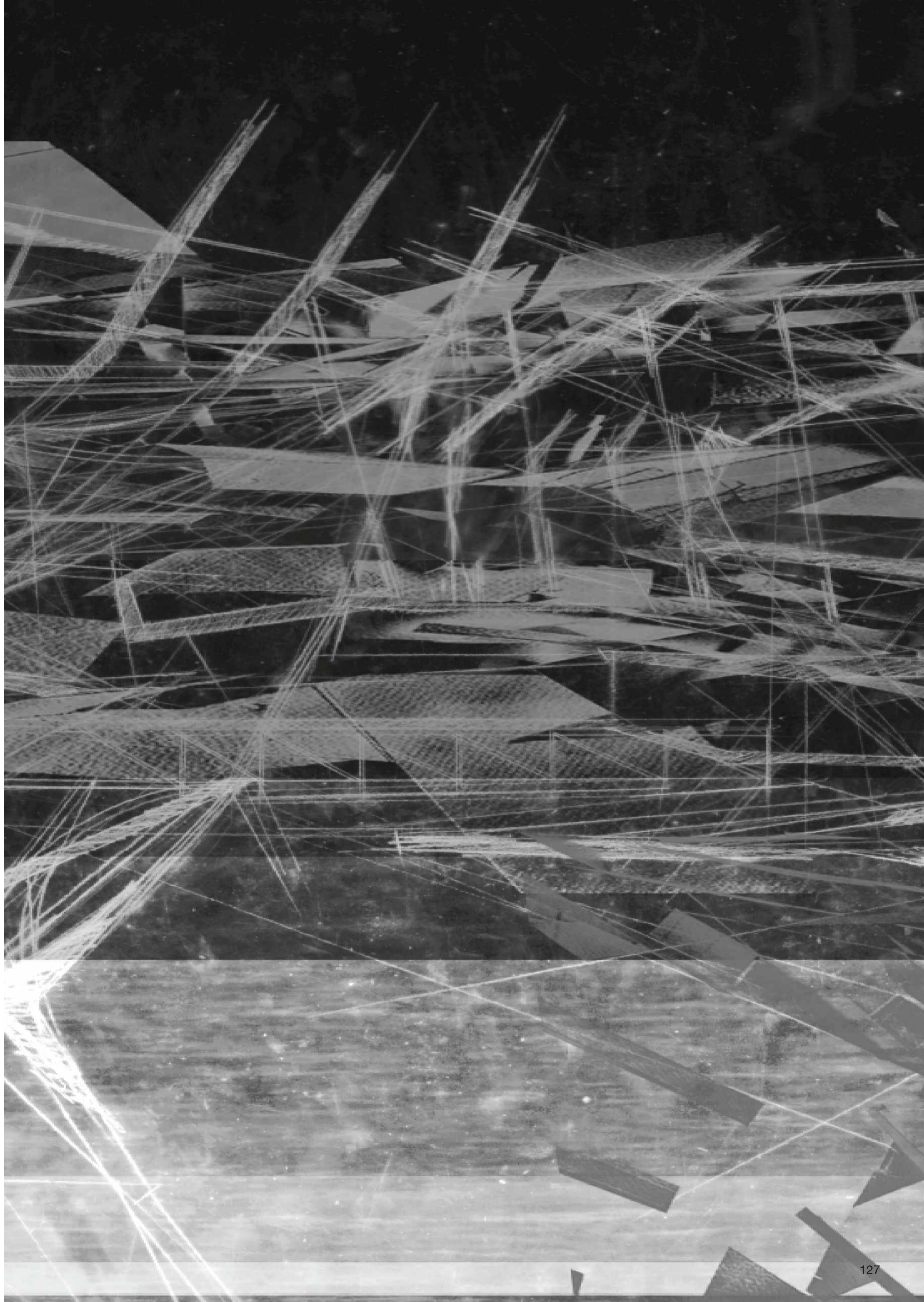
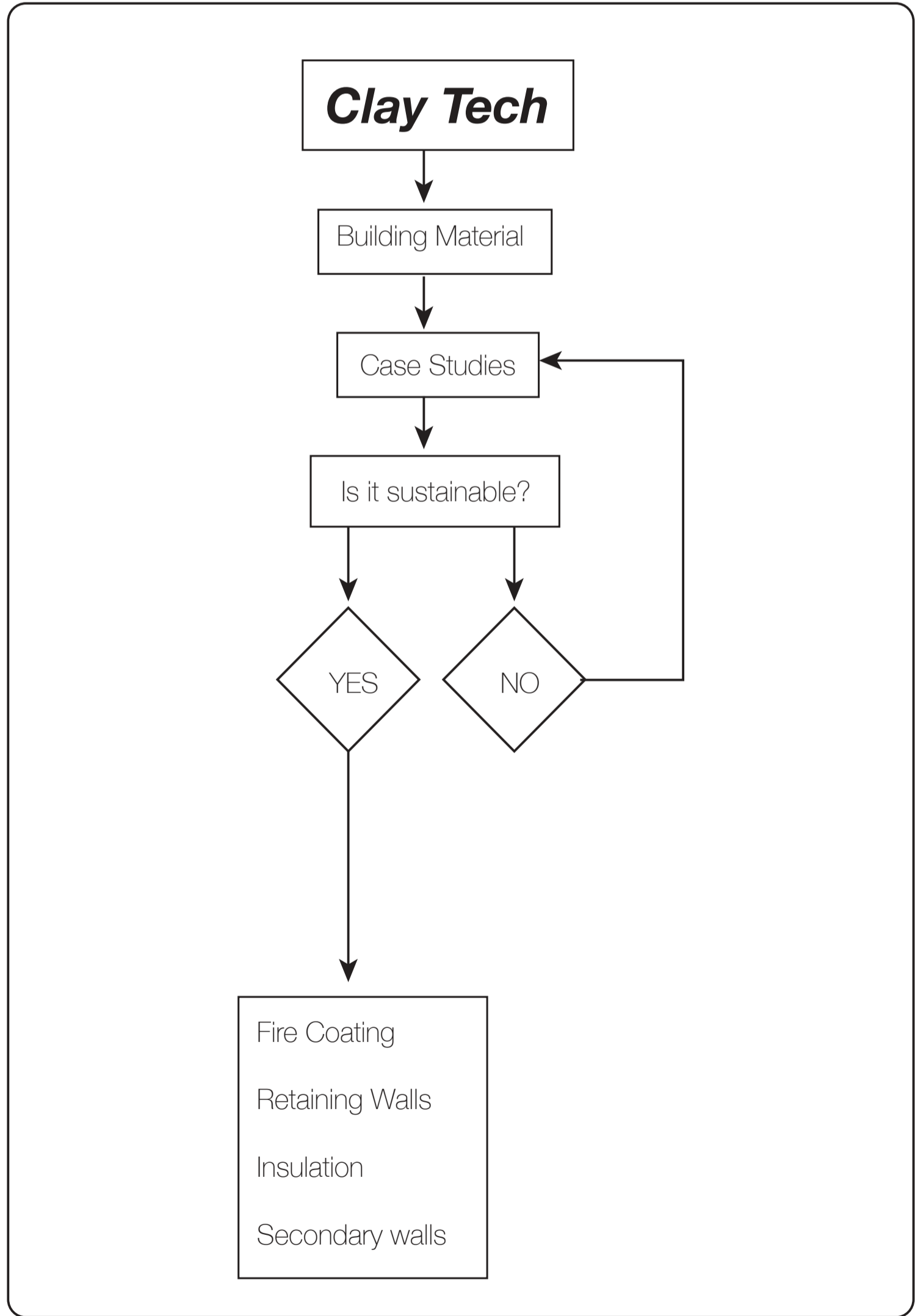




Section 7. Technical Investigation: Clay Tech

Naturally, when I wanted the building to use a significant amount of clay to build the auction house, the effusive term 'clay' does not suffice. One must be specific, which is what I have strived to do in section 7, carrying out a technical investigation on clay. I researched on the different types of clay that could be used in pottery, construction and real-life instances where clay was applied as a material used for the creation of an architectural building. The university trip to Venice inspired me in this process as I photographed walls that used clay as a sacrificial layer, something that caught my eye.

Again, I have been quite fascinated by how we can use traditional materials, such as clay, to create modern constructions, re-adapting traditional architecture and learning from its techniques to create an innovative, new design. Clay as a building material has the capacity to be practical, beautiful and sustainable, something I think is very important for the success of an architectural building.



Clay as a Building Material

Pottery

The process to form pottery clay from natural clay is relatively consistent: the clay is harvested, screened to remove impurities, mixed with water to form a slurry or slip, and kneaded to evenly distribute clay particles. Once the mix settles, impurities are removed from the bottom and the water drains. The clay is then wedged to remove air bubbles.



Earthenware

From sedimentary rock and fired at low temperatures. Easy to work with and cheap, but porous, less durable, and not suitable for food.



Porcelain

Mined from kaolin deposits and fired at high temperatures. Very pure and water-resistant, but expensive and low workability.



Stoneware

Mined from weathered granite (which has fewer impurities) and fired at high temperatures. Durable and water-resistant, but less workability.



Terracotta

Contains iron-oxide which leads to the characteristic reddish-orange color. Grog (ground-up fired clay) is mixed in to provide workability. Inexpensive and low-firing, but fragile and porous.

Clay Powder

To form clay powder, natural clay is mined, cleaned, ground, and sieved to achieve constant particle size. This powder is then packaged for use in a variety of industries, including ceramics, construction and cosmetics. It often functions as a binder or absorbent. Some common clay powders are kaolin clay, bentonite clay, red clay, and stoneware clay.



Glazes

A clay powder is combined in a slurry of fluxes and mixes in order to improve surface adhesion and add color to an object.



Cosmetic Facial Masks

Clay powder is mixed with water and other extracts to form a paste. The clay powder helps to absorb and remove impurities and excess oil from the skin.



Cat Litter

Clay powder is treated with deodorizers and clumping agents. The powder helps to make the litter absorbent and easy to clean.

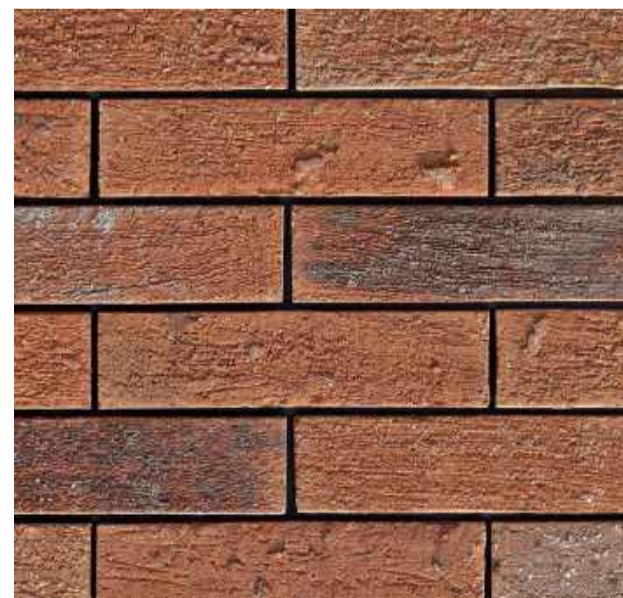


Paper

Clay powder is mixed with pigment and fillers, then applied to the surface of the paper to create a smooth surface. This helps with printing quality and improves strength and durability.

Construction

Clay's durability, sustainability, and insulation properties have made it a natural construction material for millennial. For construction, clay is usually ground and mixed with other materials (sand, straw, etc.) to lower its density and provide additional structure.



Bricks

Clay is crushed into small pieces and mixed with water and sand for texture and consistency. This mixture is then extruded to mold it into its final form.



Tiles

Clay is usually mined from a quarry and then is ground into a fine powder. This powder is mixed with feldspar, silica, and talc to improve its strength and durability. Different molding techniques produce complex shapes.



Clay Plaster

The plaster consists of a mixture of sand and water. Sometimes straw or other natural fiber is added to improve strength and texture. The plaster is applied in layers to interior walls and ceilings.



Cob Construction

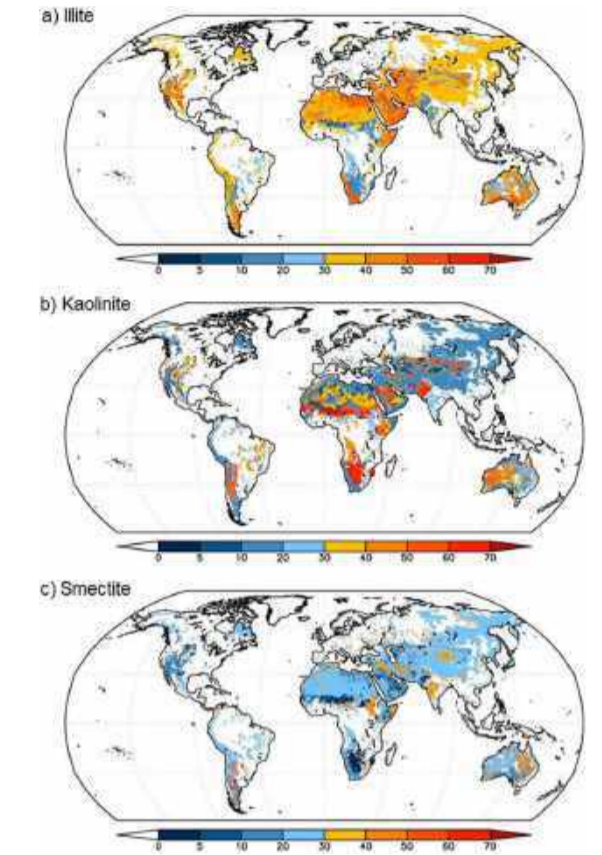
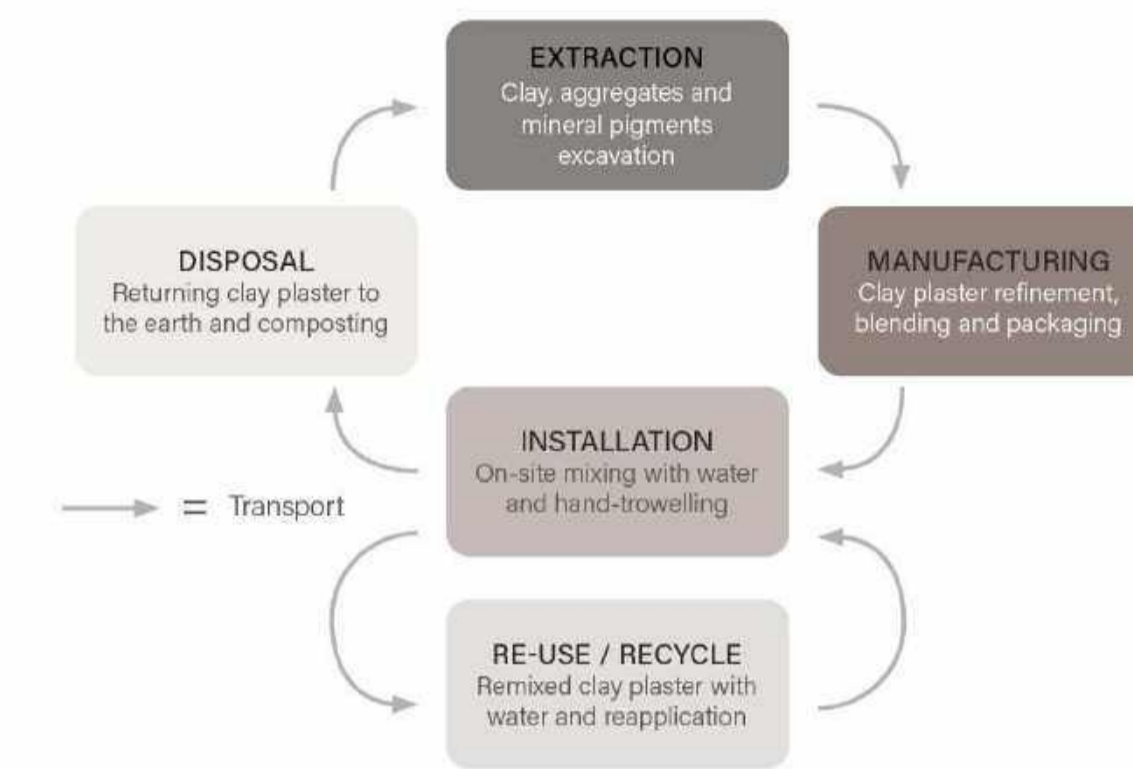
Traditional building material. 3-4 parts sand and 1 part natural clay is mixed by foot, and straw is added. The resulting mixture creates incredibly thick, durable walls.

Clay Sustainability

Advantages

Clay is a highly abundant natural occurring mineral. It is non-toxic and non-polluting. Clay has a low thermal conductivity, making it a very energy efficient building material. It is incredibly durable and long-lasting, and can be easily recycled when needed.

Compared to many other materials, clay produces relatively low emissions. Clay bricks, for example; emit 0.5 kg CO2 per kg of material, lower than both steel (1.7 kg CO2) and cement (0.9 kg CO2).



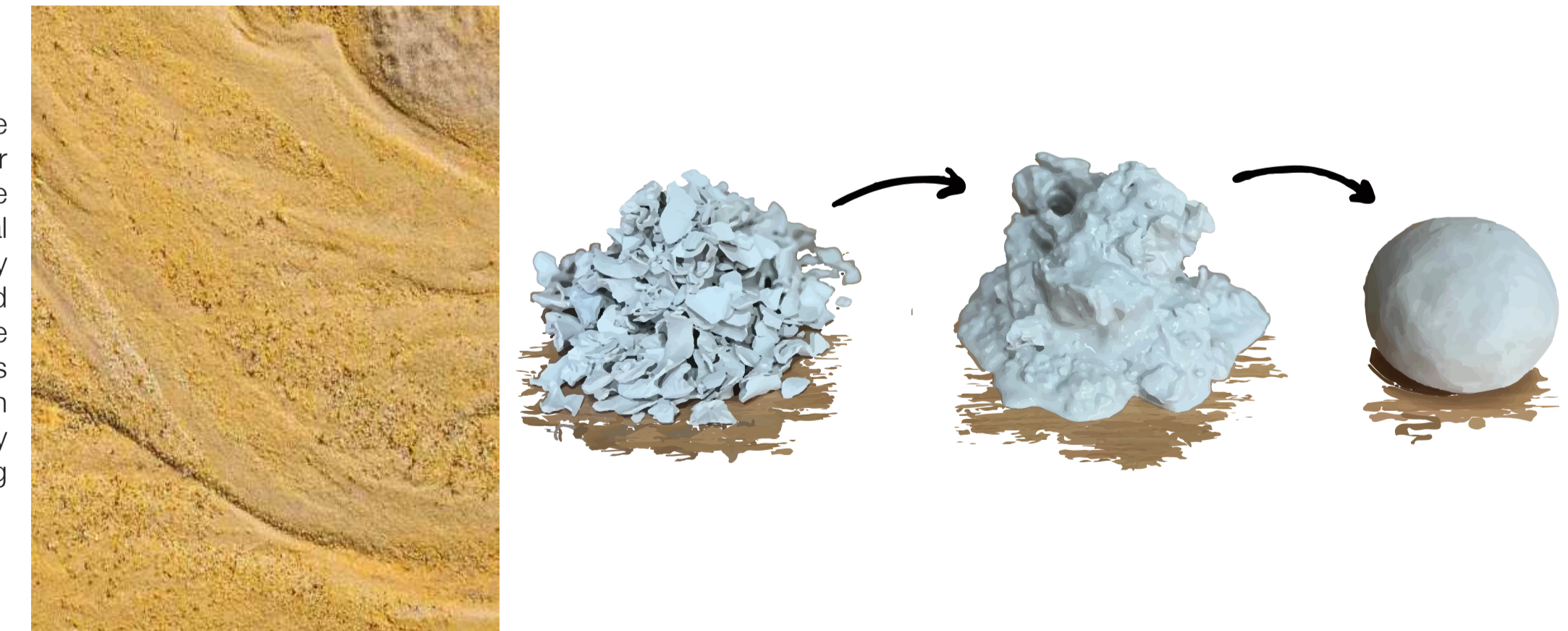
Disadvantages

Most of the environmental impacts of clay processing arise from mining emissions and firing emissions (covered in another section). Mining clay deposits leads to soil erosion and generates large amounts of waste. These tailings are the rocks, minerals, and other material extracted from the deposit but not needed for natural clay. These waste products are often toxic as a result of heavy metals, and can contaminate water and soil. Though mining clay consumes significantly less water and energy than other forms of mining, it still drains around 100-1000 gal/min of water.



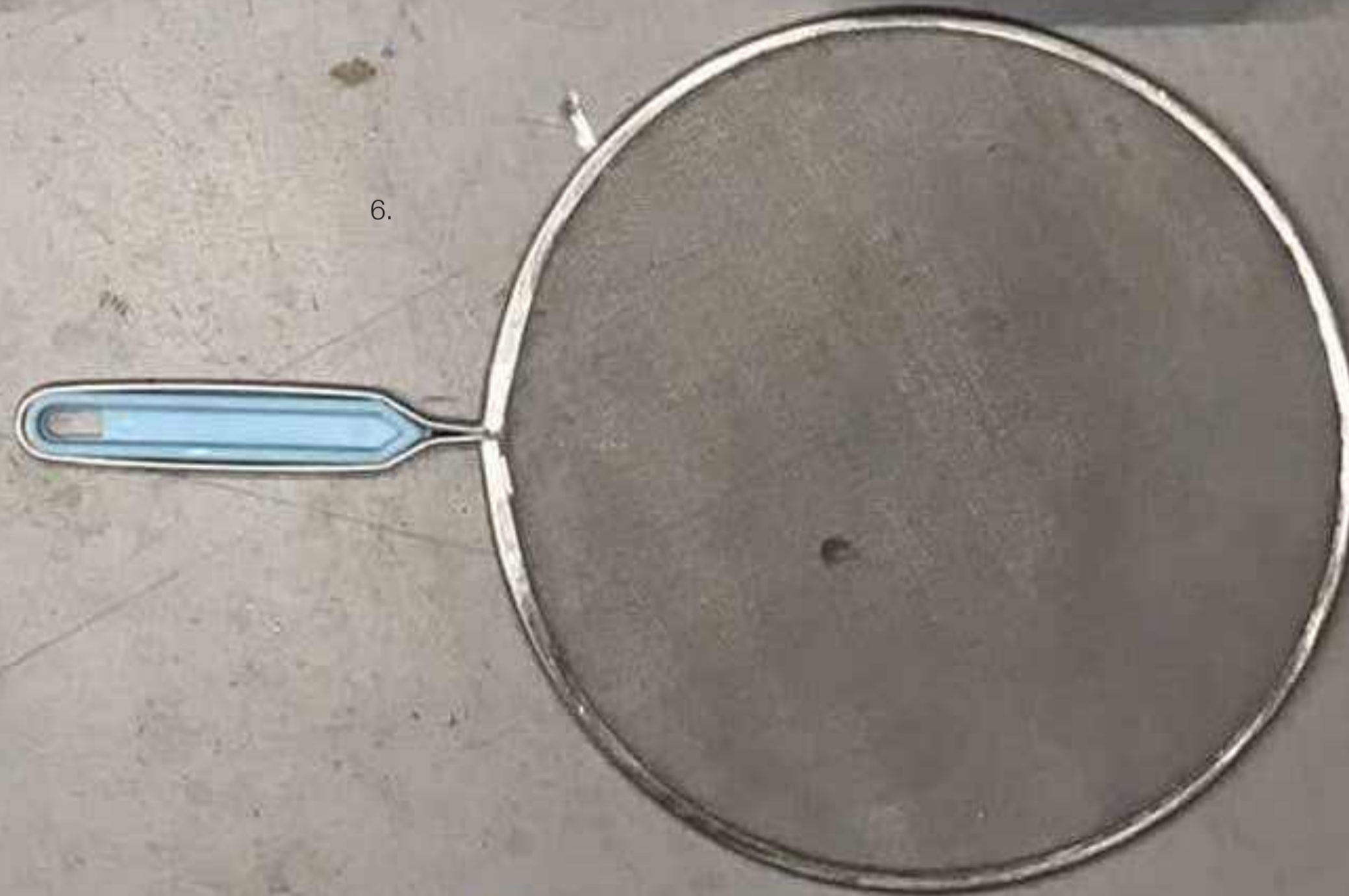
Improvements

Several steps can be taken to reduce emissions from clay. During mining, better waste management practices can be employed to decrease the environmental destruction caused by tailings. Energy efficient mining equipment (e.g. powered by renewables) can help to decrease energy use. To bypass mining emissions completely, it is important to reduce the need for clay as much as possible by reducing the density of clay and by recycling existing clay.



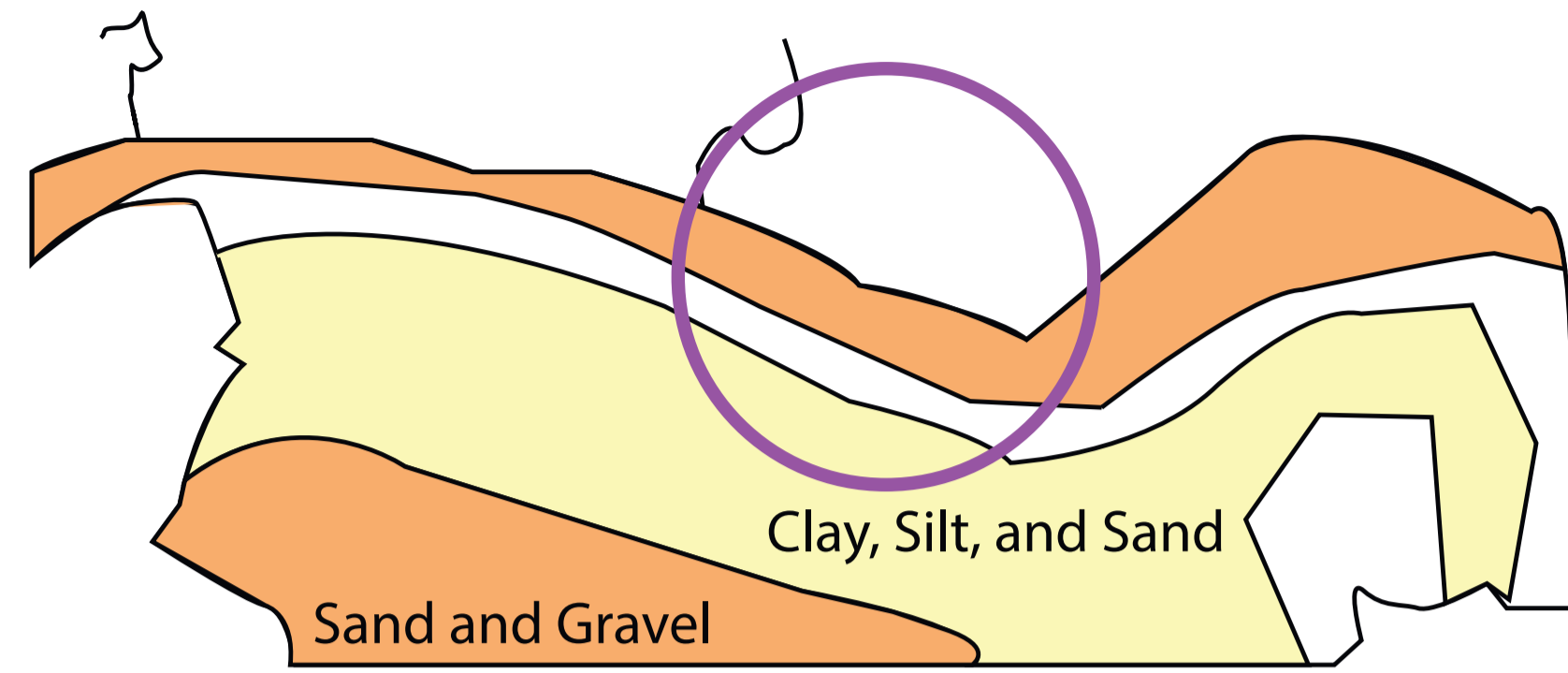
Summary

Overall, clay is a sustainable building material. Though clay mining can consume lots of water and energy and wreak environmental destruction, it is a durable, recyclable, and naturally-occurring material. Its emissions are significantly lower than many other building materials. However, this doesn't mean that we shouldn't improve its environmental impact by increasing recycling and decreasing mining waste and energy use.



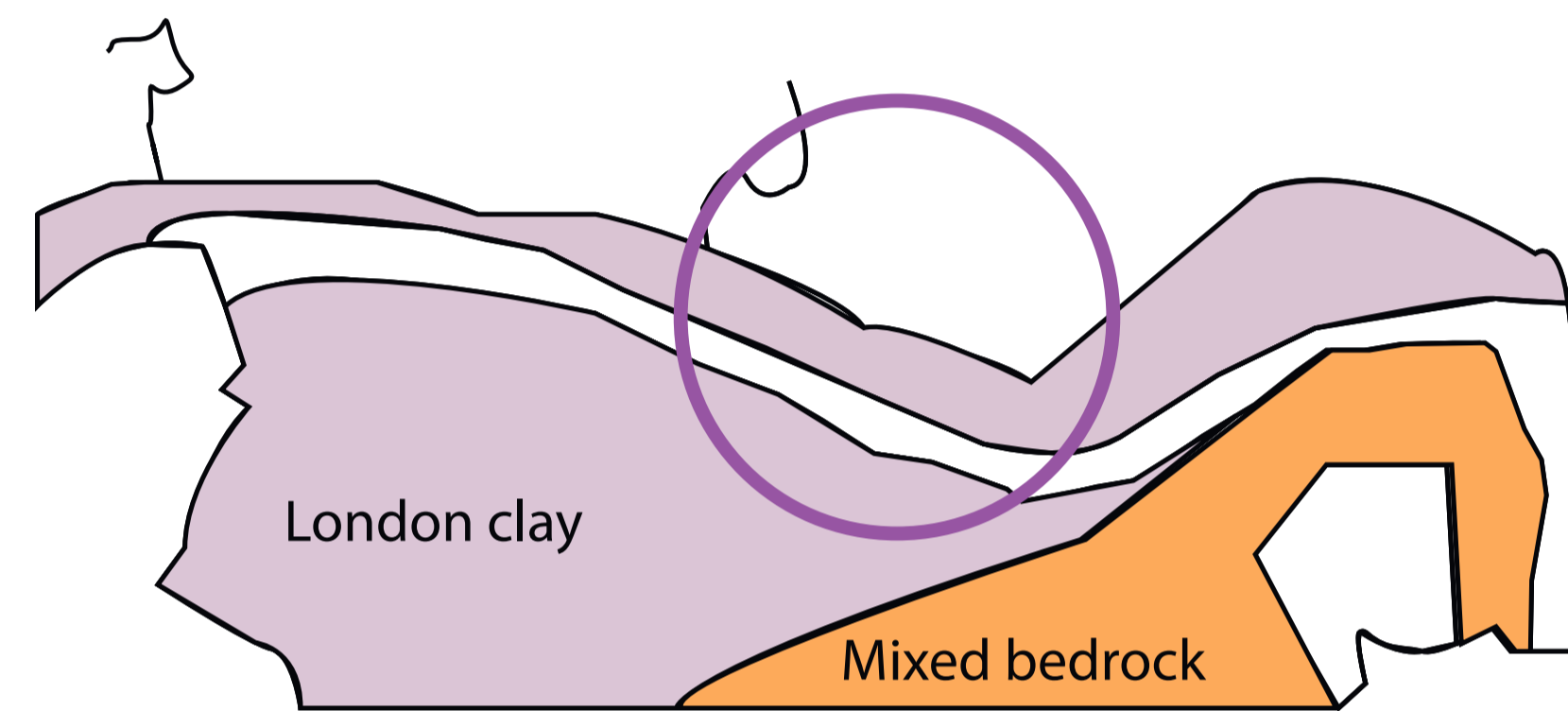
- 1-bucket
- 2-big siv
- 3-raw clay
- 4-steel hammer
- 5-rubber hammer
- 6-small siv

Specific Adaptations to Site Conditions



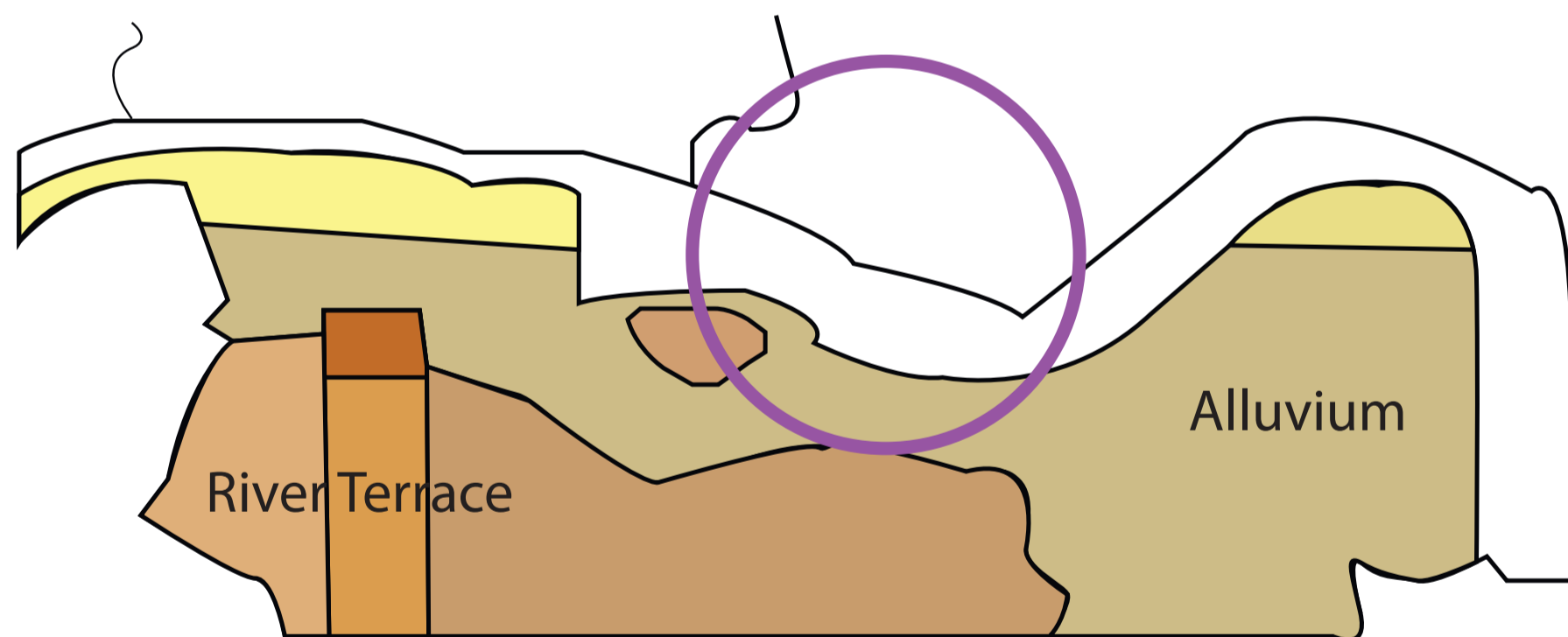
Sand and Gravel Composition

This map indicates the soil composition along the section of the River Thames that includes our site. The pink circle indicates the exact site. As demonstrated, our site consists primarily of clay, silt, sand, and gravel.



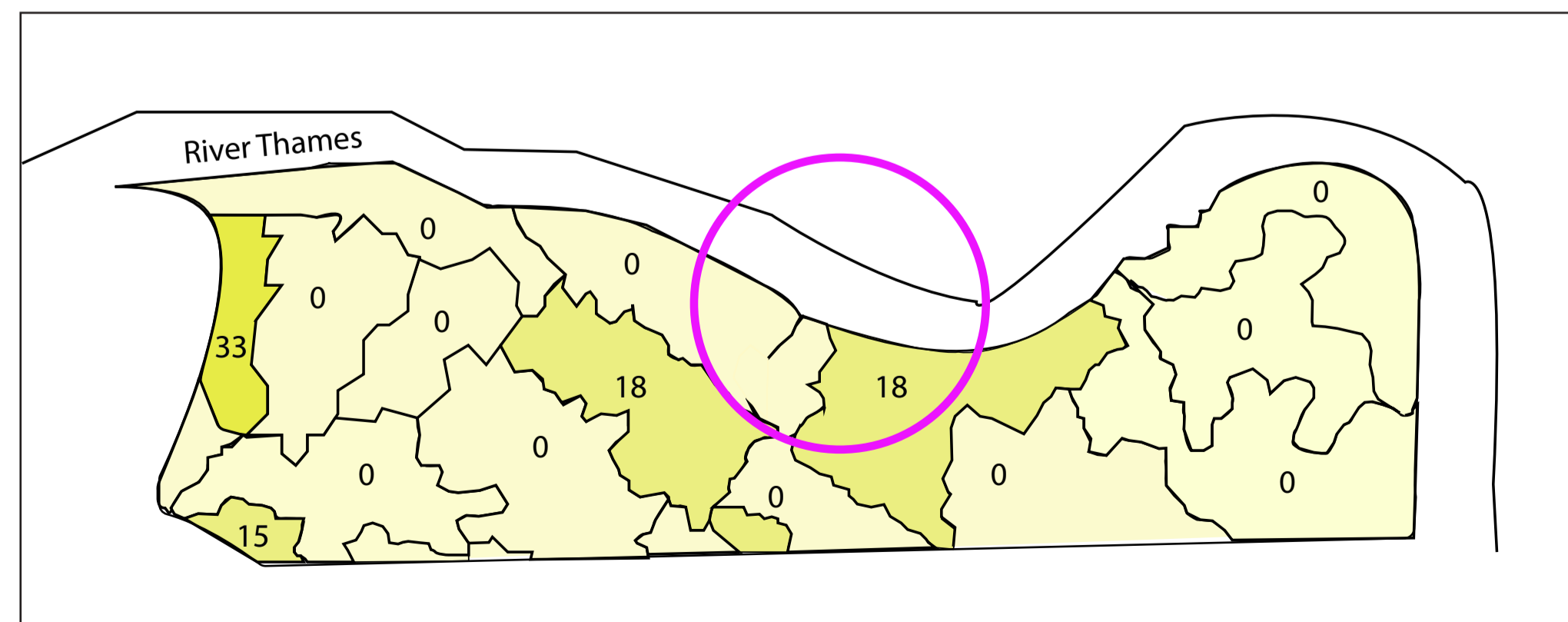
Clay Mapping

This map shows the clay availability surrounding our site. As demonstrated here, there is an abundance of London clay located along the River Thames. This clay is stiff and becomes brownish with oxidation. It is silty, sandy, and contains pyrite.



Alluvium Deposits

Soil deposits by river beds are known as alluvial. This combination of clay, silt, and sand leads to fertile, but typically looser, soil. This soil can also have poorer drainage. The foundations of our design will take into account these characteristics.



Soil Average Plasticity Index

Map showing the plasticity index (PI) of soil surrounding the site. This measurement indicates the range of water contents where the soil exhibits plastic properties. It can be correlated with workability and shrinkage. In this map, a PI of 0 could indicate either a lack of plasticity or a lack of data for that specific region.

Outcomes



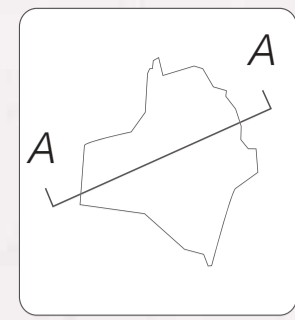
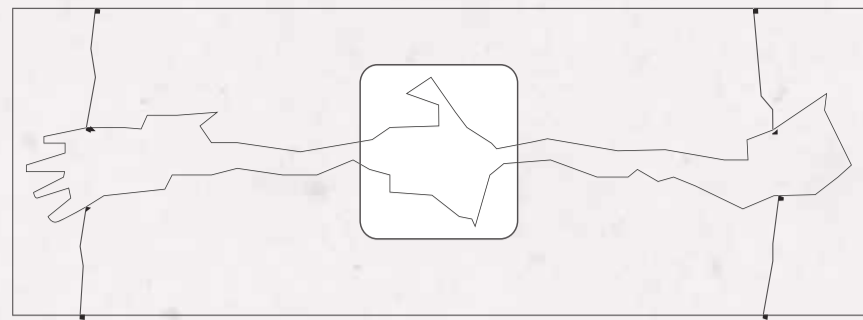
Car sculpting with clay.



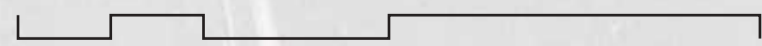
Replacing fire coating insulation with clay instead of concrete.

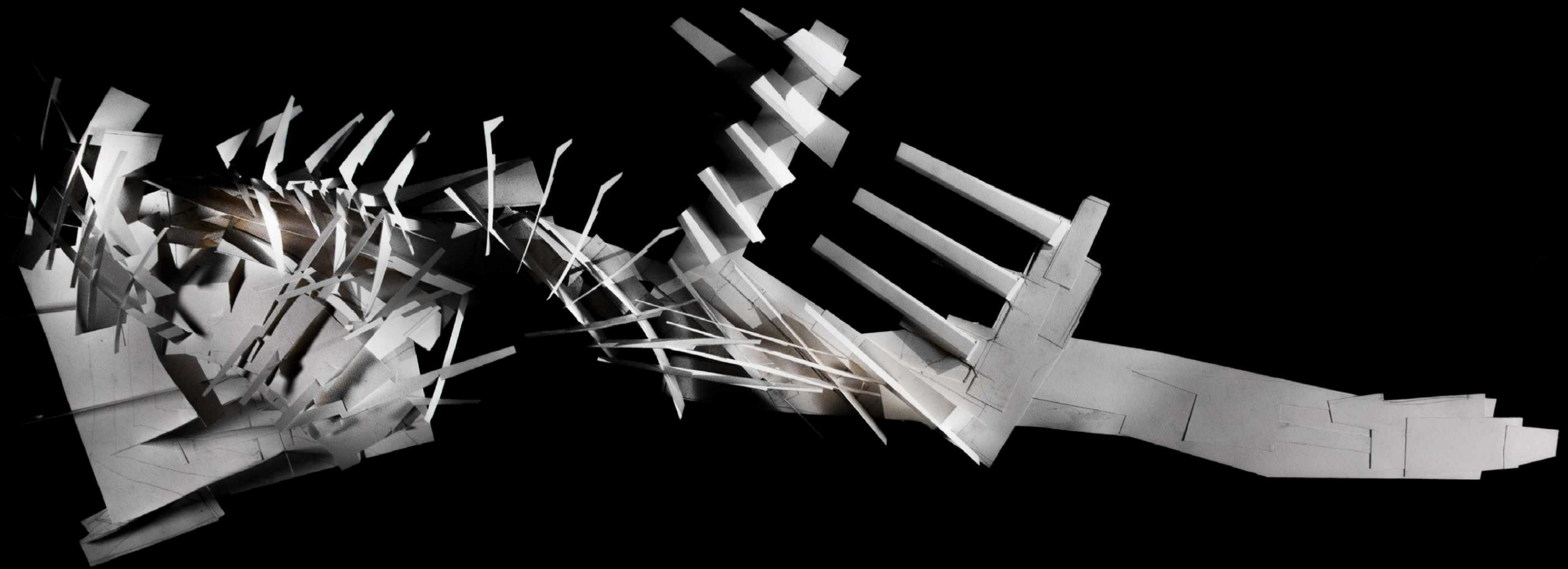


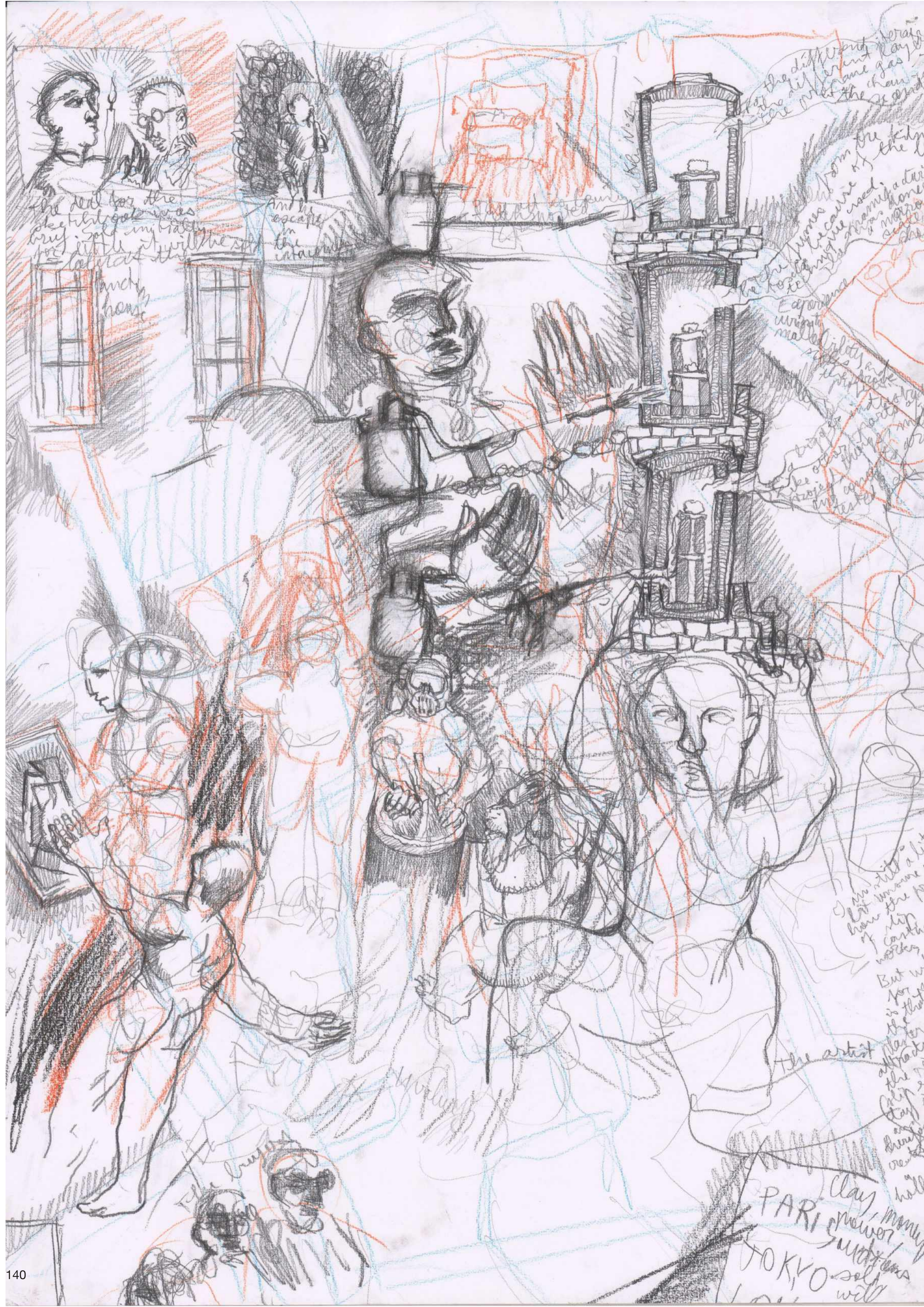
Sacrificial layer.



1-inner fire proof clay coating on steel members
2-sacrificial layer against water

1:50 on A1  4m

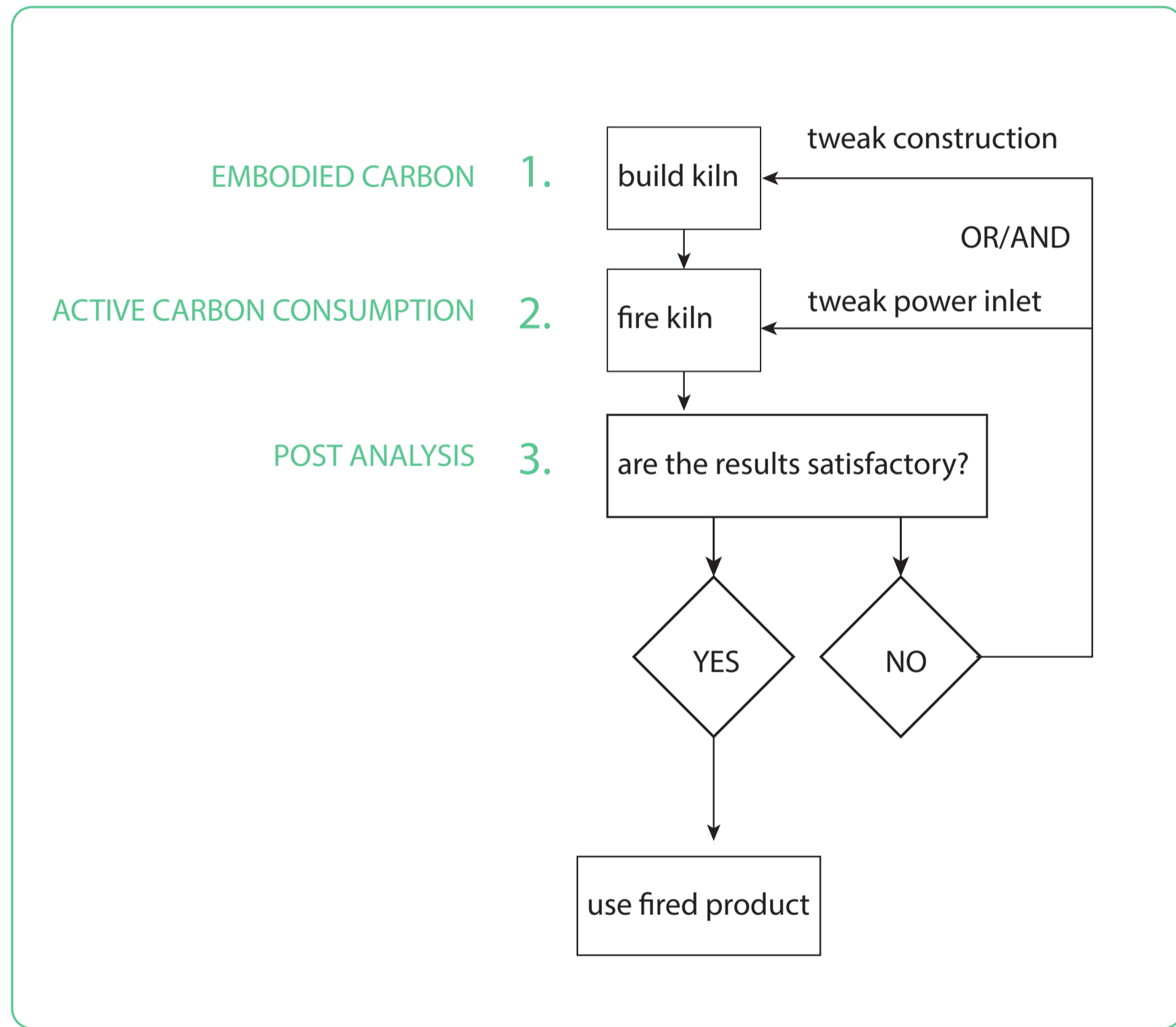
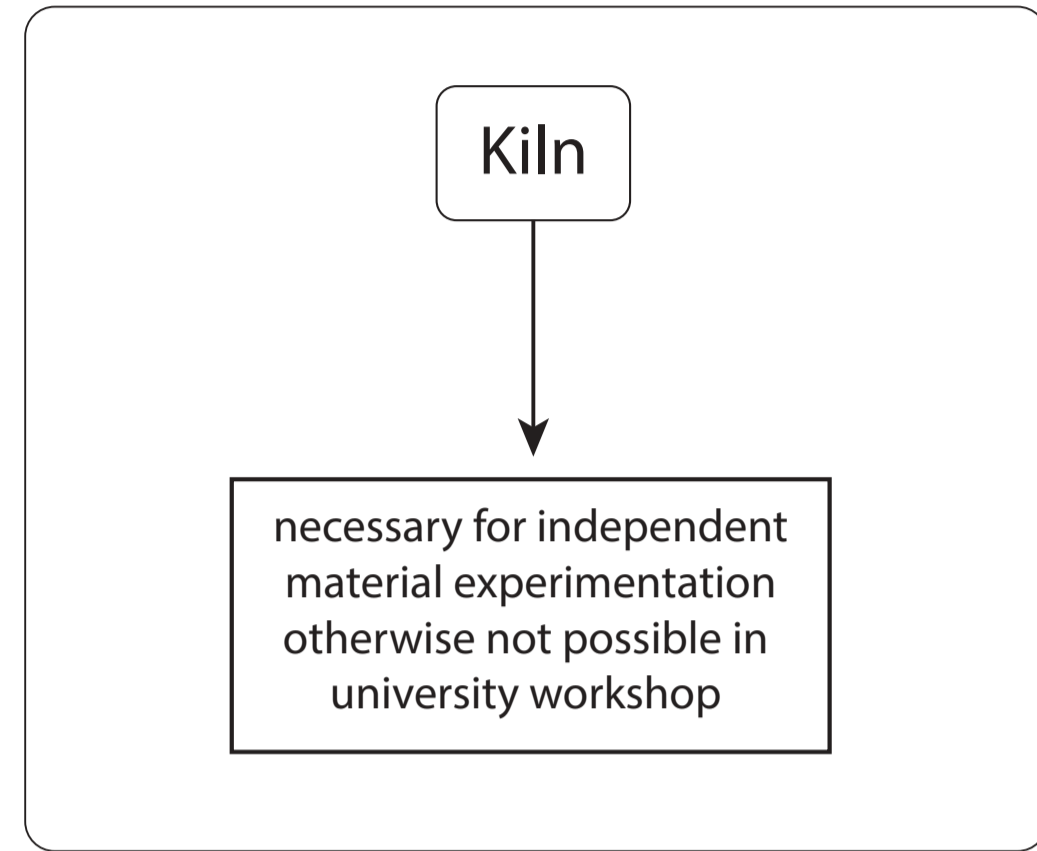




Section 8. Re-Use, Construction and Deconstruction Logic: Kiln

We cannot have the auction house be constructed in such a way that the result will be a grand fiasco. This is why the process of its construction is important, and I created digital images for the viewer to visualise how the construction process of the auction house on the Thames River will look like. The production of clay within kilns was something that had to be researched because without any data on clay there would be no construction of an auction house. The fabrication processes had to be figured out, and the re-use of materials was deeply important as part of a general effort to reach a certain level of sustainability. The kiln became an academic project in itself, and what was learned with it then informed the building. The kiln was always working throughout the year, and parallel to other research, it kept producing new outcomes. The transportation of materials that would be carried out for the auction house construction will have to involve water transports which will be much harder to be carried out than an auction house that would have been constructed on land. A general consensus on the methodology to be used with the construction had to be reached, and this was attained with help of visual images, such as the creation of digital images on how the construction of the auction house would look like.

Kiln Understanding





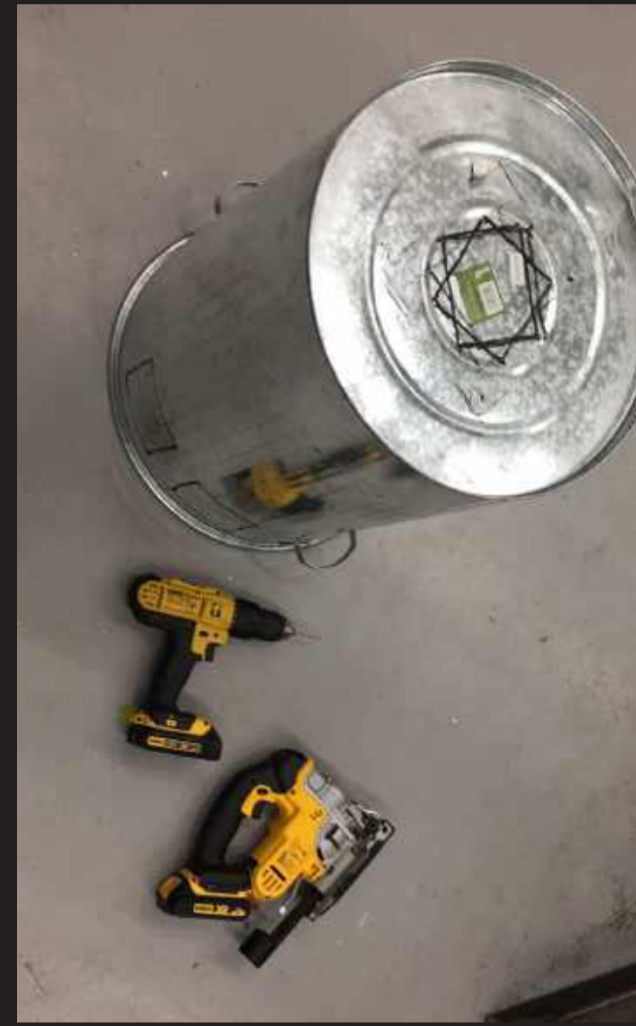








Making a kiln



1-90L steel bin, drill and J-saw to cut openings.



2-Protection clothes with tape around wrists.



3-Ceramic insulation is toxic due to its dust particles.



4-Extract fan is on and cut insulation.



5-Holes are made for fire resistant wires.



6-The wires are put through to hold the fire blanket.



7-Leave some extra insulation on the sides.



8-Make openings for inlet and outlet.



9-Take fire bricks for flooring and shelves.



10-Coat kiln shelves.



11-Install shelves inside.



12-Plug in propane gas with weed burner.

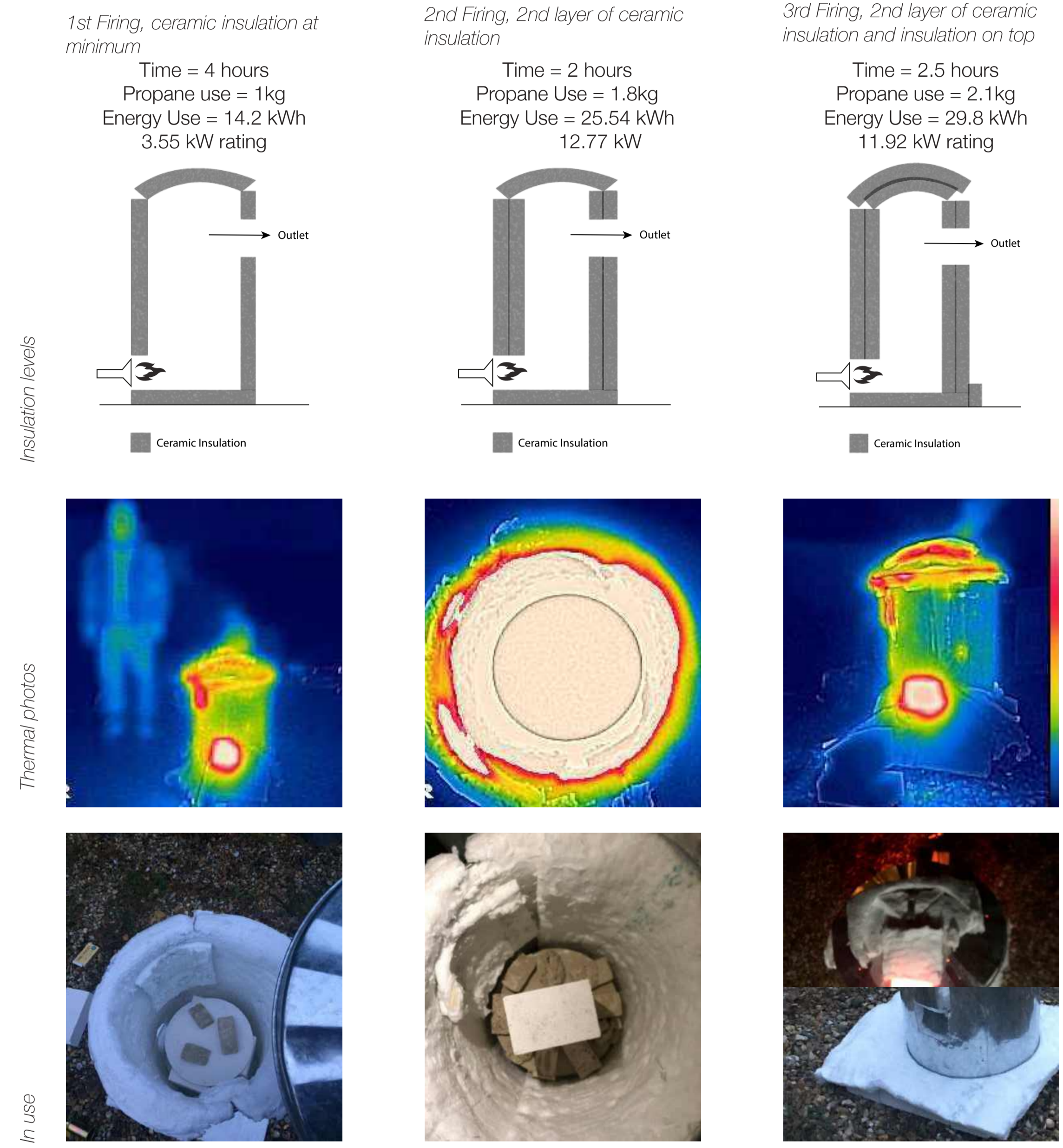


- 1) weed burner
- 2) propane gas
- 3) inlet
- 4) outlet
- 5) welding gloves
- 6) fire bricks flooring
- 7) fire bricks holds
- 8) ceramic insulation floor
- 9) gravel
- 10) kiln

Value and Energy Testing

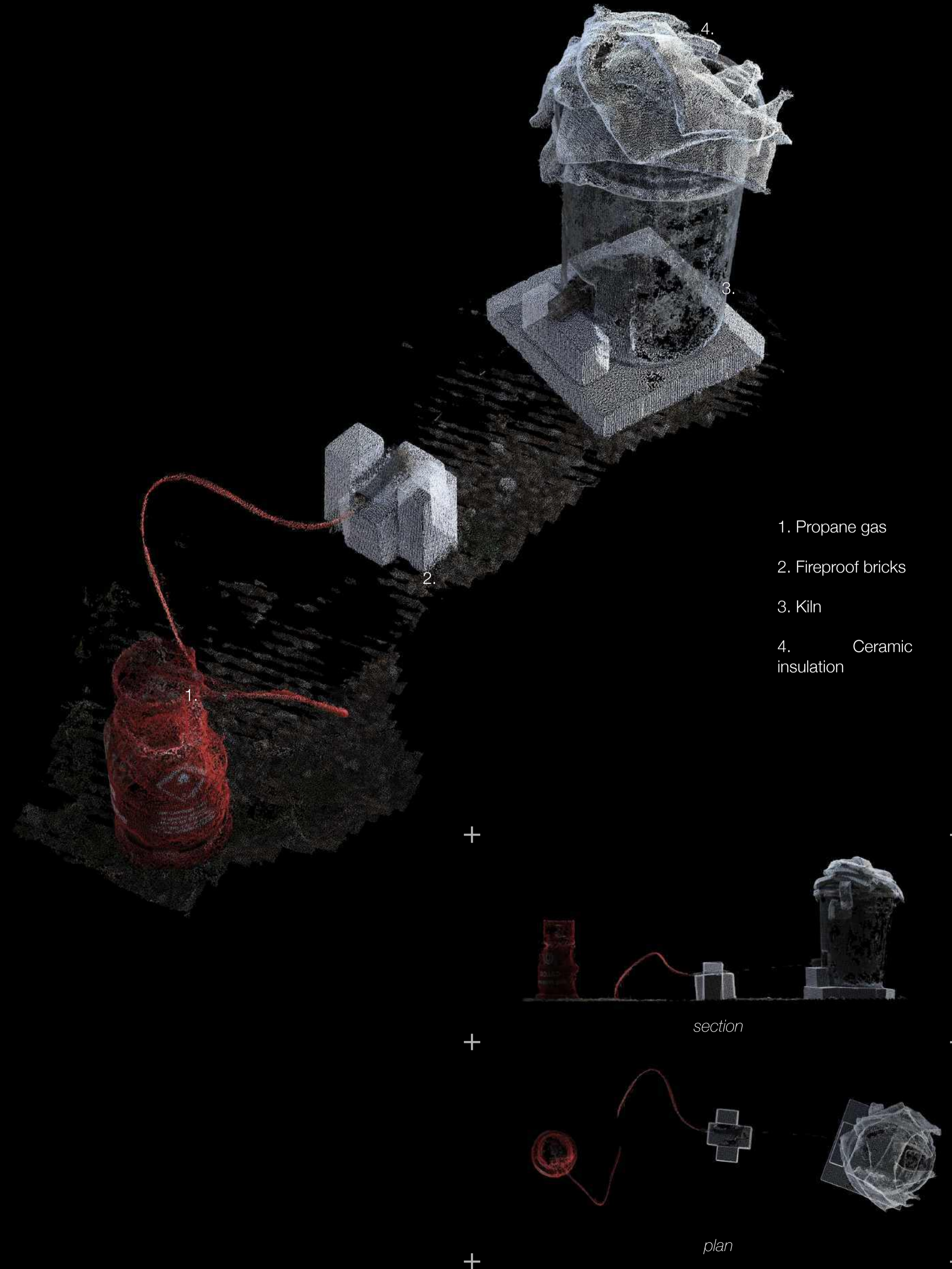
Kiln Energy Use

An experiment was conducted to determine energy use and the optimal role of insulation. Insulation was successively added to the kiln in three configurations. For each of these configurations, firing time and propane use was carefully measured. Using this information, the kWh and kW rating has been calculated, where 1kg of propane is 14.19kWh (per FloGas).



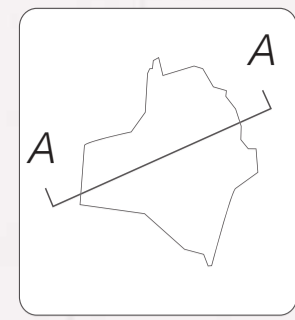
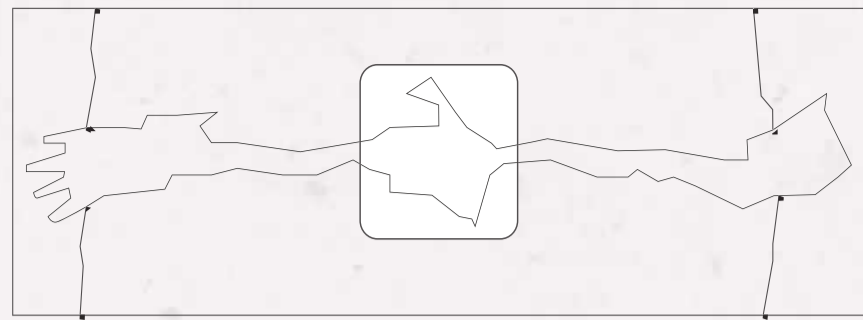
Summary

Small kilns draw between 1.5-1.8 kW, while medium-sized kilns draw between 5-8kW. The ceramic kiln design therefore falls well below the energy usage of a typical kiln. The ceramic kiln is additionally sustainable because it is constructed entirely using a sustainable material (clay) rather than employing a steel frame.



KILN





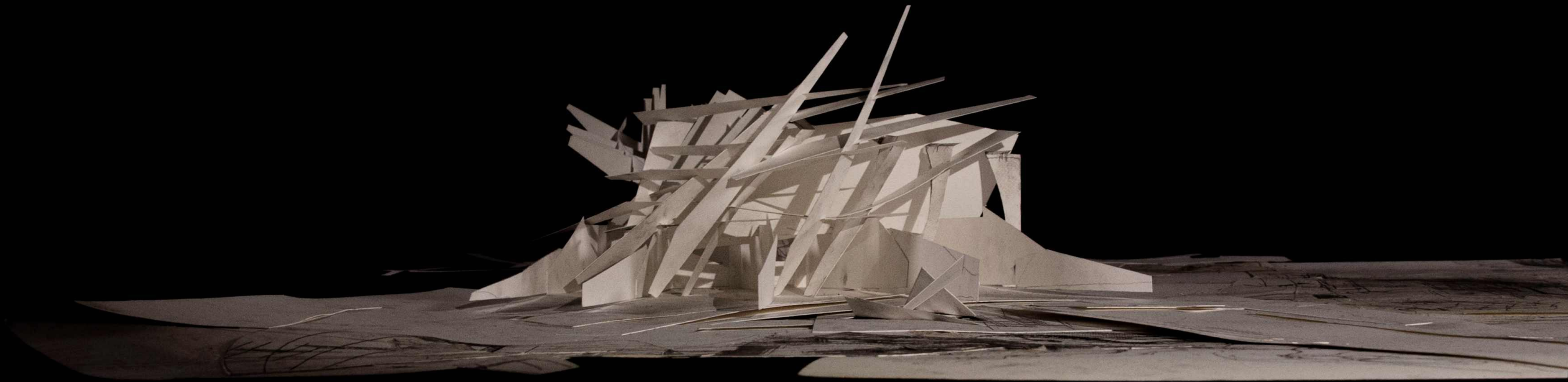
smoking

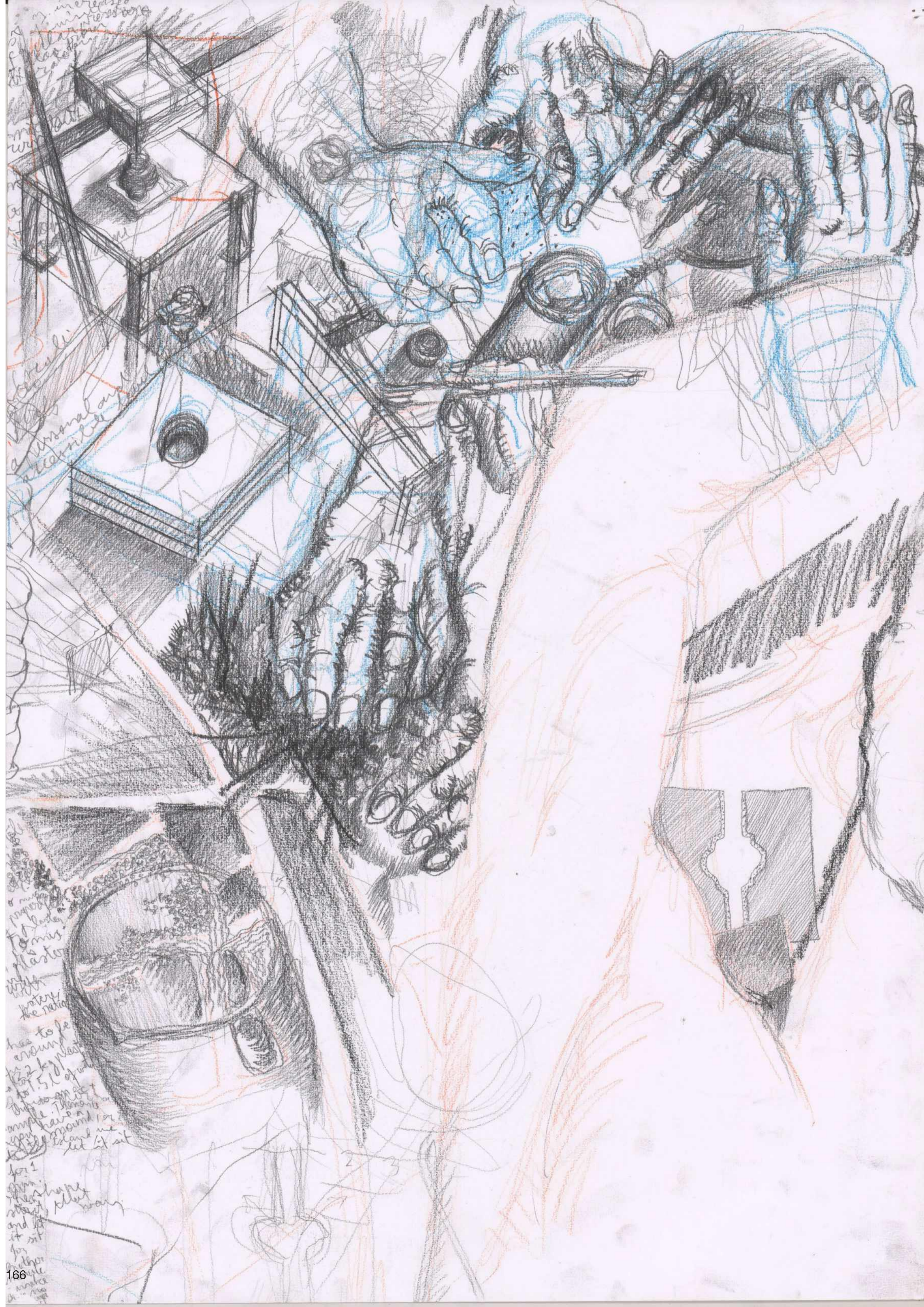
fumer

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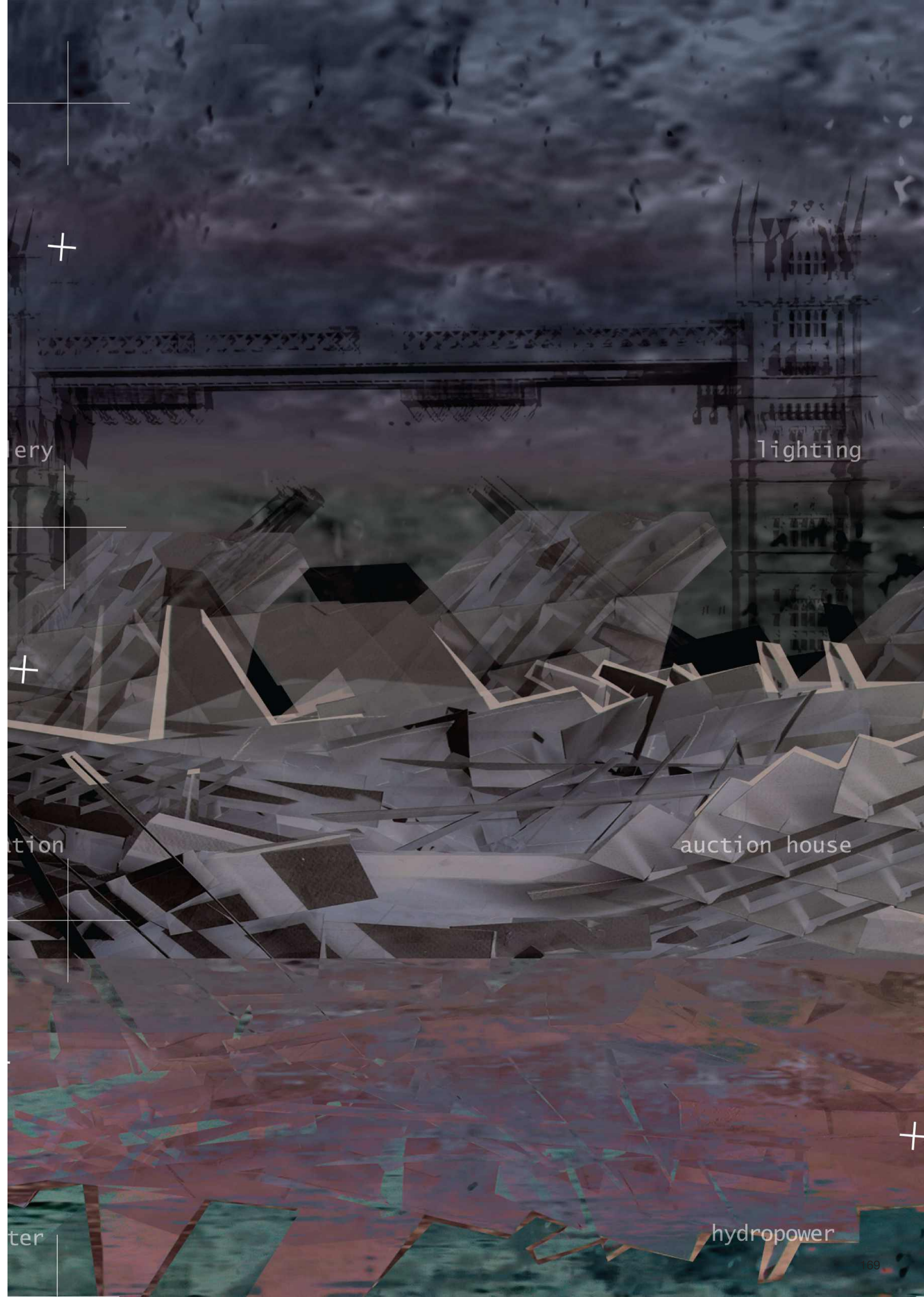
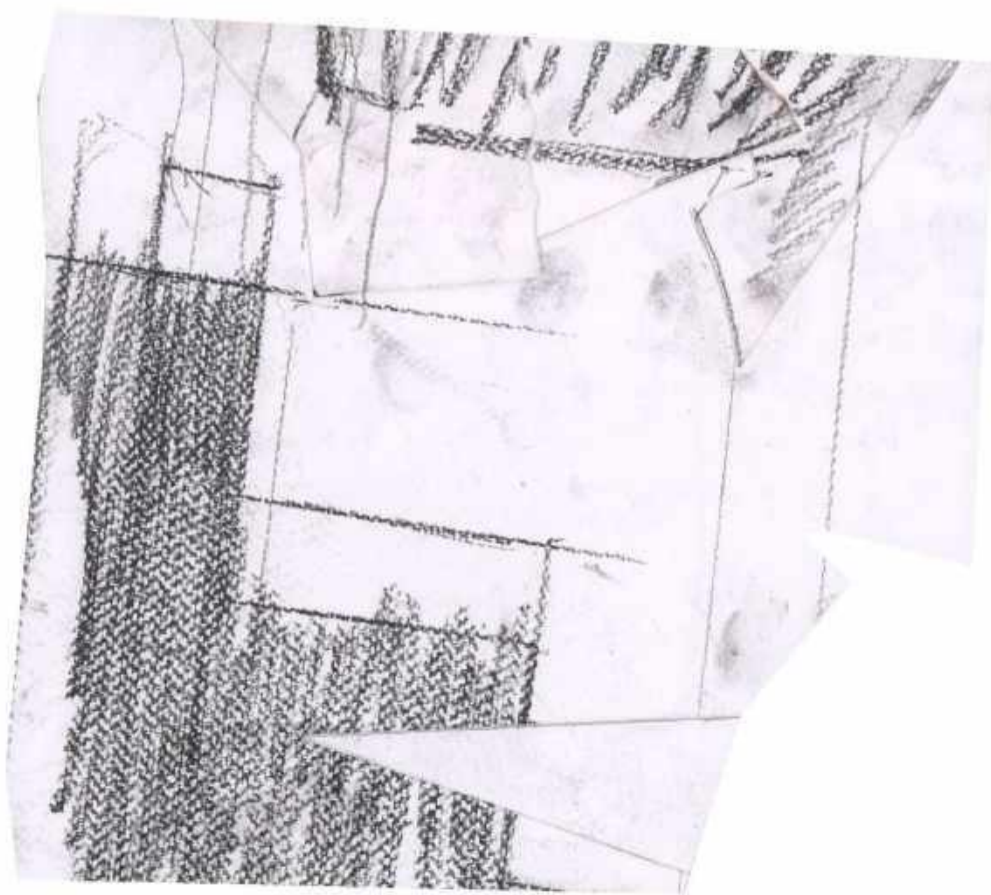
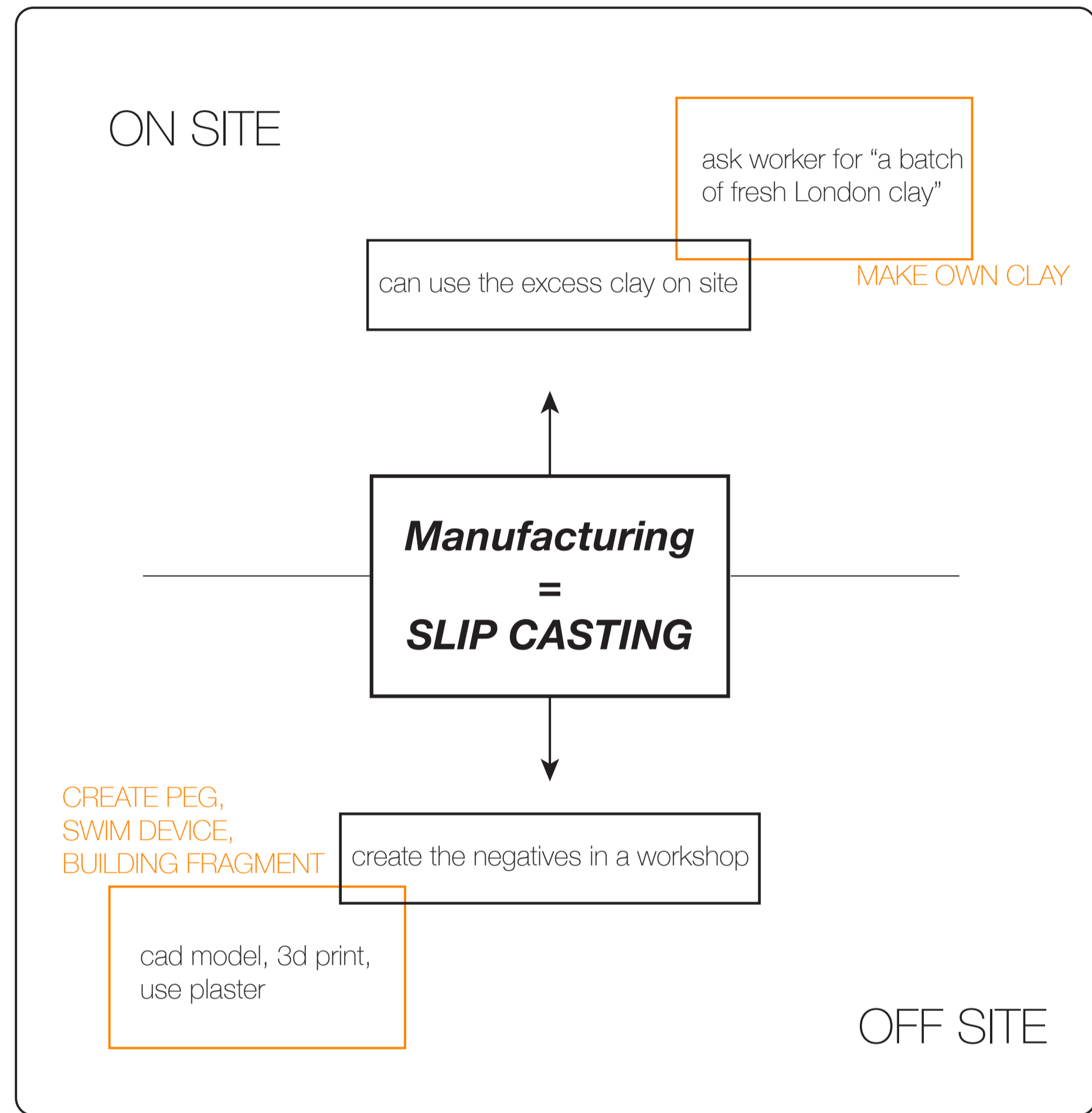
4m



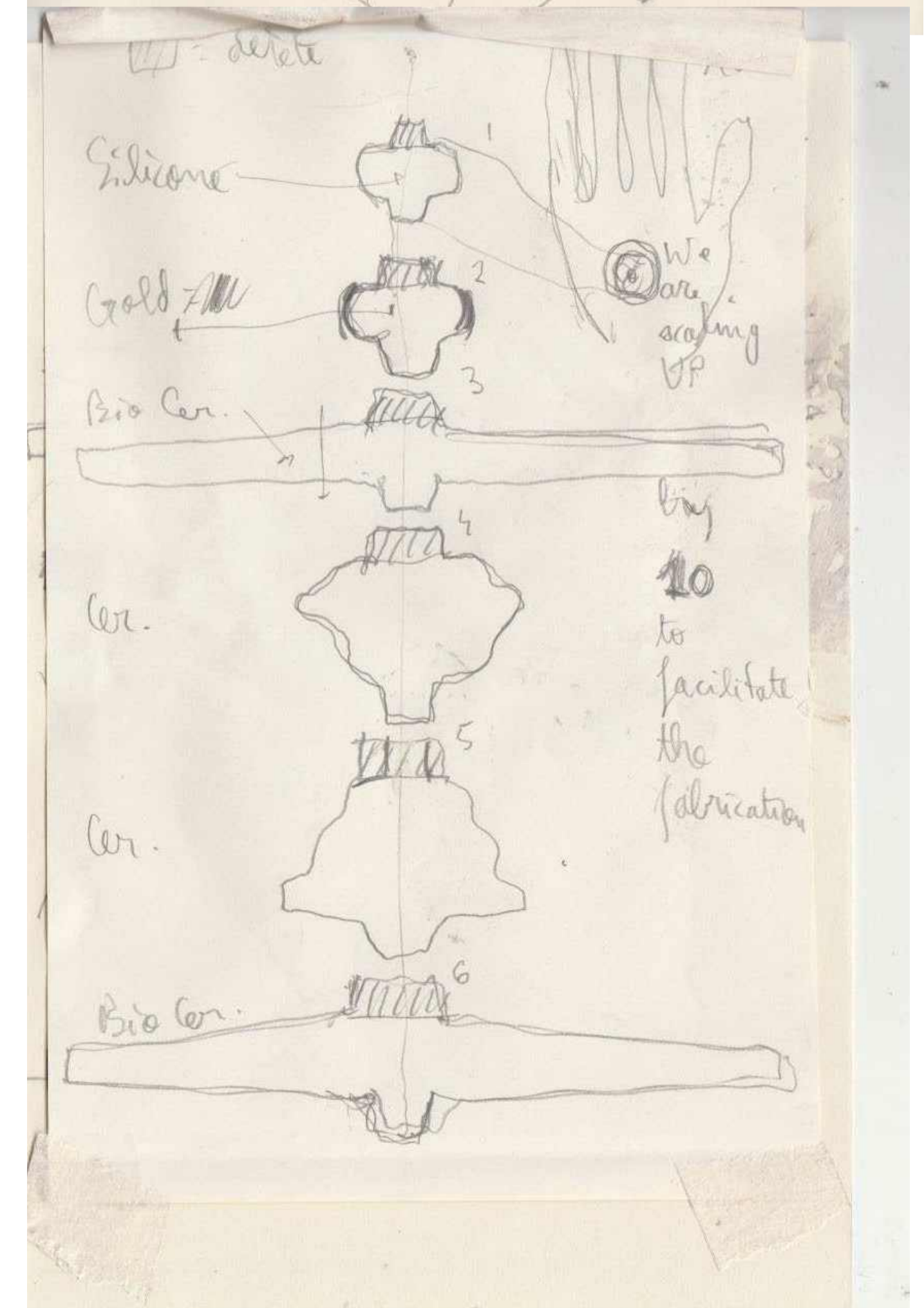
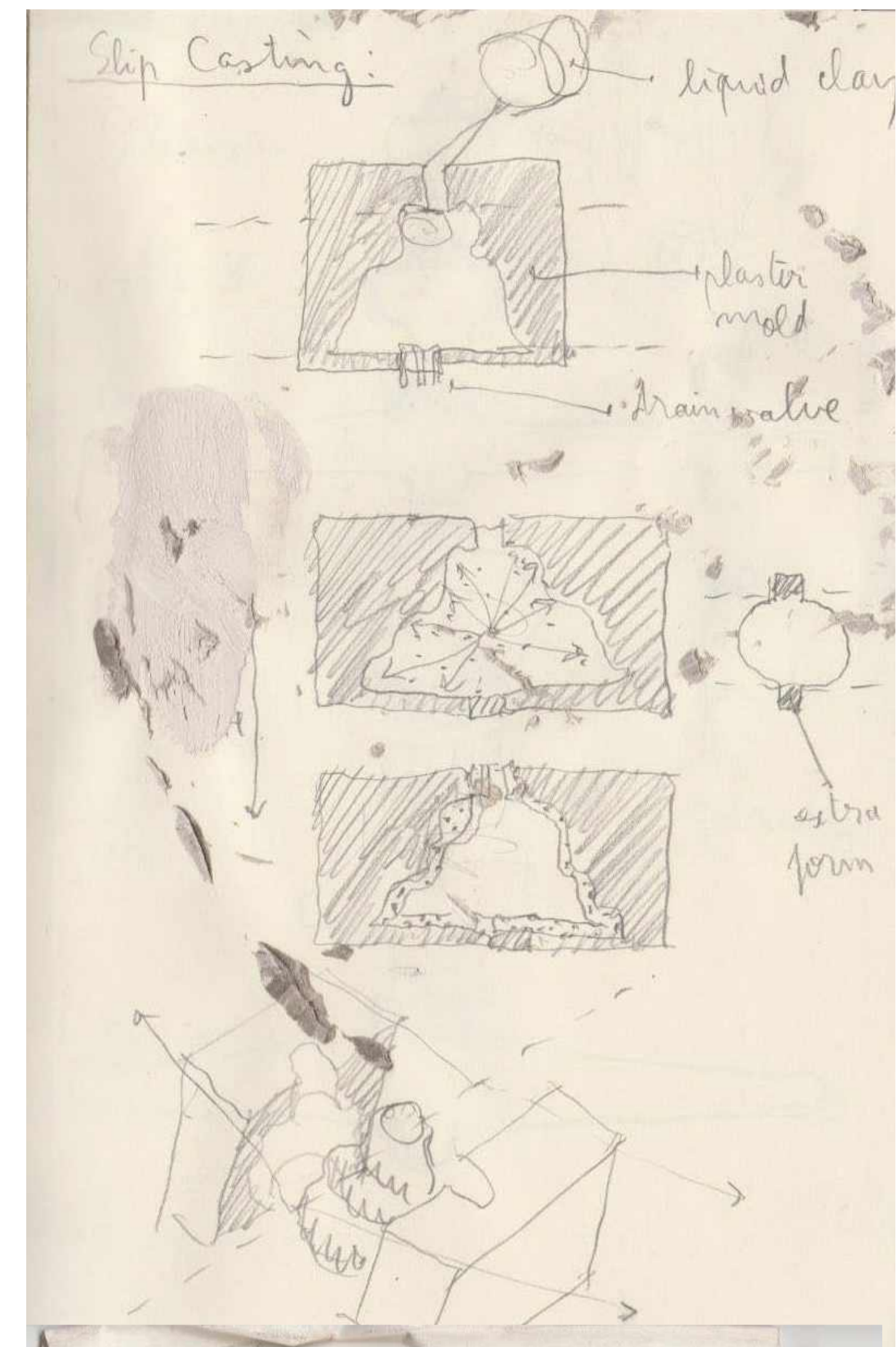
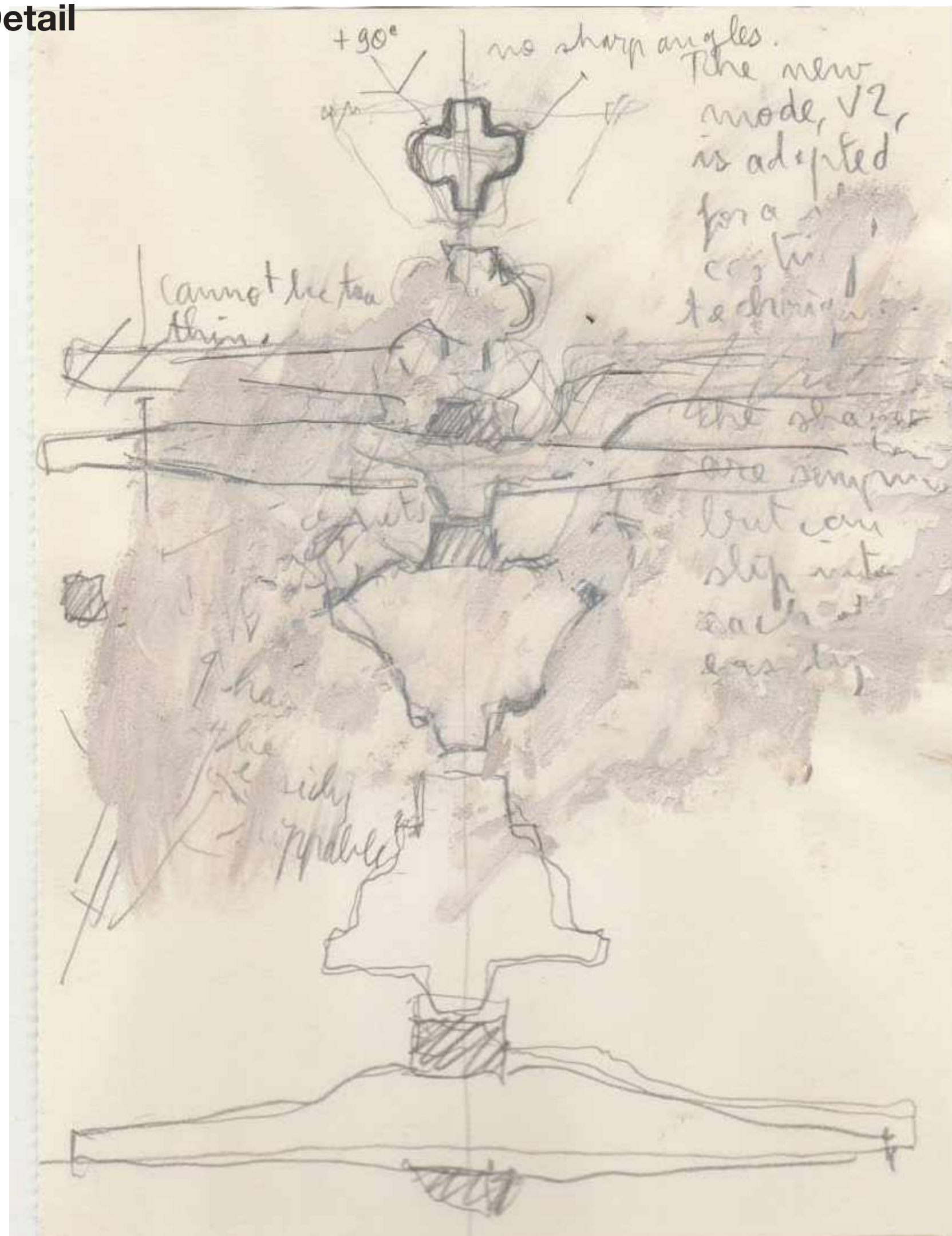


Section 9. Individual Material System: Manufacturing

For section 9, it was necessary to highlight the construction constraints that would be faced as well as a general assessment of specialist equipment to be used. As such, manufacturing slip casting was explored as a manufacturing mean for clay. This material can be used to create homeware pieces. It can also produce complex clay tiles in large quantities. It was absolutely vital to see what sort of method can be used in relation to the creation of clay as well as having a method related to art. My research into slip casting was helpful, and I related it to my experiment and concept design PEG. For this project, PEG would be manufactured using the technique of slip casting. I decided that in theory, the pull buoy could also be created out of slip casting, as well as the building fragment. It created an exponential scaling that naturally took form later on in a building proposal.



Slip Castin Feeding Tube and Facade Detail



Gold leaf glazed slip casted geometries

Slip casting was the main technique used to fabricate the different clay geometries. Later on burn out technique was applied onto the geometries that created irregular forms within conventional shapes.

Different glazes were applied, including leafed gold for the bacterial free properties it has when in contact with the skin (for the feeding tube).



Slip Casting Detail



1-3d print detail, put clay and soap on it



2-cover half of the mold with plaster



3-cover the other half with plaster



4-make sure it s dry enough



5-molds



6-put rubber bands and peg



7-ready to pour slip



8-prepare slip



9-pour slip in mold

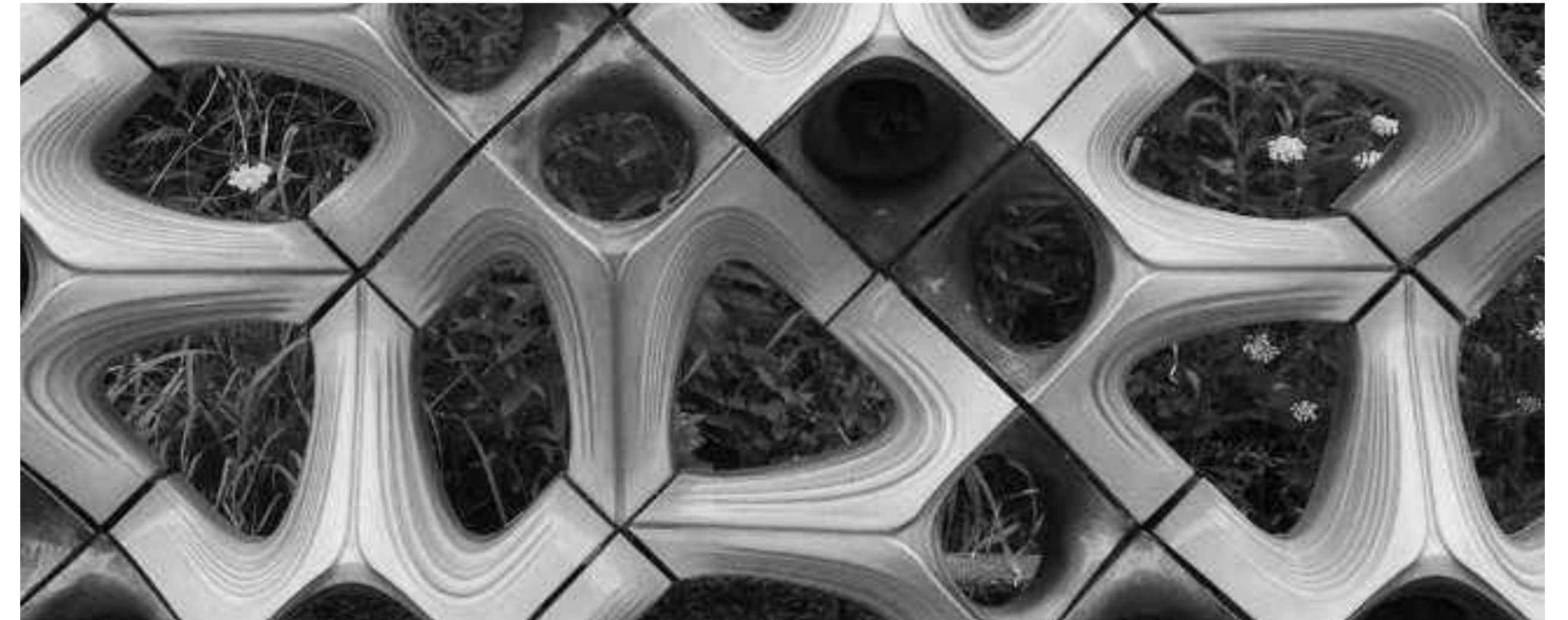


10-pour extra slip out



11-let it dry for 2 days before firing

Industrial Slip Casting



Buro Happold facade system for urban rewilding.

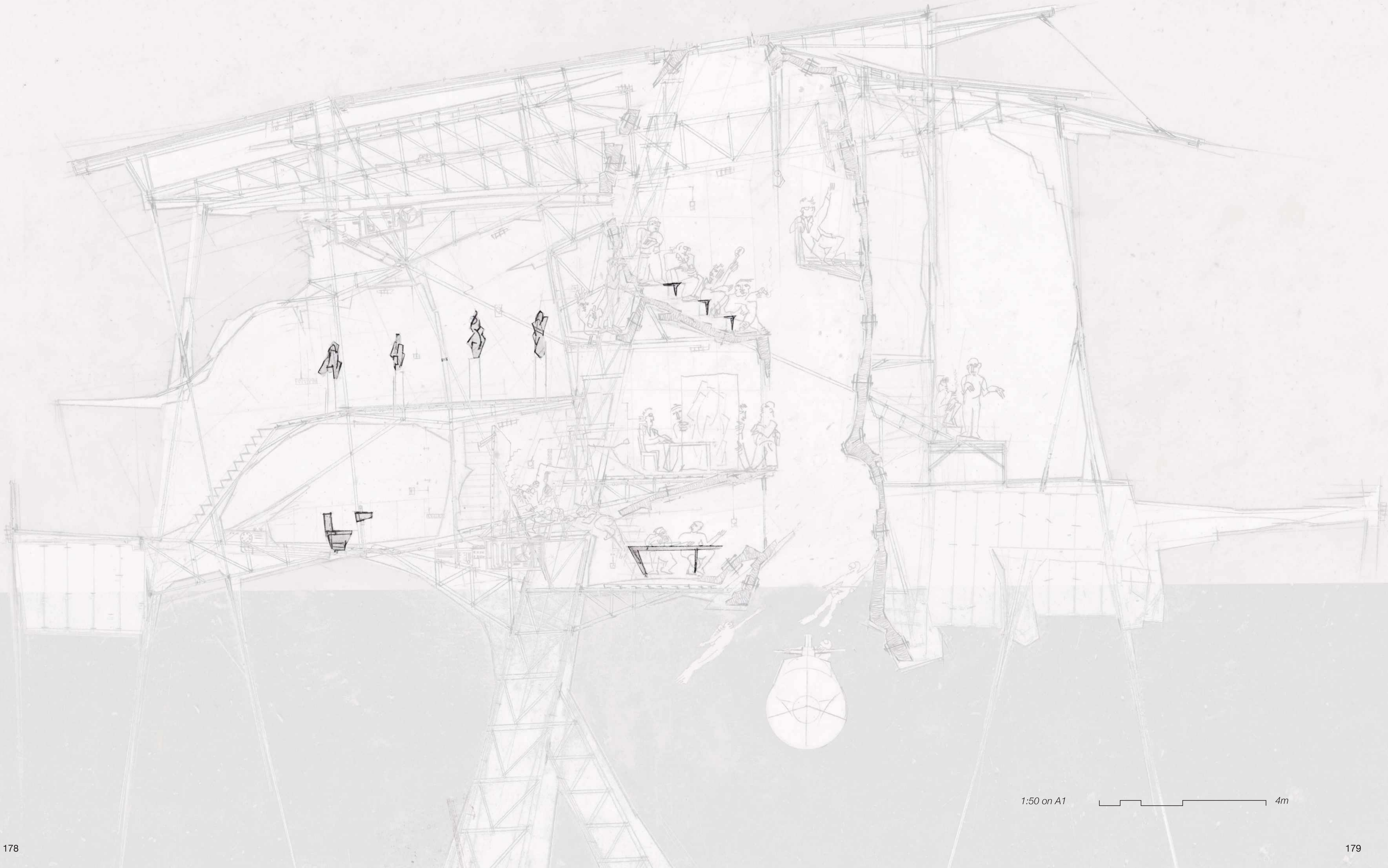
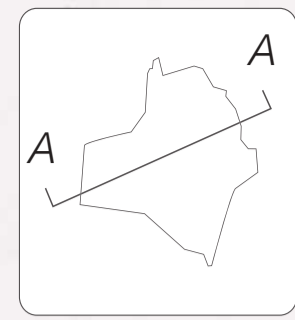
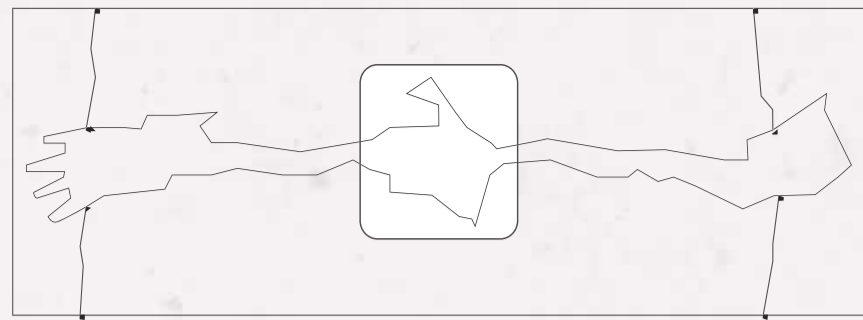


Architectural decorative detail

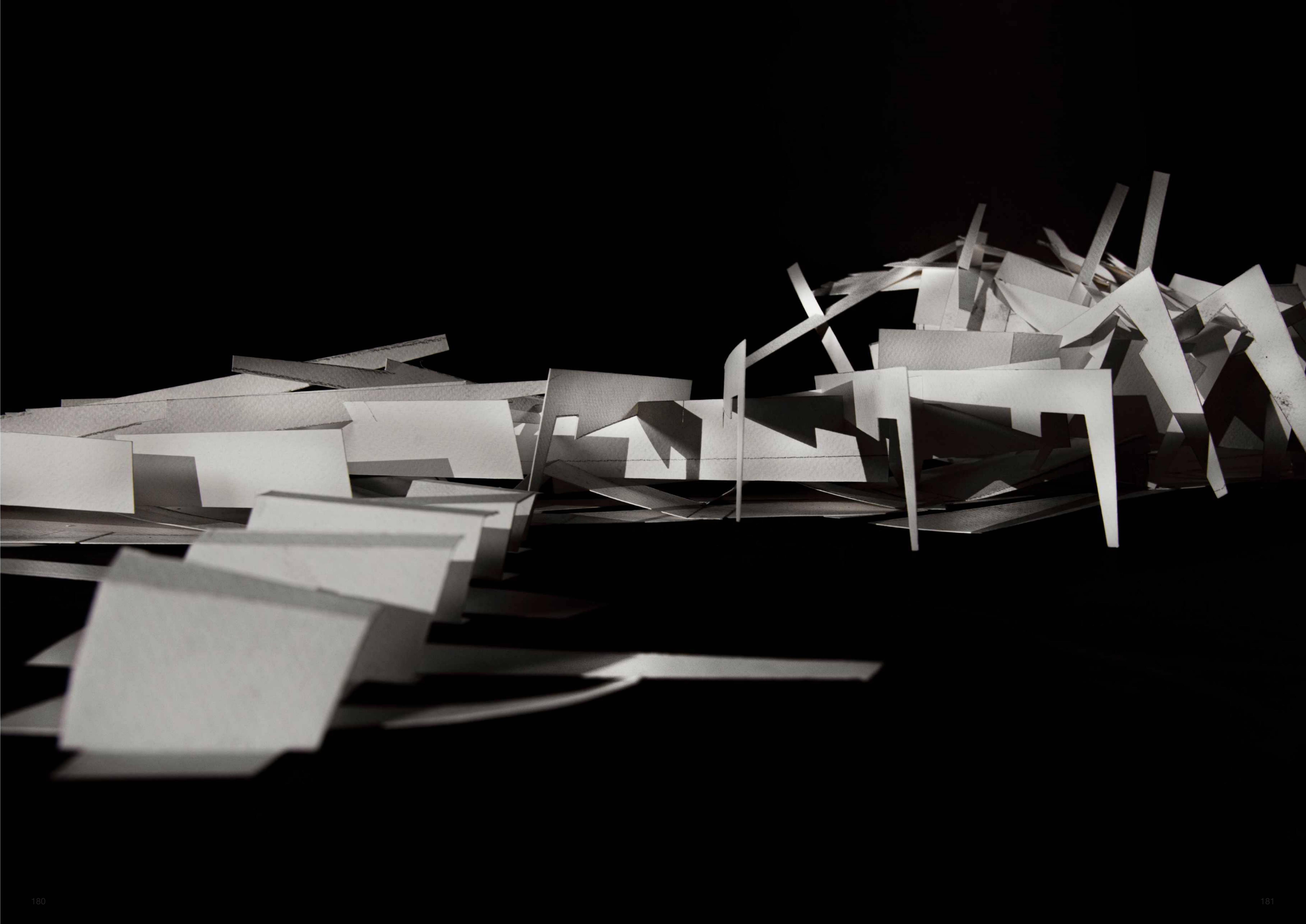


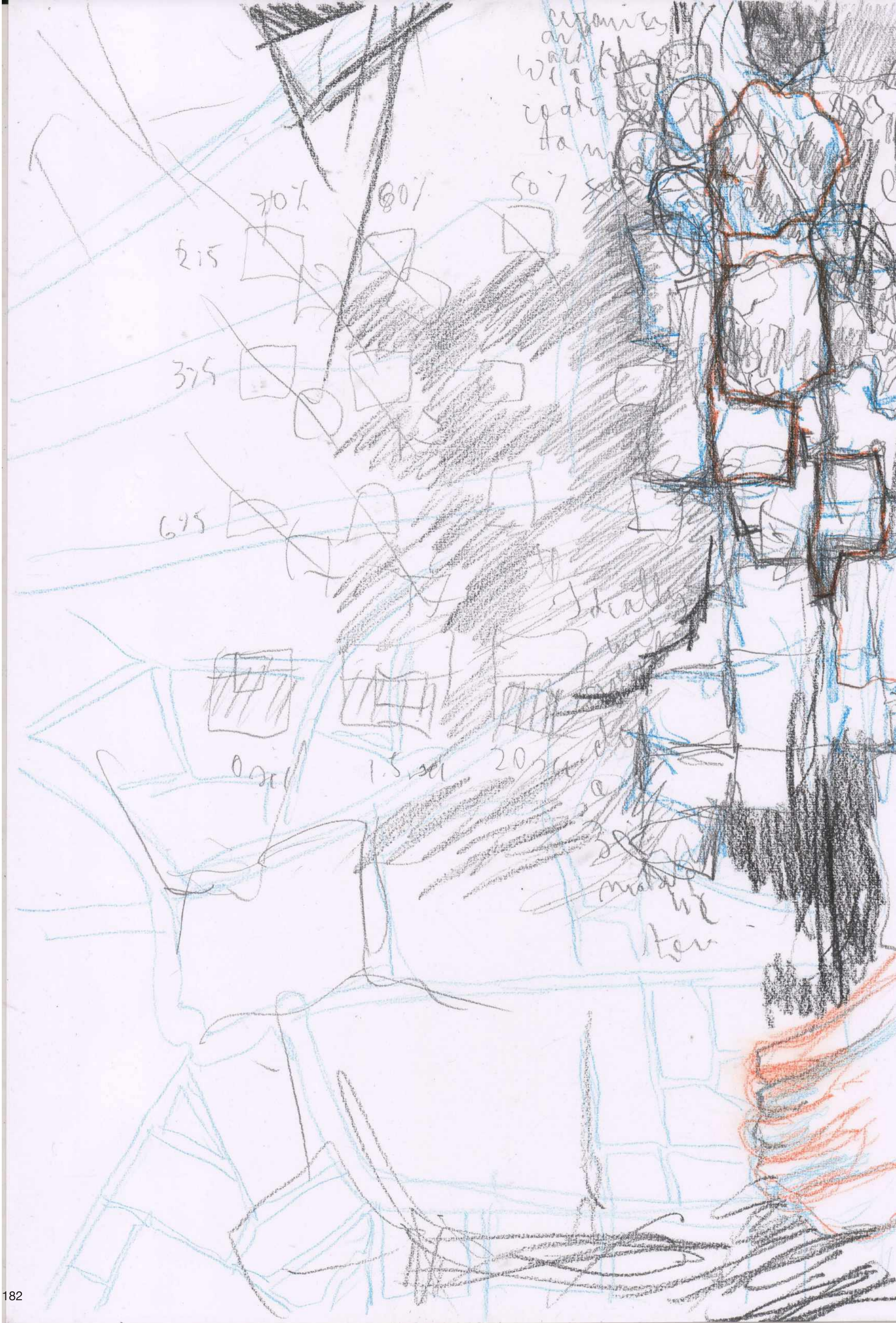
Toilets





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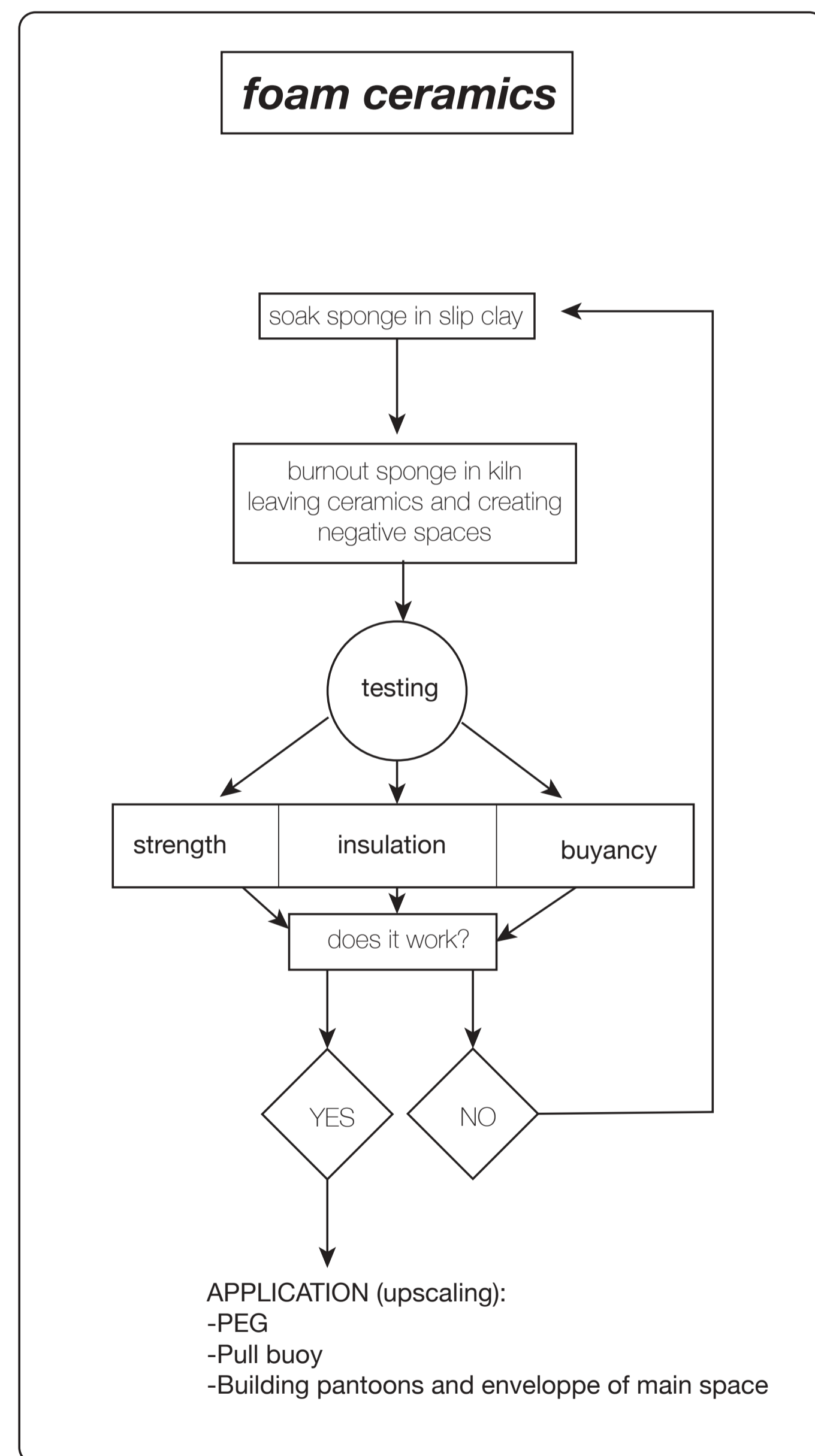


Section 10. Strain Test: Foam Ceramics

Concerning ceramics and the project as a whole, I can state that foam ceramics was the part in which I studied extensively. It was researched rigorously and was very much in depth. I decided to research first on the material typology, such as clay, concrete and foamed concrete, as well as a classified and up to date comparative table of different kinds of foam ceramic processes. These included starch consolidation, direct foaming, replication, gel-casting and lost material aeration. I had to see which methods were more industrially used, more convenient, its cost, which one was most efficient, as well as the positives and negatives of each. At the heart of this strain test on foam ceramics was sustainability. Each aeration method was also judged for its sustainability, which remains a very important element within this project. There were strength, buoyancy and insulation tests that were done, and the self made kiln was allowed to do different burn out experiments, being independent of B-made. The fabrication primarily served as a material exploration, but it was also an aesthetic means to create intricate materials that looked like corals. The aesthetic element was certainly not forgotten. After being finished with the experiments, the materials could theoretically be put to practical use.

ON HOLD
ON HOLD

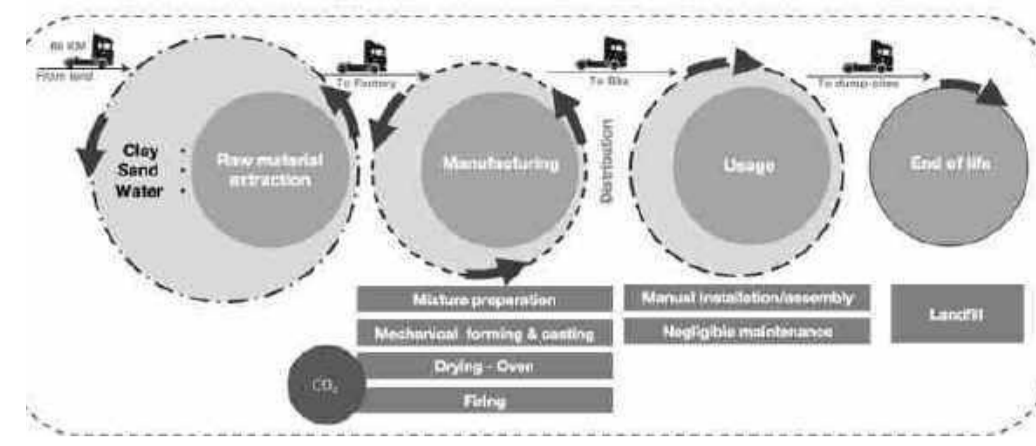
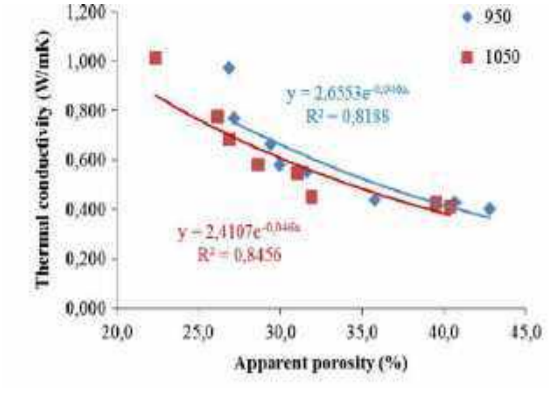
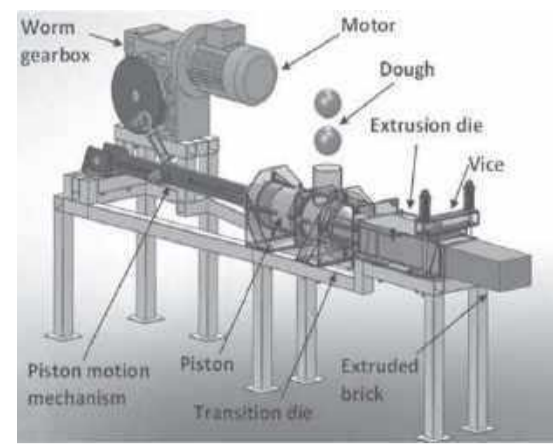
The use of fire as a mean to create negative spaces is of interest. Future works will be done in this field, not covered in the project.
8th June 2023, London UK
Maxime Ostrovhenry
Maxime Ostrovhenry



Material Typology

Clay (Brick)

Ground clay in bricks is usually comprised of alumina, silica, lime and iron oxide. It is mixed with 10-15% water, extruded through a die to remove air bubbles, and cut into the final shape. These bricks are then dried and fired. The process takes between 10 and 40 hours.



Main Uses: Construction (Facades, driveways, etc)

Bulk Density: 1800-2000 kg/m³
Porosity: 30-40%

Compressive Strength: 12-40 MPa
Thermal conductivity: 0.5-1.0 W/mK

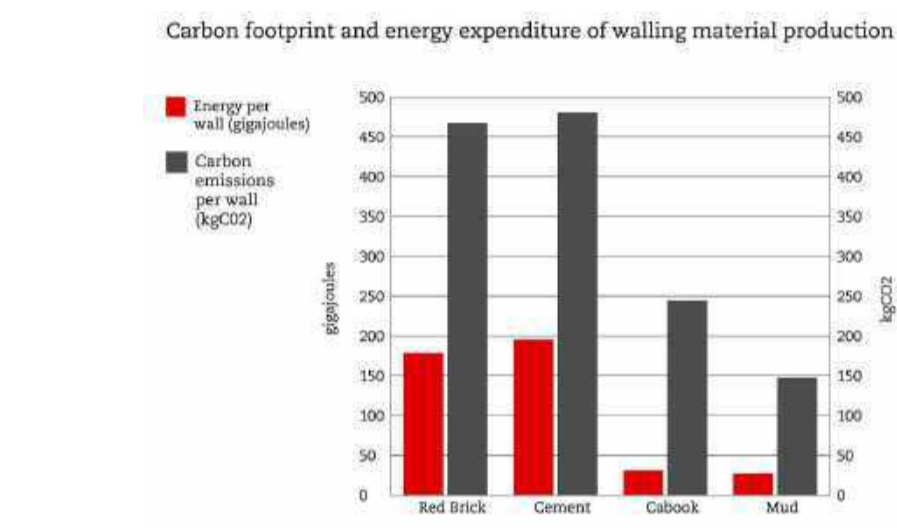
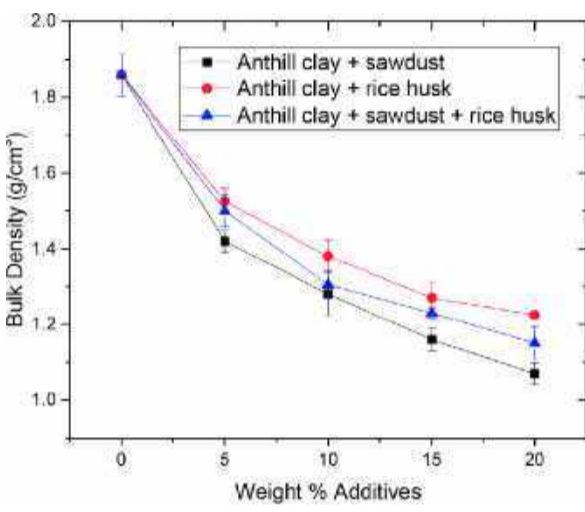
Sustainability: Considered a green building material. Consists of naturally abundant raw material and very durable. Emissions vary based upon firing technique, though coal is the most common (.81 kg CO₂ per brick)

Mud and Natural Additive

The practice of adding additive to earth has been in practice since antiquity, when mud was mixed with straw and vegetable matter and dried by the sun. It is now employed widely in low-income settings and by DIYers to cheaply and simply decrease density while improving insulation. Additives commonly include straw, vegetable fiber, and animal fiber. The brick usually dries via the sun, though sometimes is sintered to remove the additive.

Brick samples (Clay + % Additive)	Bulk Density ρ (g/cm ³)		
	Total	Trial 2	Trial 3
Reference Value	1.85	1.80	1.73
Clay + Typha 1%	1.73	1.81	1.71
Clay + Typha 5%	1.65	1.62	1.77
Clay + Typha 9%	1.60	1.64	1.68
Clay + Typha 13%	1.48	1.49	1.62
Clay + Typha 20%	1.44	1.49	1.59

* Coefficient of variation = 100 * (σ/μ)



Main uses: construction, infill

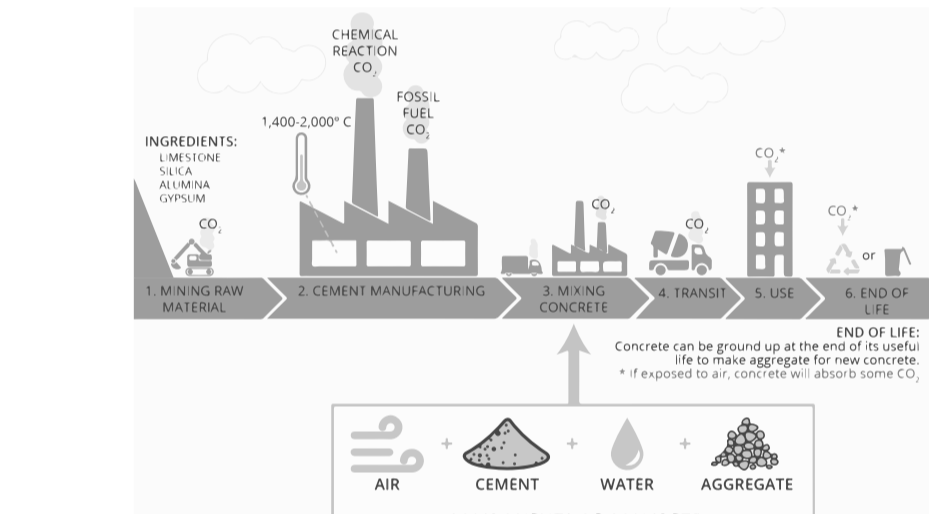
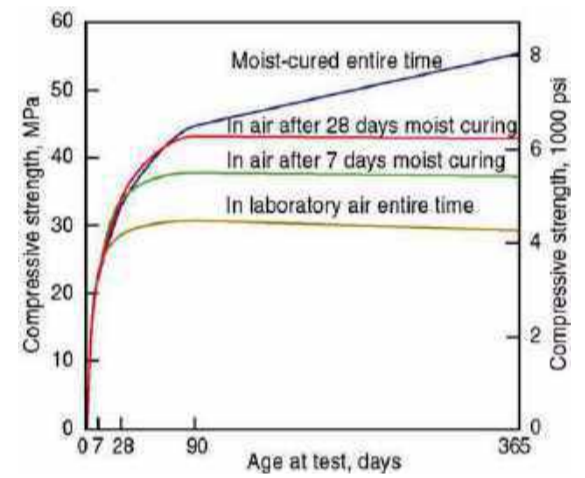
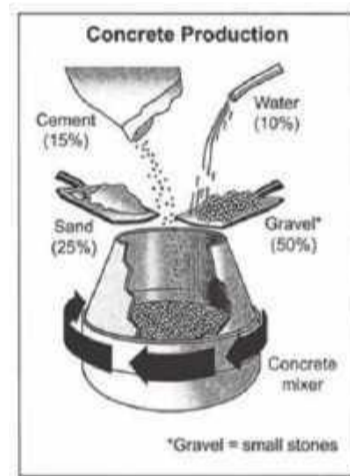
Bulk Density: 1000-2000 kg/m³
Porosity: Varies widely.

Compressive Strength: 3 -17 MPa
Thermal conductivity: 0.3 - 0.9 W/mK

Sustainability: Considered a green building material. Consists of naturally abundant raw materials and incorporates waste. Often doesn't include a firing process. However, not very durable.

Concrete

Concrete is created by combining 10-15% portland cement (primarily lime, silica, and alumina) with 10-15% water and 65-75% aggregate. Steel fiber may also be added to the mix to create reinforced concrete. The water hydrates the cement causing it to solidify. Before the concrete hardens, it is placed and compressed to remove air bubbles. While the concrete cures, its moisture must be carefully regulated to control strength and avoid cracking.



Main uses: construction, transportation, and insulation.

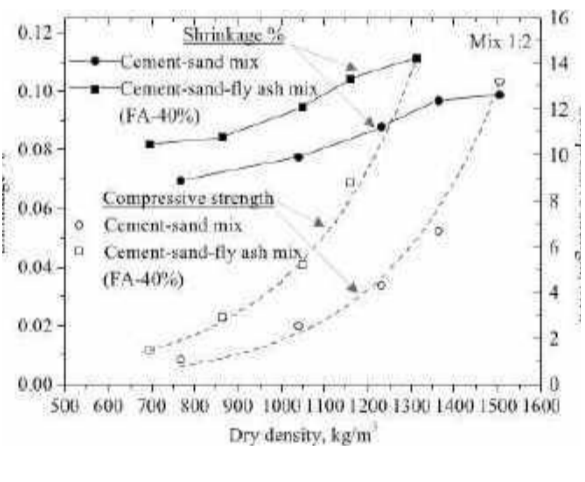
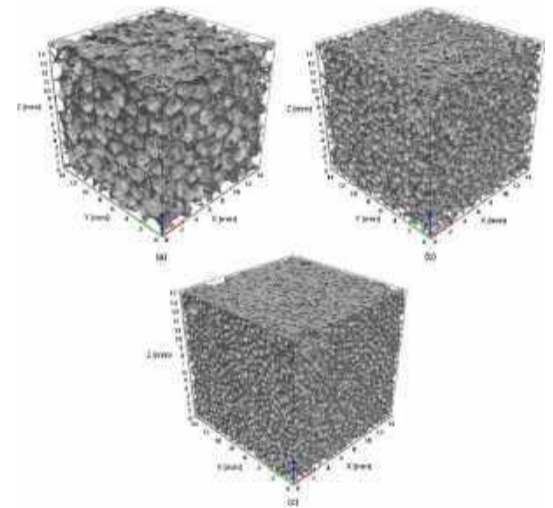
Bulk Density: 1900-2400 kg/m³
Porosity: 9-10%

Compressive Strength: 17-40 MPa
Thermal conductivity: 0.6-3.3 W/mK

Sustainability: Cement production is the 3rd largest producer of CO₂ globally (largely from calcining lime). Quarrying generates air pollution and is responsible for 10% of industrial water use.

Foamed Concrete

Cement and water (sometimes in combination with fly ash and sand) are mixed together with an aerated foam. In order to be classified as foamed, the mix must be at least 20% foam by volume. In the pre-foam method, pre-prepared foam is added to the cement. In the inline method, the foaming agent is added directly into the base material.



Main uses: insulation, bridge approaches, void fill.

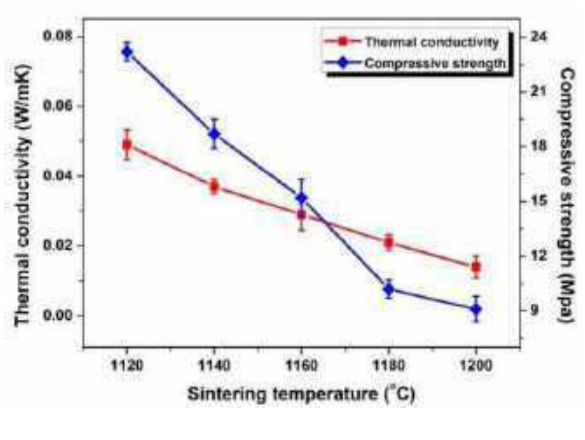
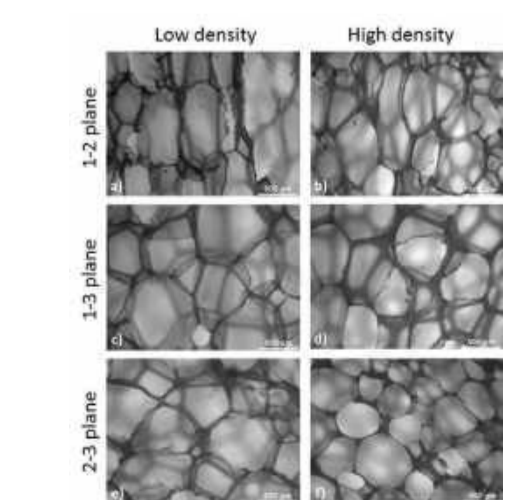
Bulk Density: 300-1600kg/m³
Porosity: 30-70%

Compressive Strength: 0.2-18 MPa
Thermal conductivity: 0.1-0.7 W/mK

Sustainability: Lower thermal conductivity results in better insulation for buildings. However, uses more cement than normal concrete -- resulting in water use and CO₂ emissions.

Foamed Ceramic

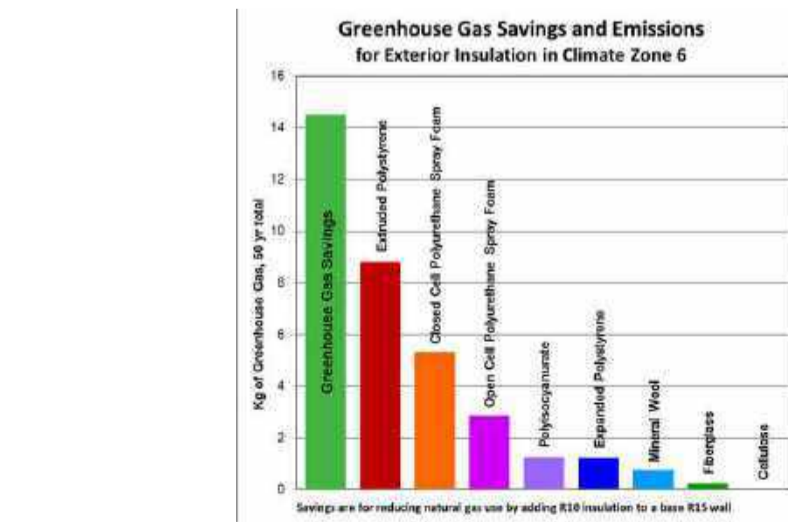
Several different manufacturing processes (outlined 10.1) can be used to produce foam ceramics. In industrial applications, the most commonly used ceramic foams are silicon carbide (SiC), alumina (Al₂O₃), and Zirconia (ZrO₂). Of these, alumina ceramics produced using the replication method are the most popular.



Conclusion: Foamed materials can lower the thermal conductivity by decreasing the density -- effectively replacing the solid with pores of gas of a lower thermal conductivity. Of the foamed materials, foamed ceramic is the most sustainable because it foregoes the use of cement.

Bulk Density: 300-600 kg/m³
Porosity: 70-90%

Compressive Strength: 0.1-40 MPa
Thermal conductivity: 0.05-0.7 W/mK



Sustainability: Though raw minerals are naturally occurring and non-toxic, the extraction process accounts is carbon-intensive and water intensive. Carbon emissions also depend on the firing process.

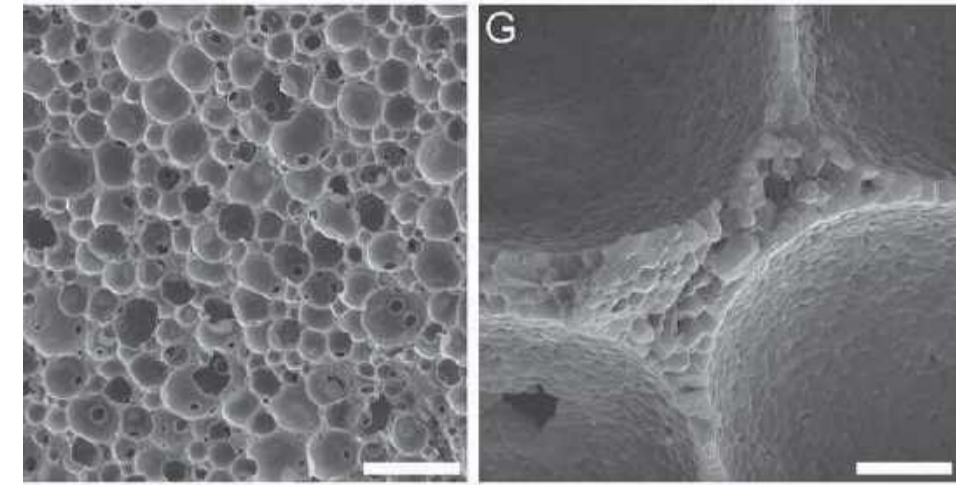
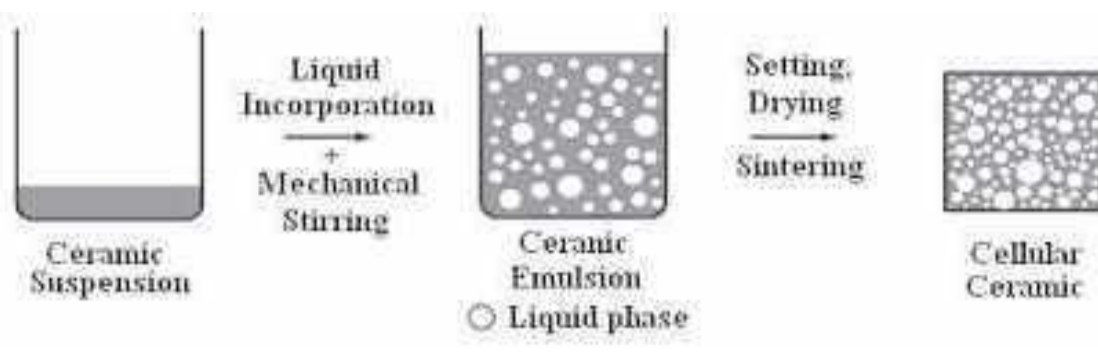
Foam Ceramic Processes

A comparison of the methods and advantages of different aeration methods.

There are two overarching classifications of foams: open-cell and closed-cell. Open-cell foams have interconnected pores and typically lower density, while closed cell foams have fully enclosed pores which have on average higher compressive strength.

Direct Foaming

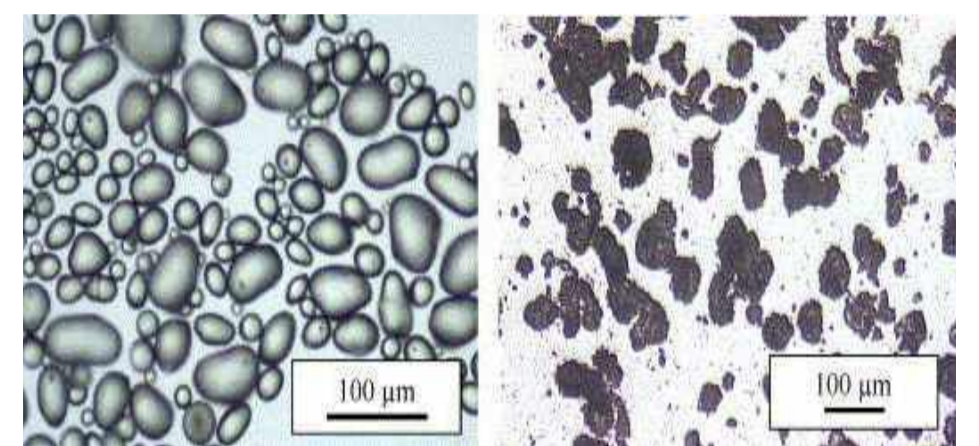
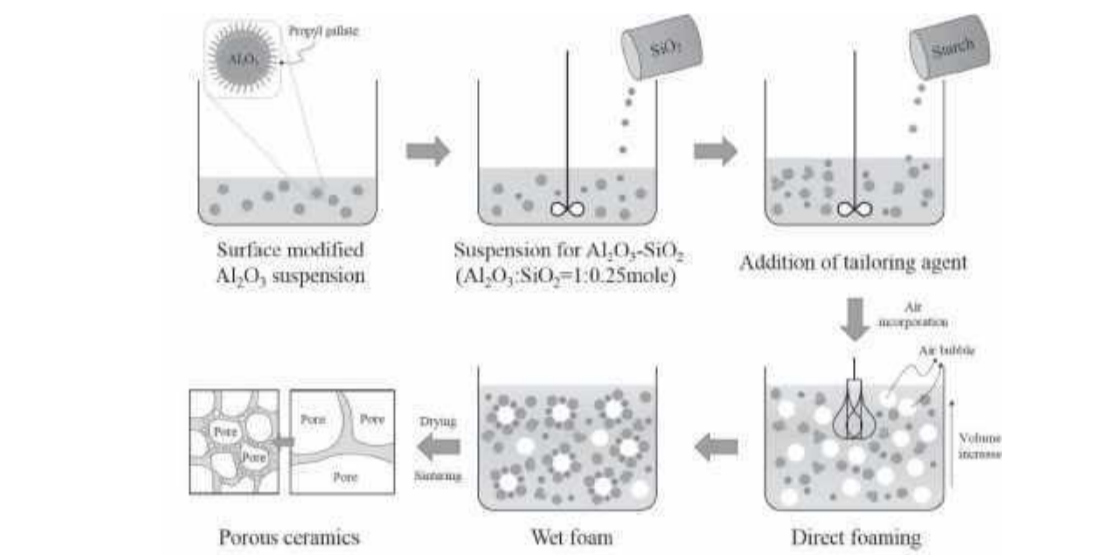
A gas or liquid is mixed into a ceramic suspension, which is then dried and sintered at high temperatures. Direct foaming leads to high mechanical strength and can be done with very low emissions. However, pore size and porosity can be difficult to control. Porosity is roughly equivalent to the amount of gas or liquid introduced, while pore size is proportional to the amount of stirring done in the mixing process.



Sustainability: Very low waste gases and liquids. Emissions affected by stirring method.

Starch Consolidation

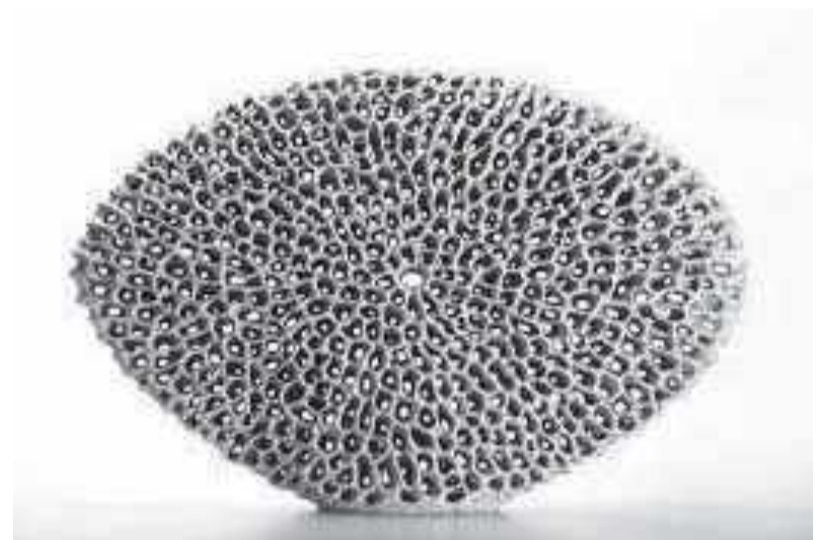
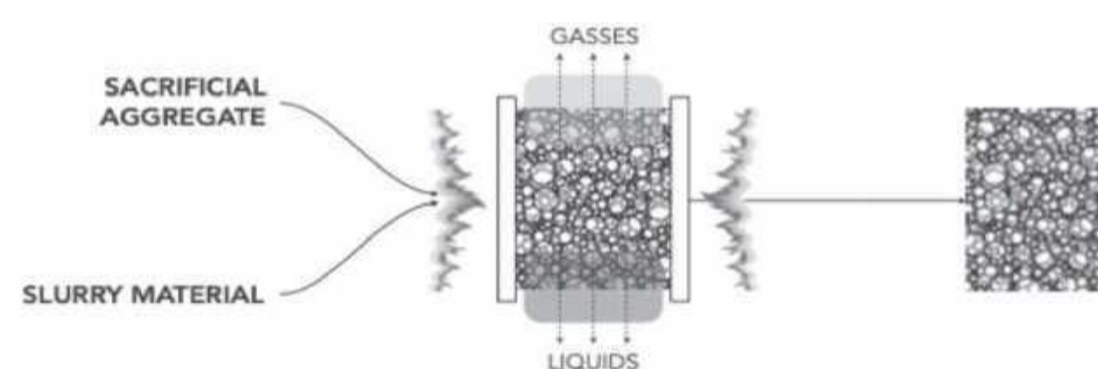
Starch and ceramic powder are suspended within distilled water. The starch is then burned out during sintering. The pore size achieved can be controlled by the amount and type of starch introduced. Starch consolidation is often combined with other foaming methods to control pore size. It is cheap, non-toxic, and environmentally friendly.



Sustainability: Starches can be naturally occurring and are often non-toxic. Emissions affected by stirring method.

Burnout / Lost Material Aeration

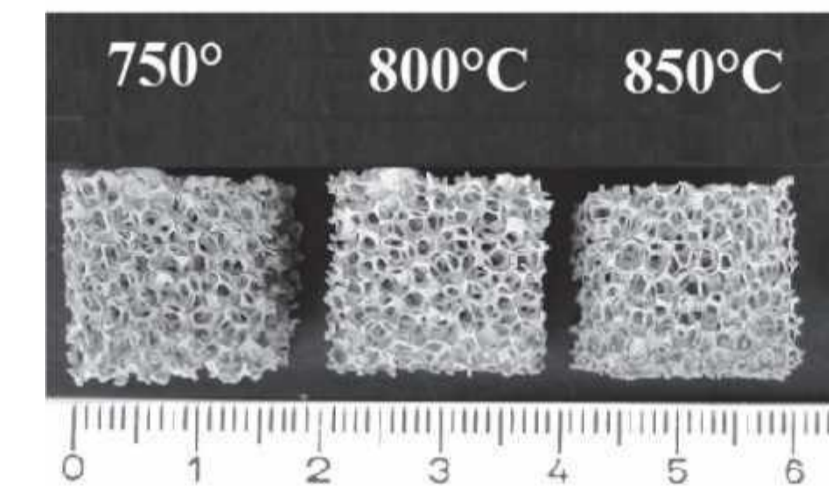
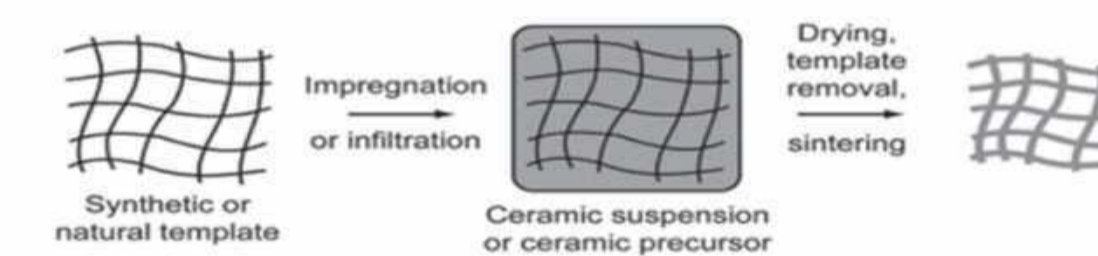
Aggregate (such as gravel, sand, and crushed rock) is mixed into the slip and then burnt out after drying. In ceramics, polyurethane (PU) is mixed into the slip and then burnt out after drying. This technique is common in industrial applications due to its relative simplicity and high thermal insulation.



Sustainability: Aggregate used is often waste material. PU is a plastic derived from crude oil, but is recyclable.

Replication

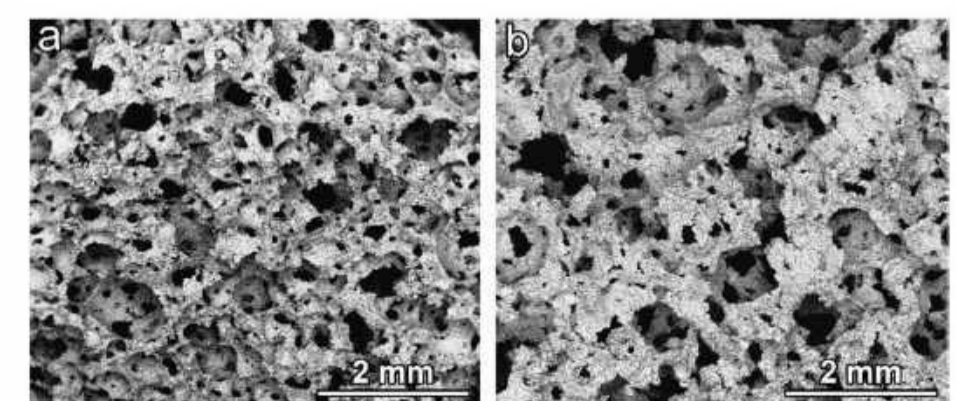
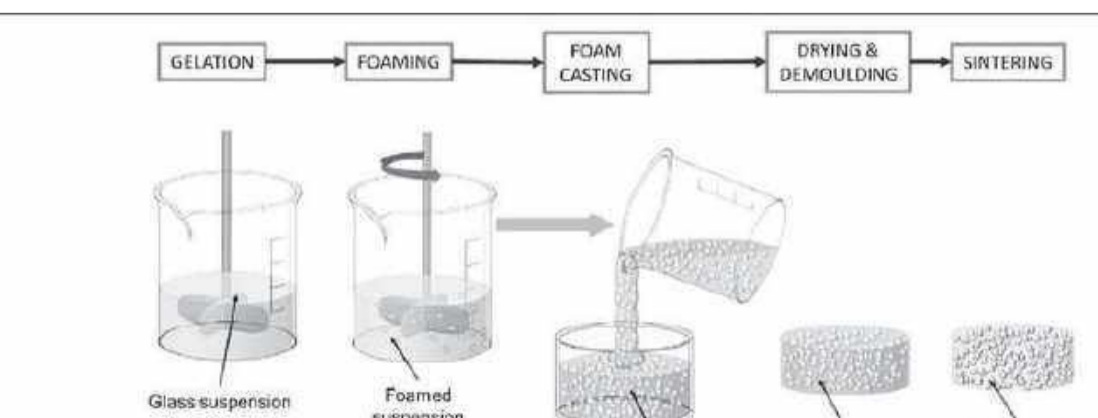
Replication utilizes a polymeric foam (often polyurethane) as a template, which is then dipped into ceramic slurry. After the coated template is dried and sintered, a replica of the original template remains. It is therefore a form of the burnout technique. The replication method's higher permeability comes at the cost of lower mechanical strength.



Sustainability: PU is a plastic derived from crude oil, but is recyclable.

Gel-Casting

Ceramic powder and organic monomers are suspended with solvents and dispersants. This mixture is poured into a nonporous mold, at which point the monomers polymerize and a gel forms. This "green foam" is removed from the mold to dry and sinter. Gel-casting can use organic materials to produce foam ceramics with high homogeneity, fewer geometry limitations, and shorter burnout times.



Foam ceramics



1-Make sludge around 50-50 water to clay ratio.



2-Take sponge.



3-Dip sponge in.



4-Leave it to dry for a week.

Case Studies in Composite Structures

Drawing inspiration from existing forms.

SkeleTHon

Steel Fiber-reinforced Concrete + Waterproof Concrete

This fully functional, 114 kilogram canoe uses a unique Free Formwork skeleton to minimize material and maximize stiffness. In this design, a 2-3mm waterproof concrete skin covers a 1mm fiber-reinforced concrete skeleton. High-resolution surface texture increases contact area between the two.



SS Atlantus

Ferrocement + Steel

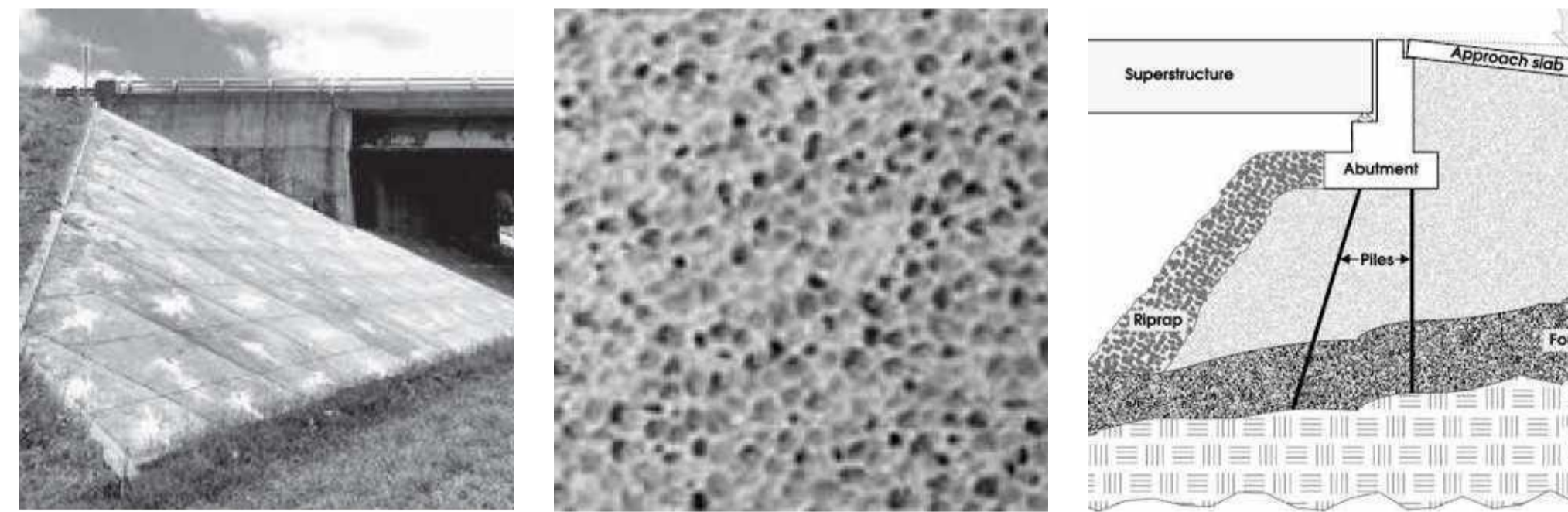
This concrete ship is the most famous in a series produced during World War I, as concrete was easier to acquire and less expensive than steel. A ferrocement hull layers wire mesh with mortar for high durability. This hull is then reinforced with steel bars. Concrete ships were removed from the navy because their thick hulls compromised hydrodynamics and cargo space.



Bridge Approach

Foamed Concrete

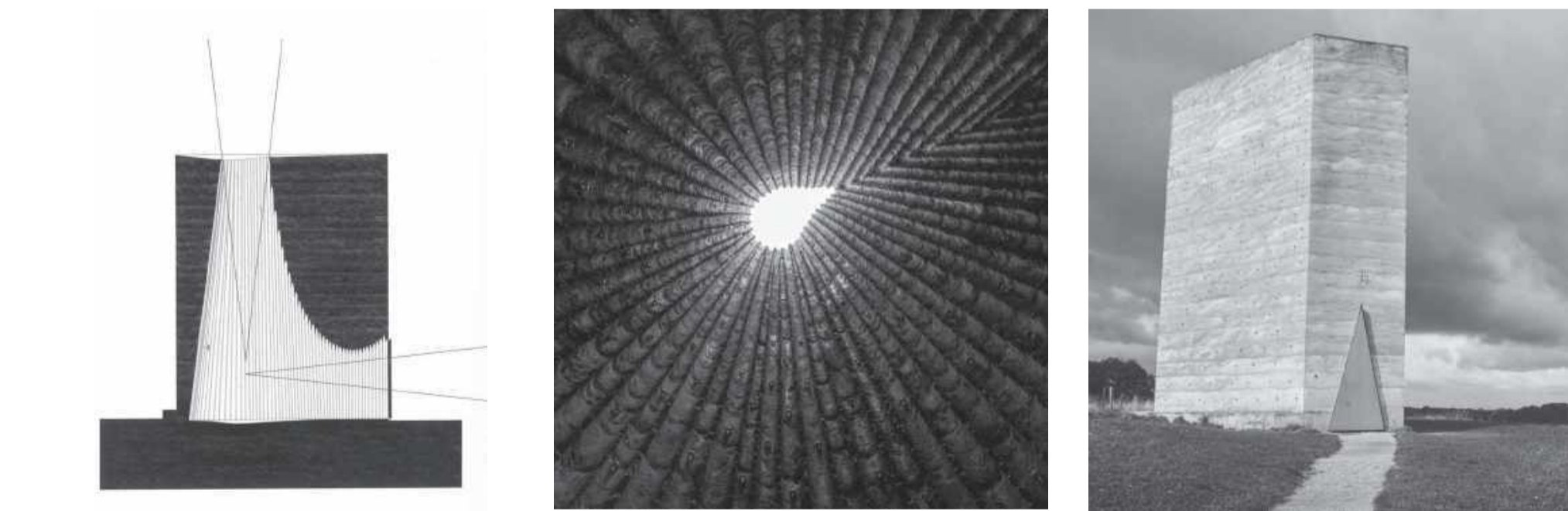
Recently, fiber-glass reinforced foam concrete has been used in railway bridge approaches. This technique has been shown to improve performance by decreasing effects of settlement and displacement on the railway.



Burder Klaus Field Chapel

Concrete, Lead, Linden Wood, Crystal Glass Marbles

Architect Peter Zumthor poured 24 layers of concrete around a teardrop-shaped wigwam frame of 112 tree trunks. Once the concrete had set, the trunks were slowly burnt out for three weeks, leaving behind a ridged and blackened interior. The cavern is open to the sky and the floor is composed of lead.



Duocel Heat Exchanger

Silicon carbide (ceramic) foam

A typical heat exchanger passes two fluids through a series of pipes or fins to transfer heat from one to the other without transferring liquid: A Duocel foam heat exchanger replaces these pipes fins with Duocel ceramic foam to increase surface area and turbulent flow. The ceramic foam's high thermal conductivity also allows for efficient heat transfer.

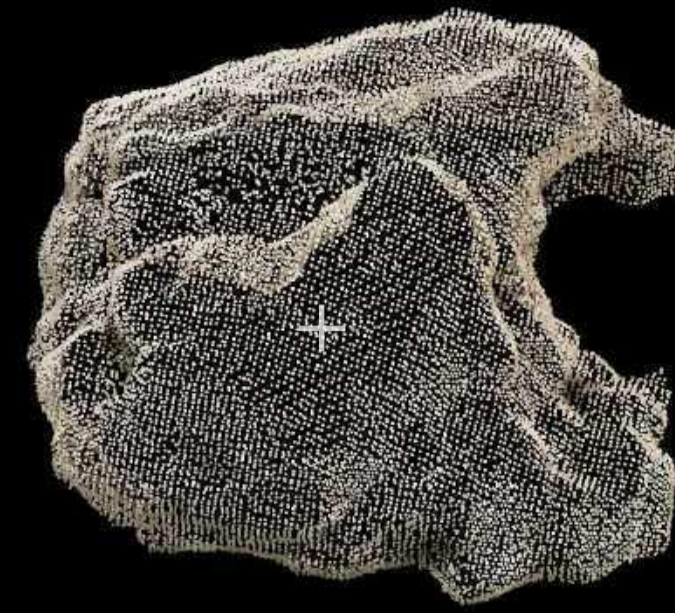
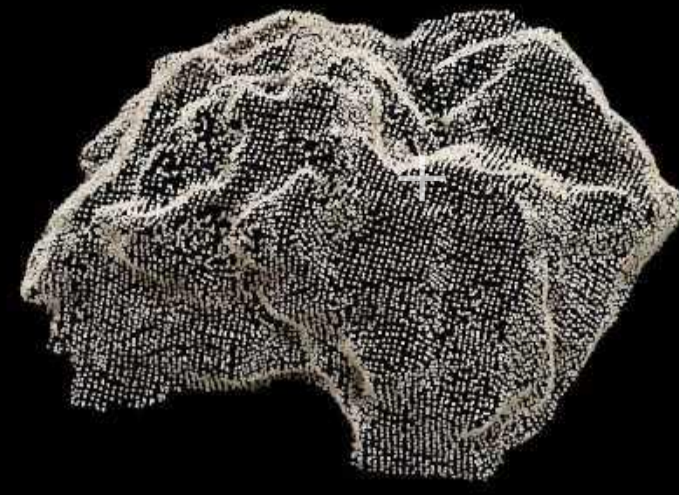


NO COATING

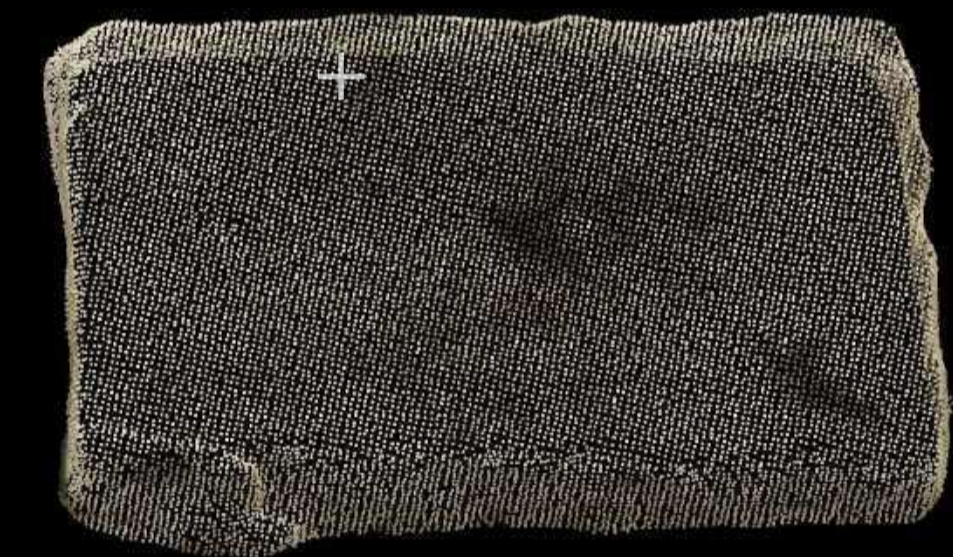
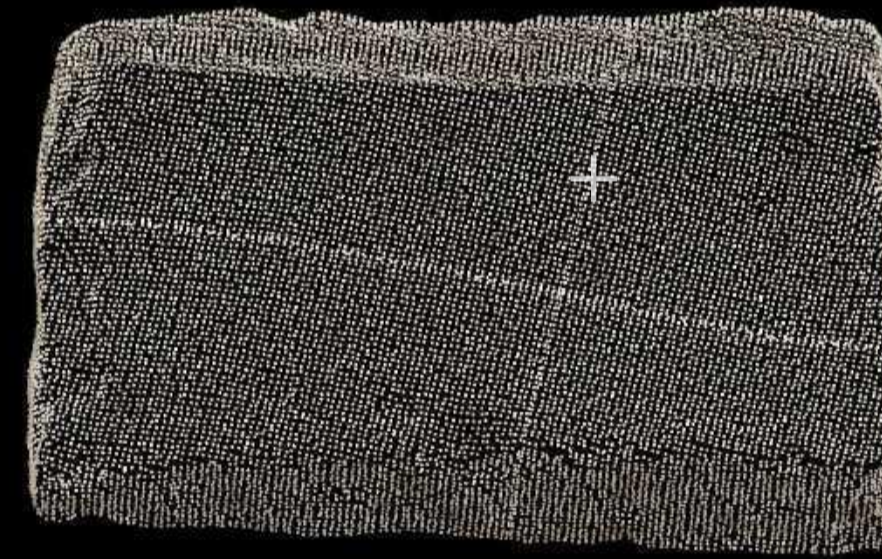
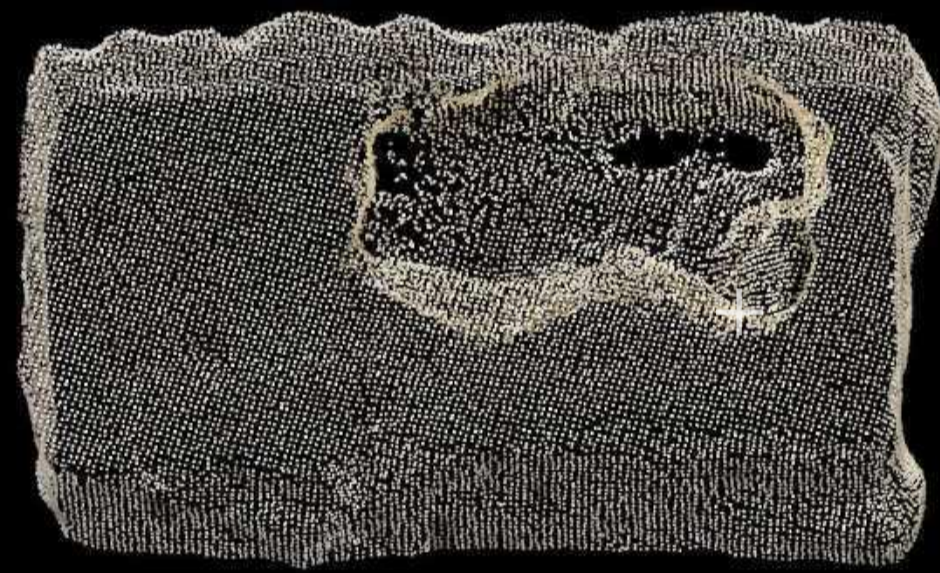
ACRYLIC COATING

BIO PLASTIC COATING

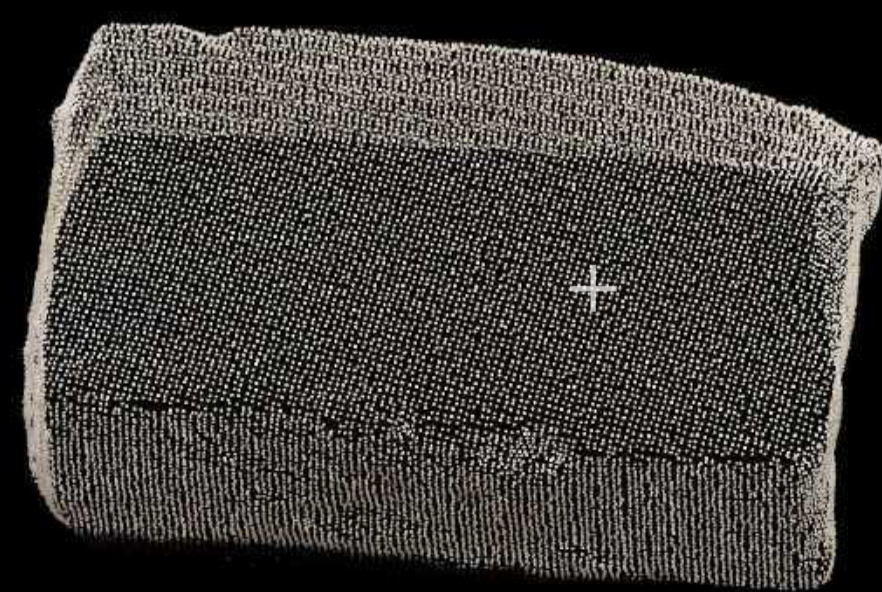
POROSITY 60%



POROSITY 50%



POROSITY 40%



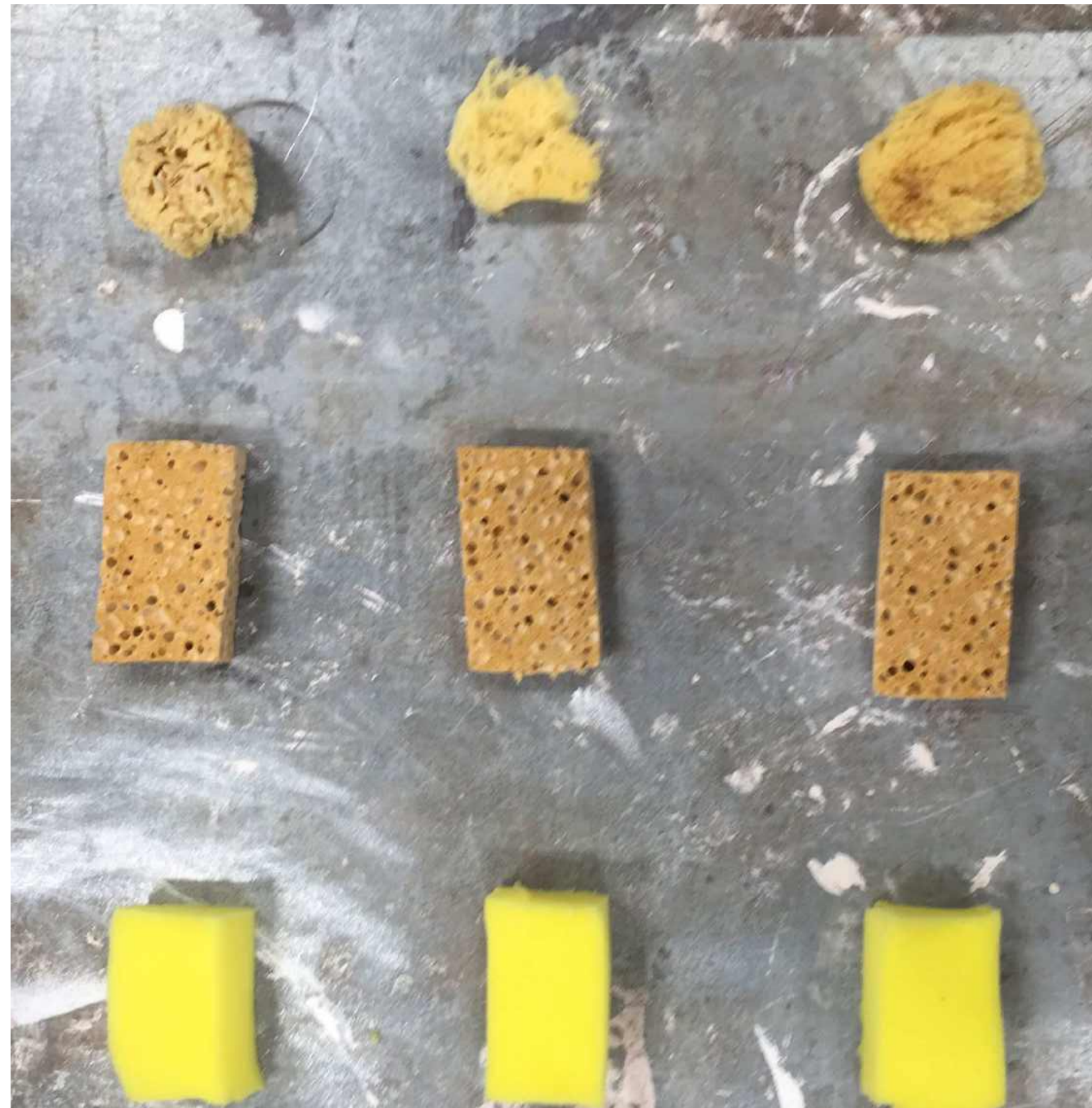
COATED FOAM CERAMICS SAMPLES

Manufactured Prototypes

Slip Making and Dipping



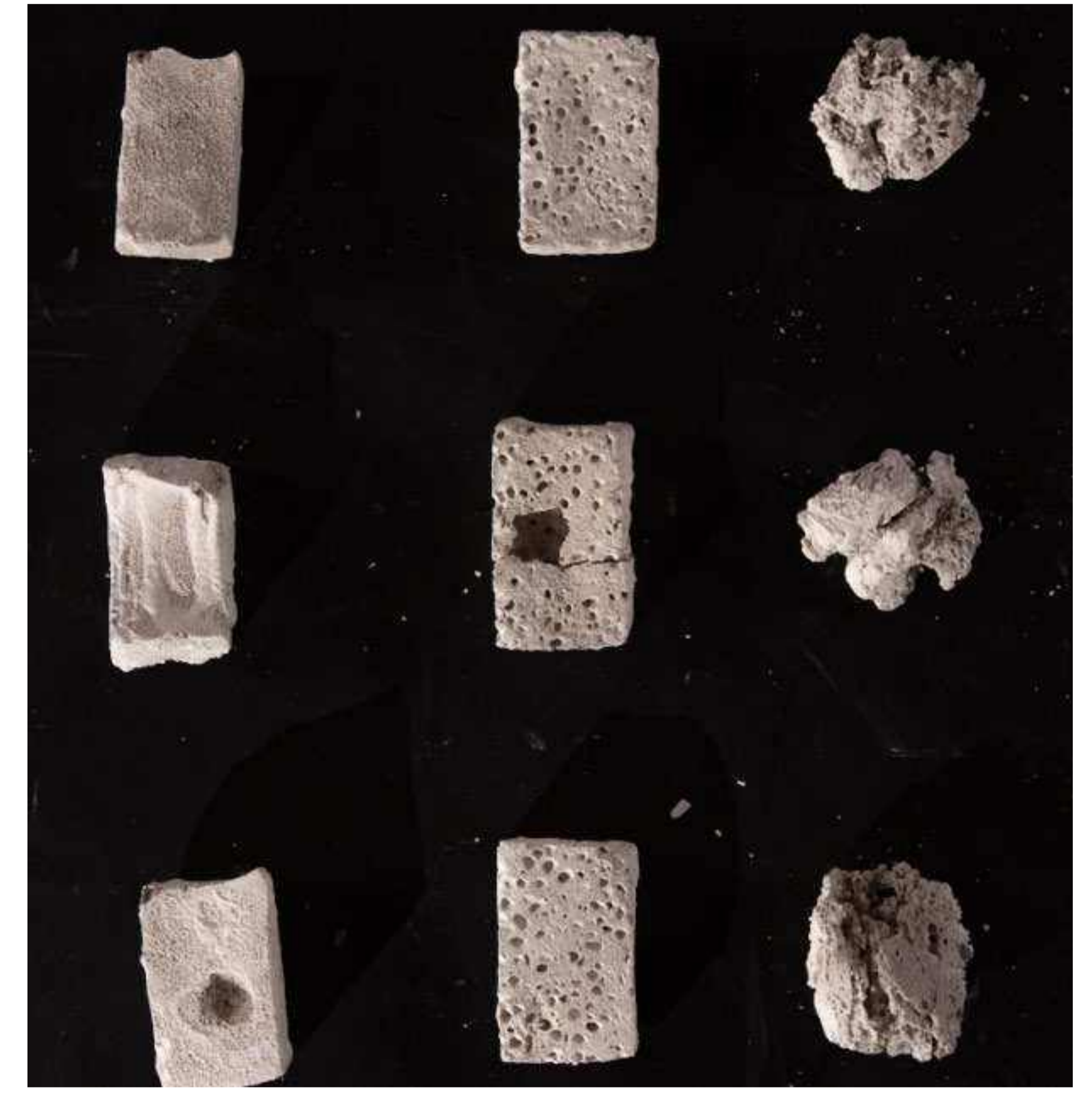
Sample Manufacturing



Sponges chosen with different porosities.



Dipped into slip clay.



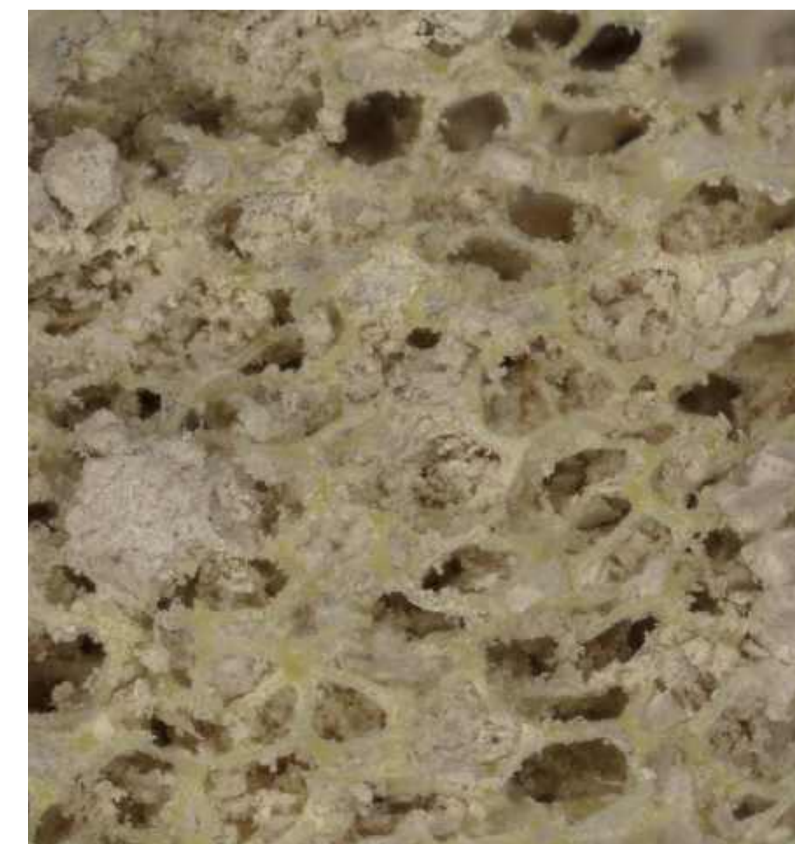
5+ hours of bisque firing.

Failed 1st sample

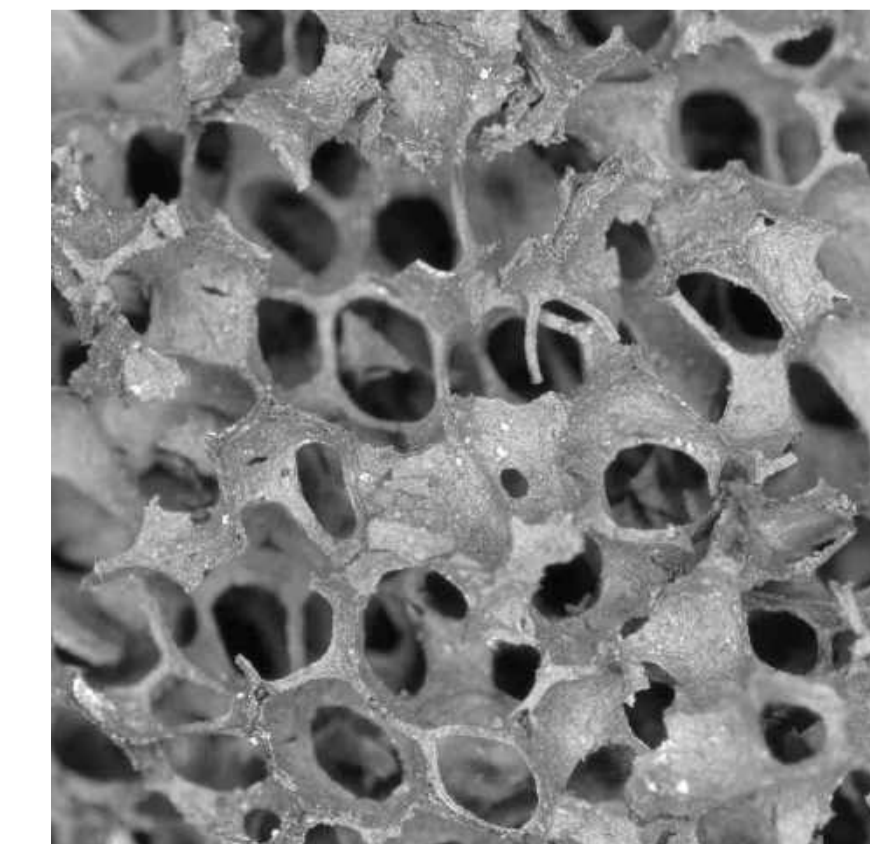


Propane nozzle did not have enough pressure. It resulted in low temperatures.

Successful Results



AFTER (under microscope)



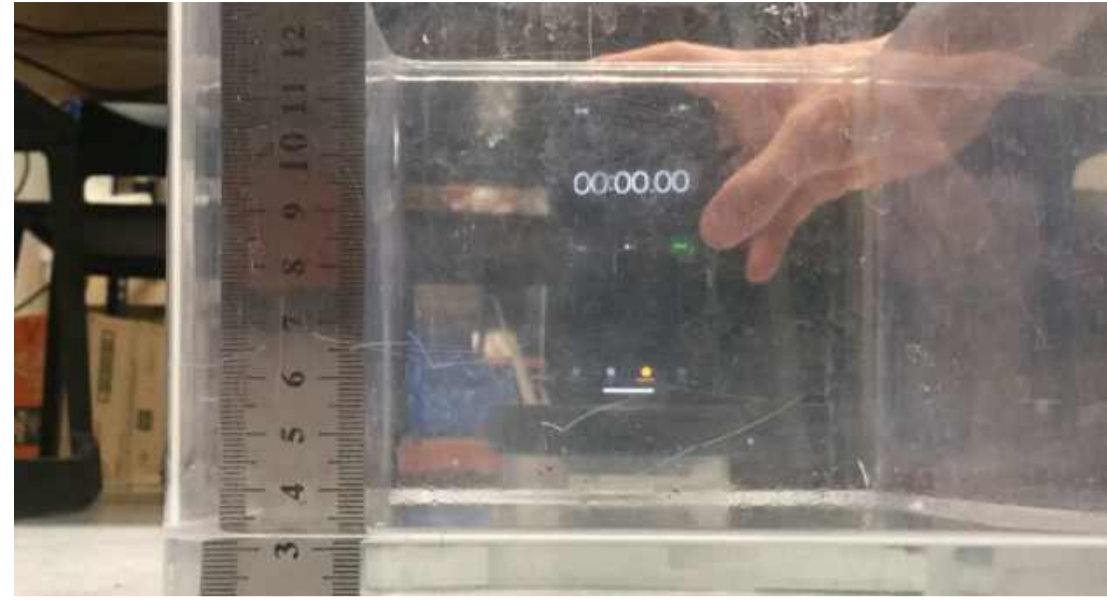
BEFORE (under microscope)

Conclusion

The burn out technique worked well, now we can test and see what use we can have from these materials.



Buyoancy Experiment

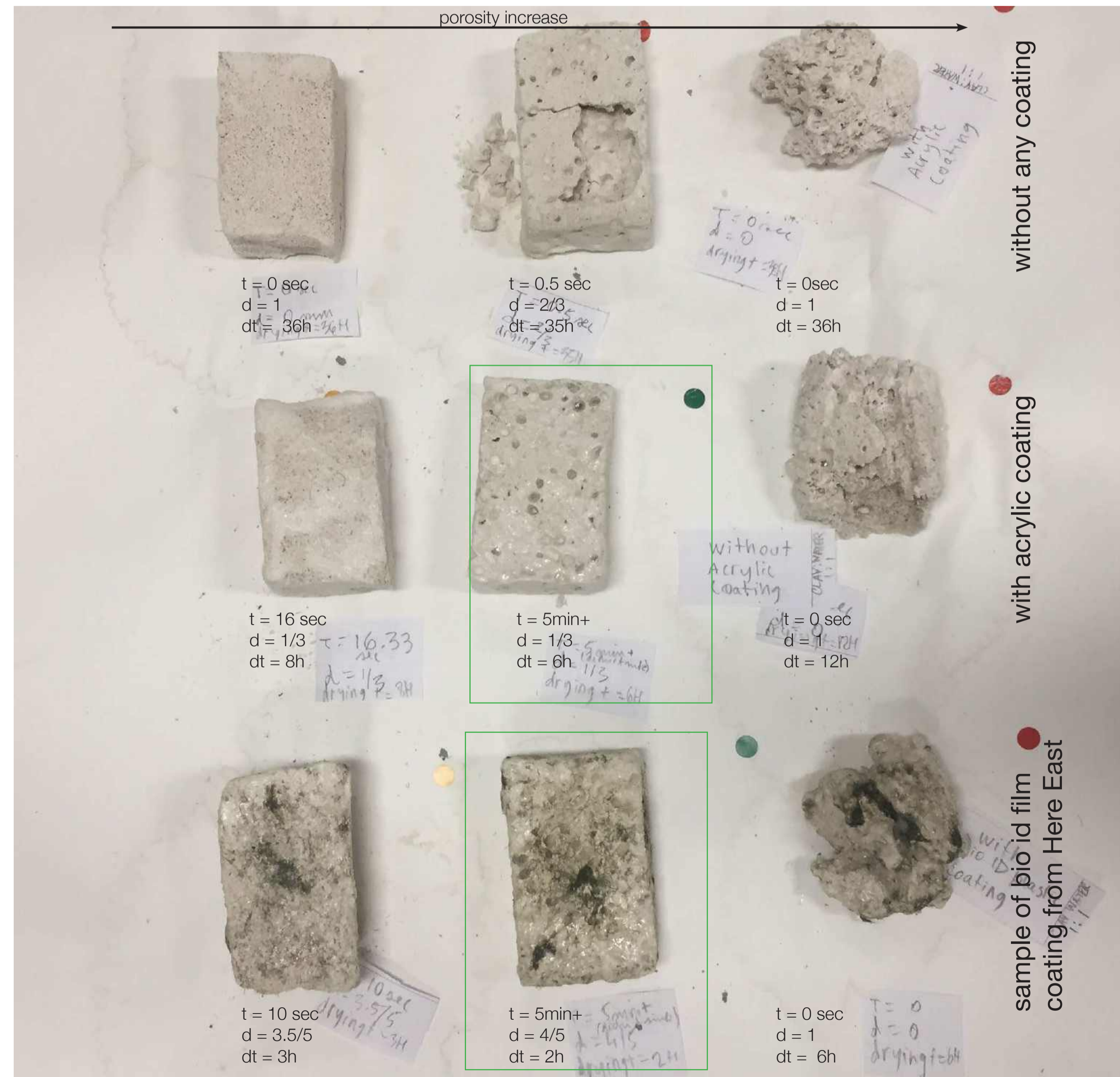


The experiment went well for two samples, which will therefore be later used to create a pull buoy and maybe pantoons on inbetween spaces on the bridge.



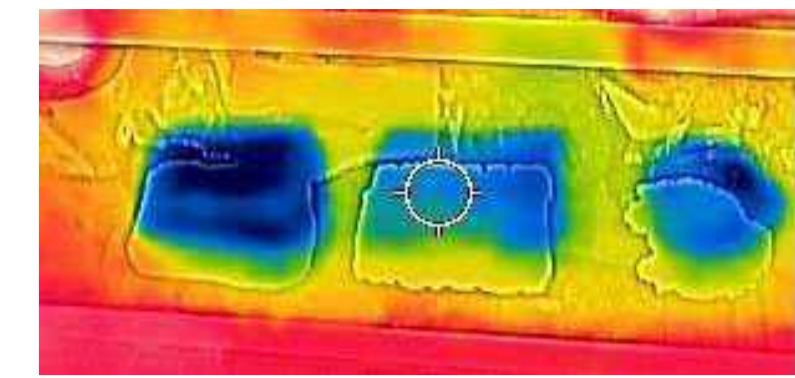
LEGENDS:

t = time afloat
d = depth of sample
beneath surface of water
dt = drying time

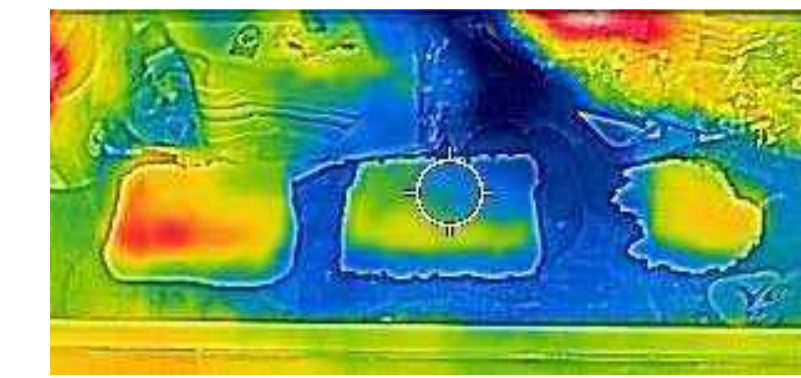


Insulation Experiment

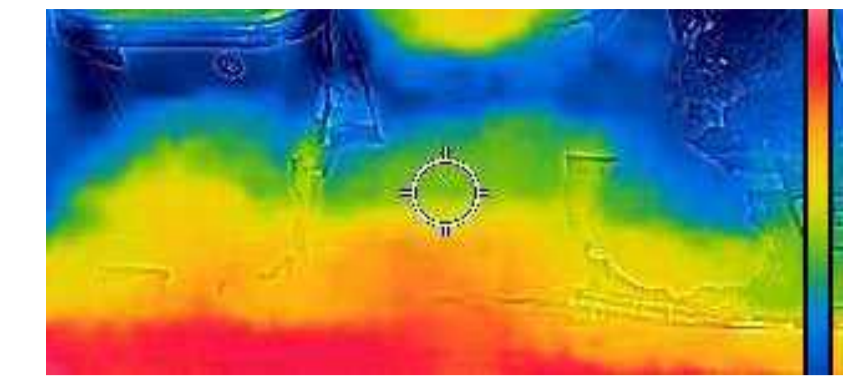
foam ceramics



After 15 min

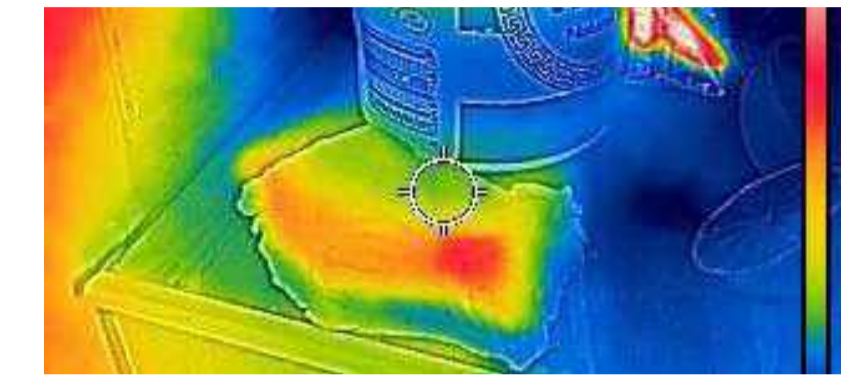
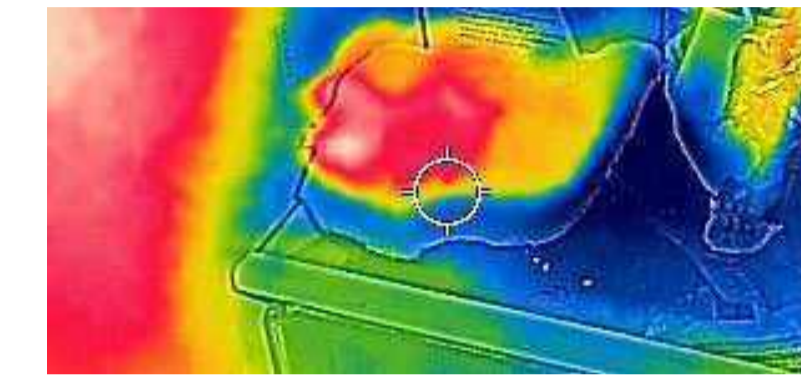
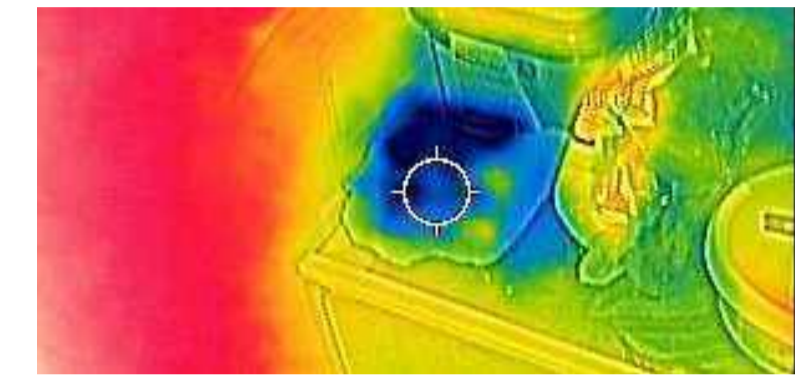


After 30 min

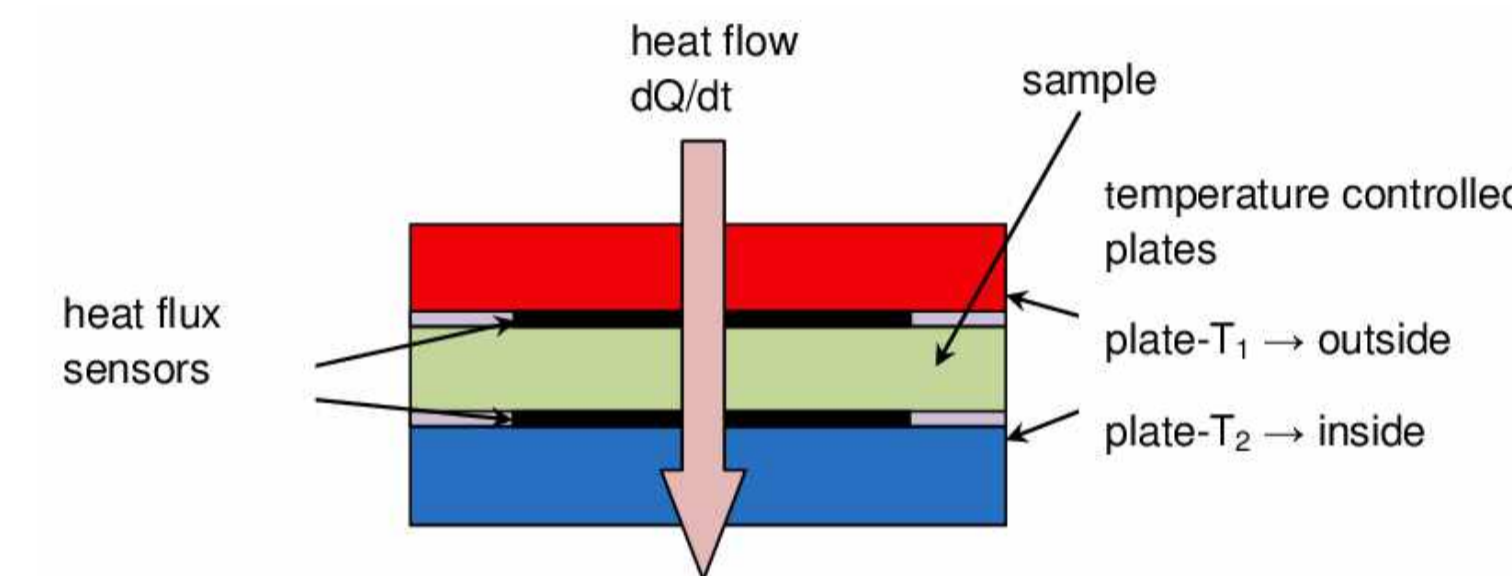


After 1h

standard insulation



All 4 samples were put in the freezer to reach a stable temperature. Then they were all put in the fridge to see how fast they let heat in. The foam ceramics reached the fridge temperature much faster than the standard insulation which was predictable. It can be used but will need to be much thicker than standard, or can help out to be put in cracks and stay there for longer than standard insulation.



freezer



Fridge

Loading Experiment



All the samples are extremely brittle, and can withstand a force only with a coating.



Coating

Shape Building

Having defined the material, the shape can be changed and manufactured. Bits of sponge were pushed into a slip casting mold. Two techniques were tried, where one was break the sponge first and the other push it in.



Technique 1 (2 steps)



Technique 2 (1 step)



shape 1 pre fire



shape 2 pre fire



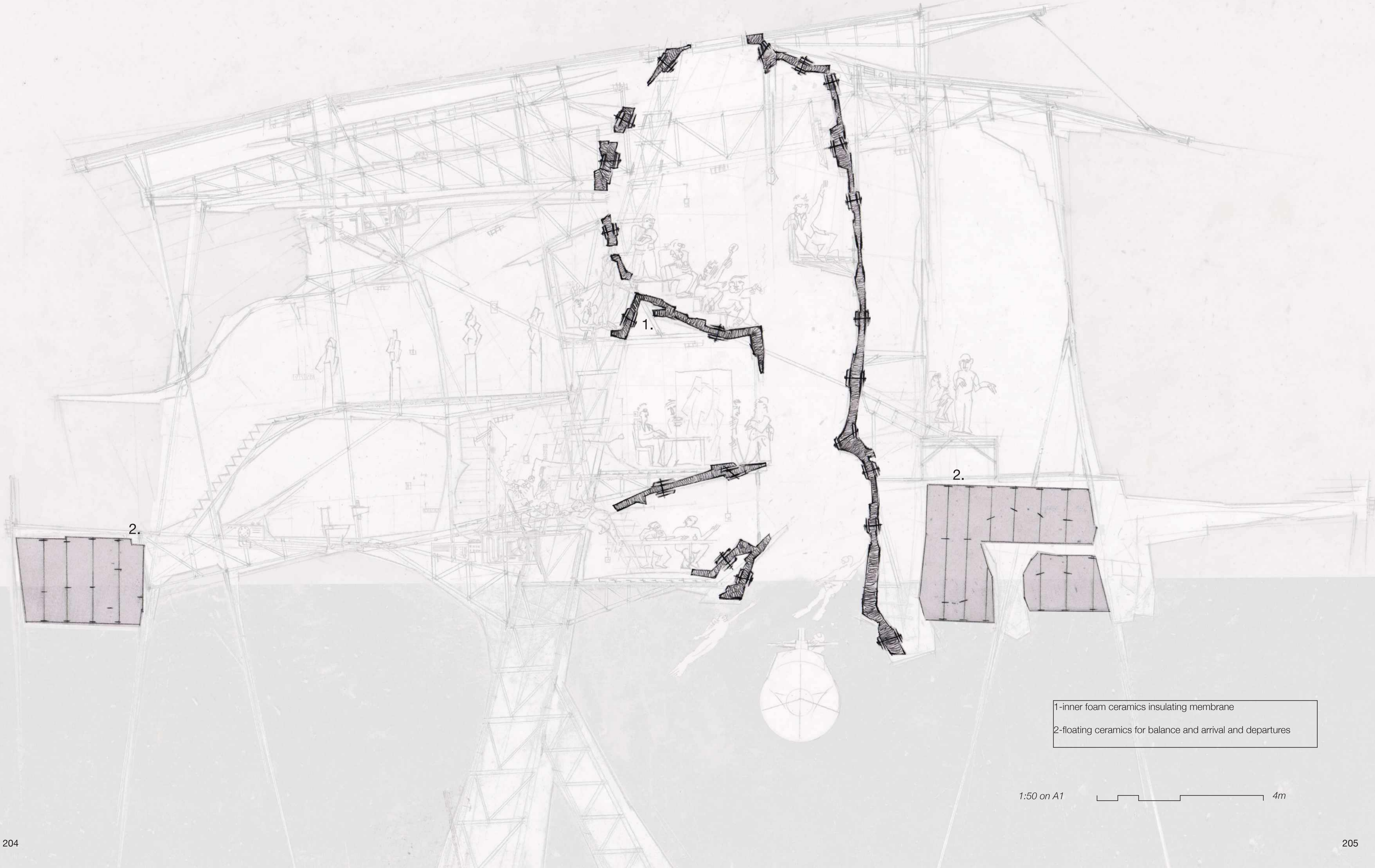
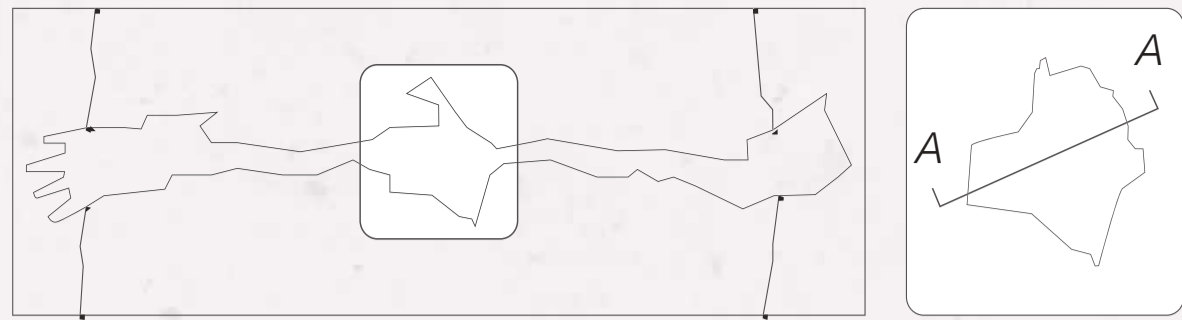
shape 2 post fire



shape 2 post fire

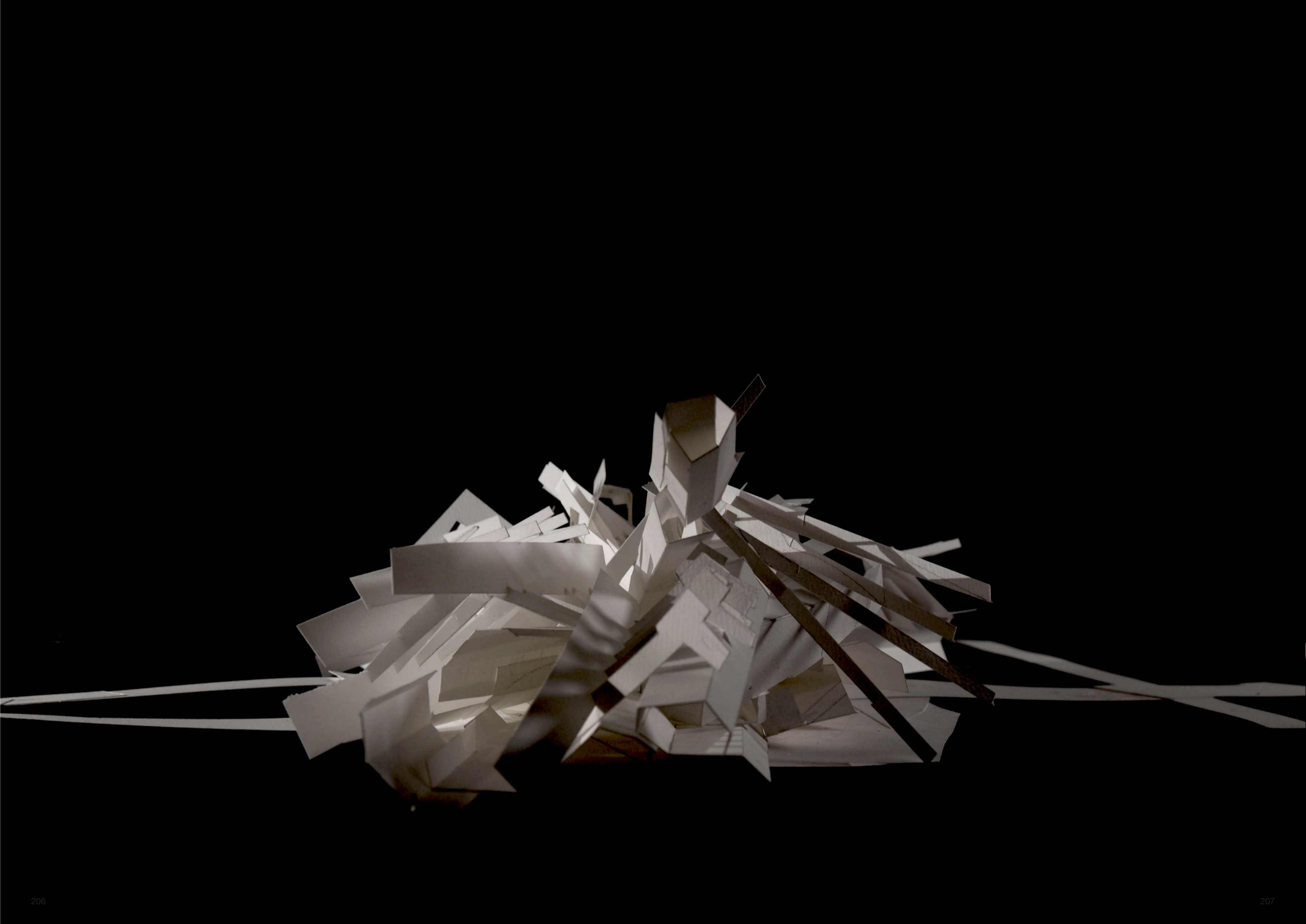
In conclusion the mold burn out shaping worked well, and next time the consistency of the sponge to slip ratio will need to be more carefully monitored.





1-inner foam ceramics insulating membrane
2-floating ceramics for balance and arrival and departures

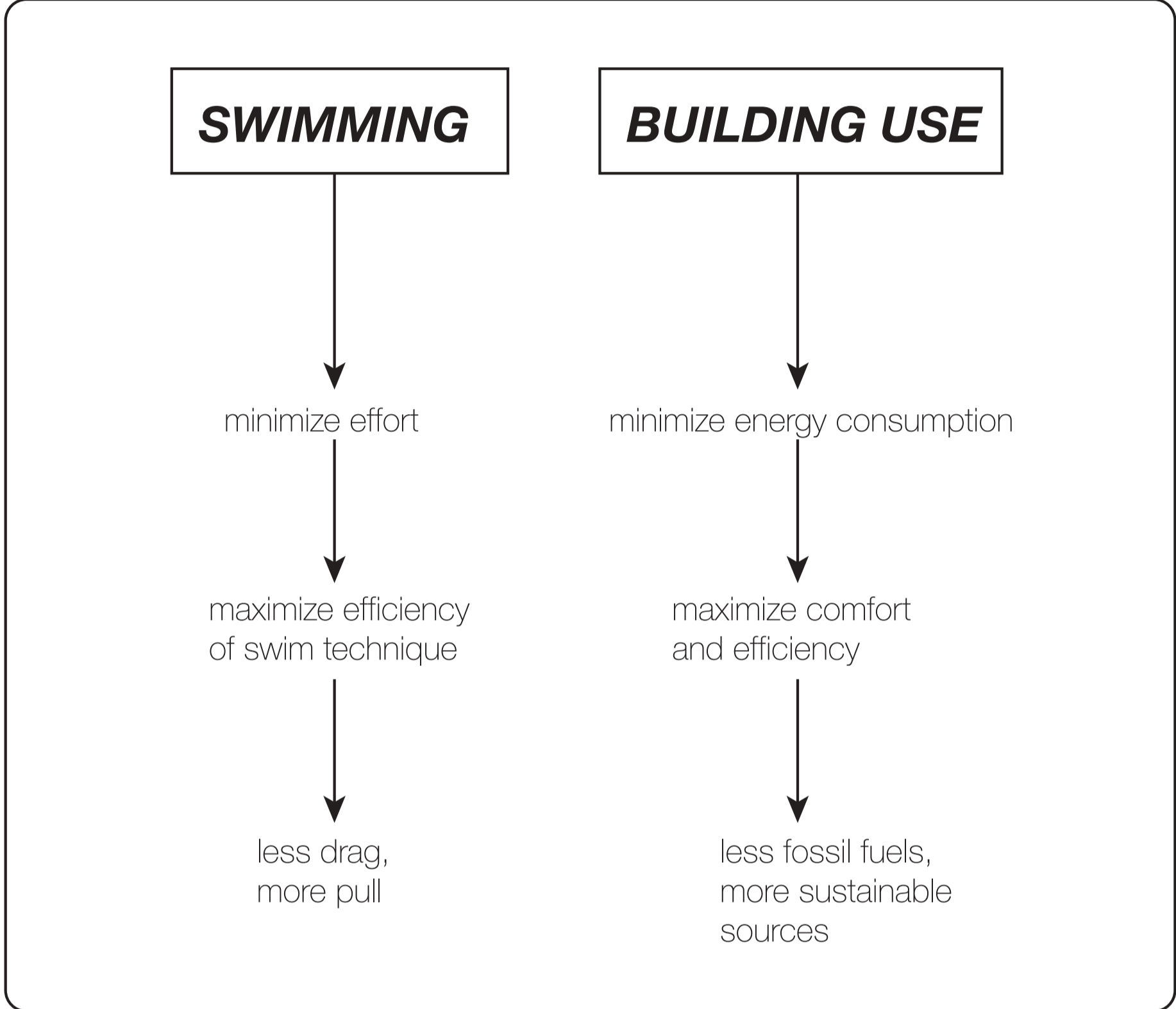
1:50 on A1  4m





Section 11. Passive Measures: Swimming

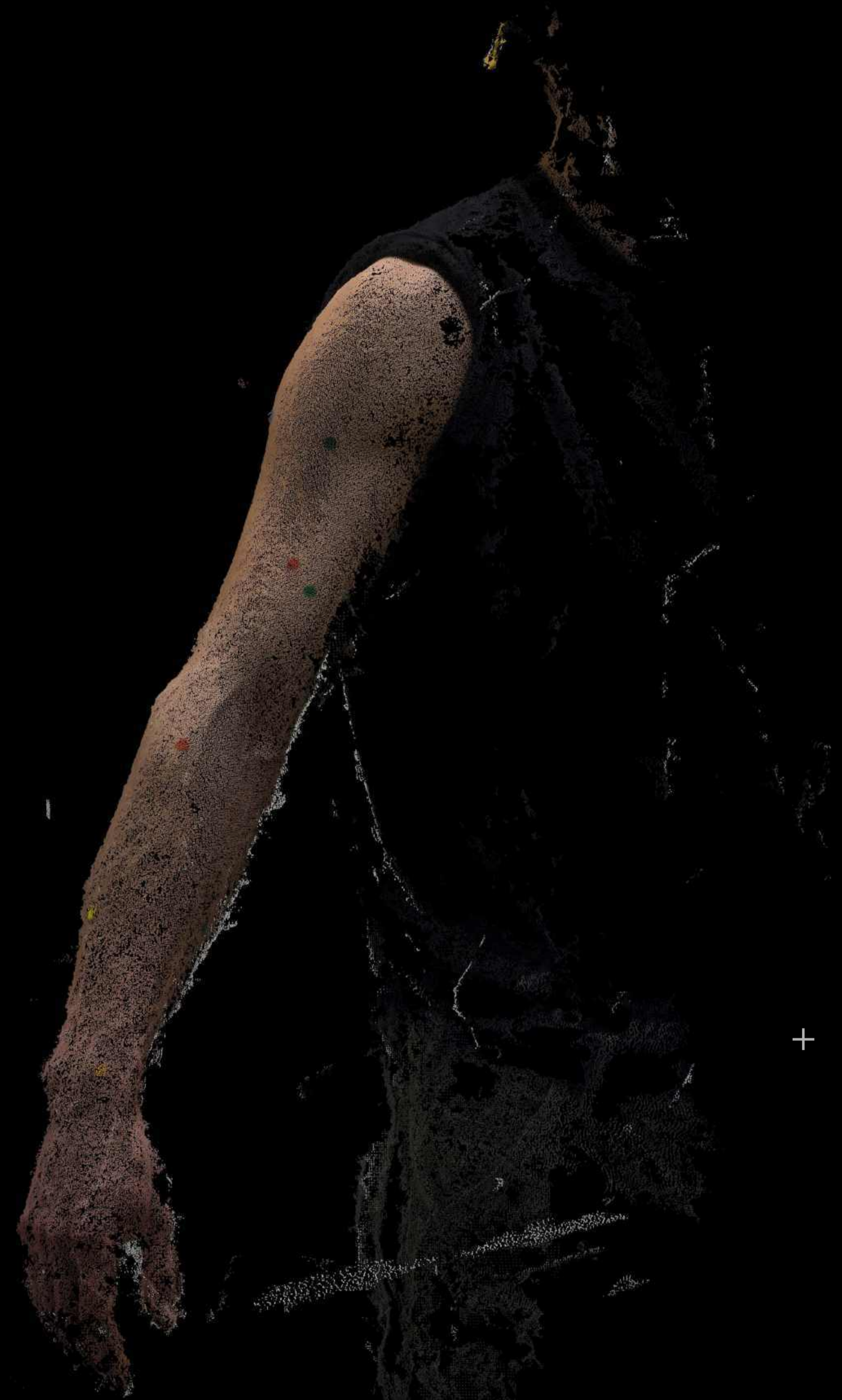
Despite its incomplete nature, this section is necessary for the project and its overall picture as a whole. Swimming was looked into as an activity for both the body and the soul: its main driving factor is that the person swims by maximising the power and minimising the effort. This was mainly to be used as an analogy to passive environmental measures. Unfortunately, it was only carried out in depth on a three dimensional scanning aspect: the body – which was me – was scanned using photogrammetry and a handheld scanner, it became a project in itself. It is something that I would call an architectural scanning folie, or folly.



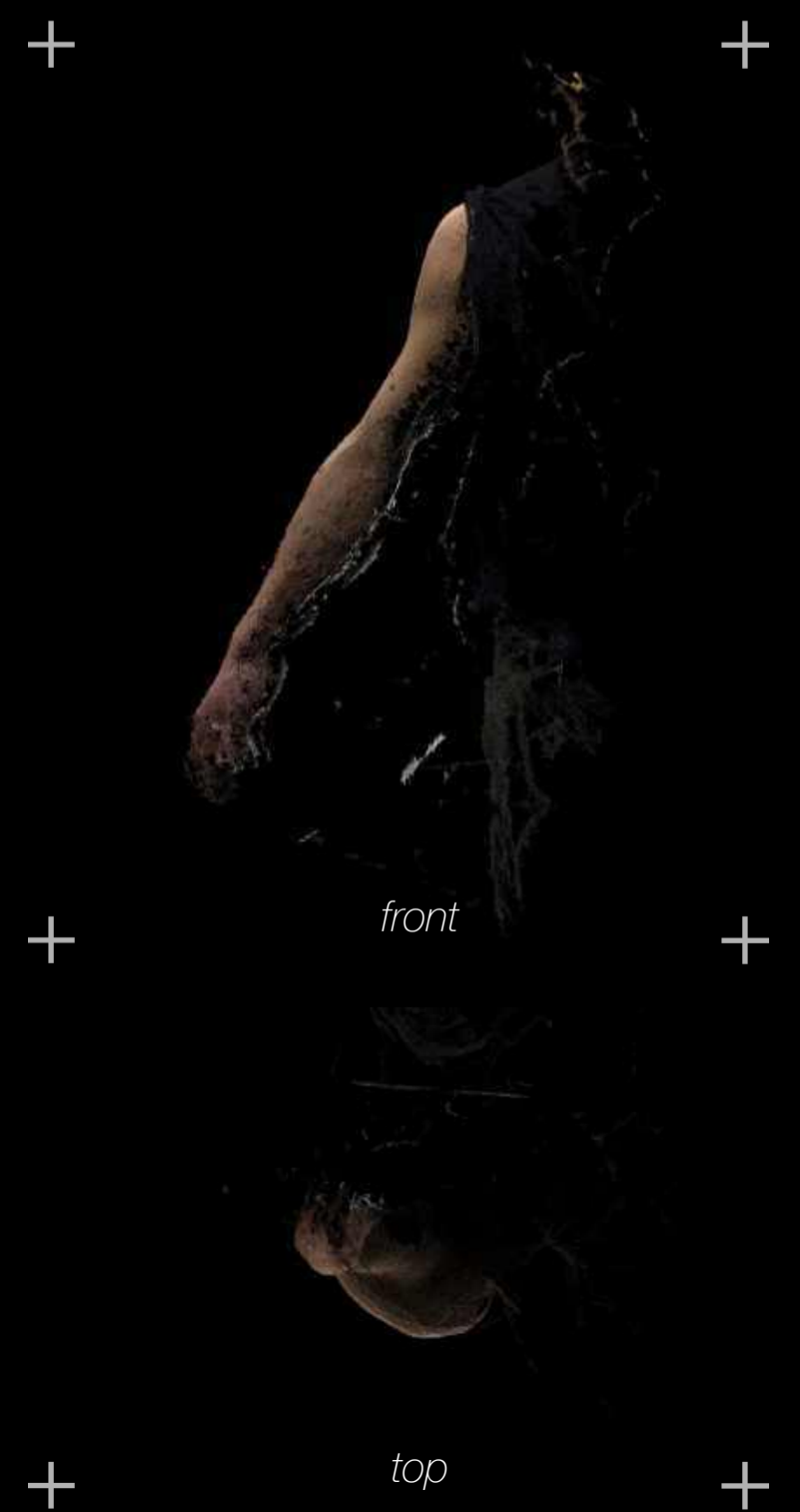
La Thamise

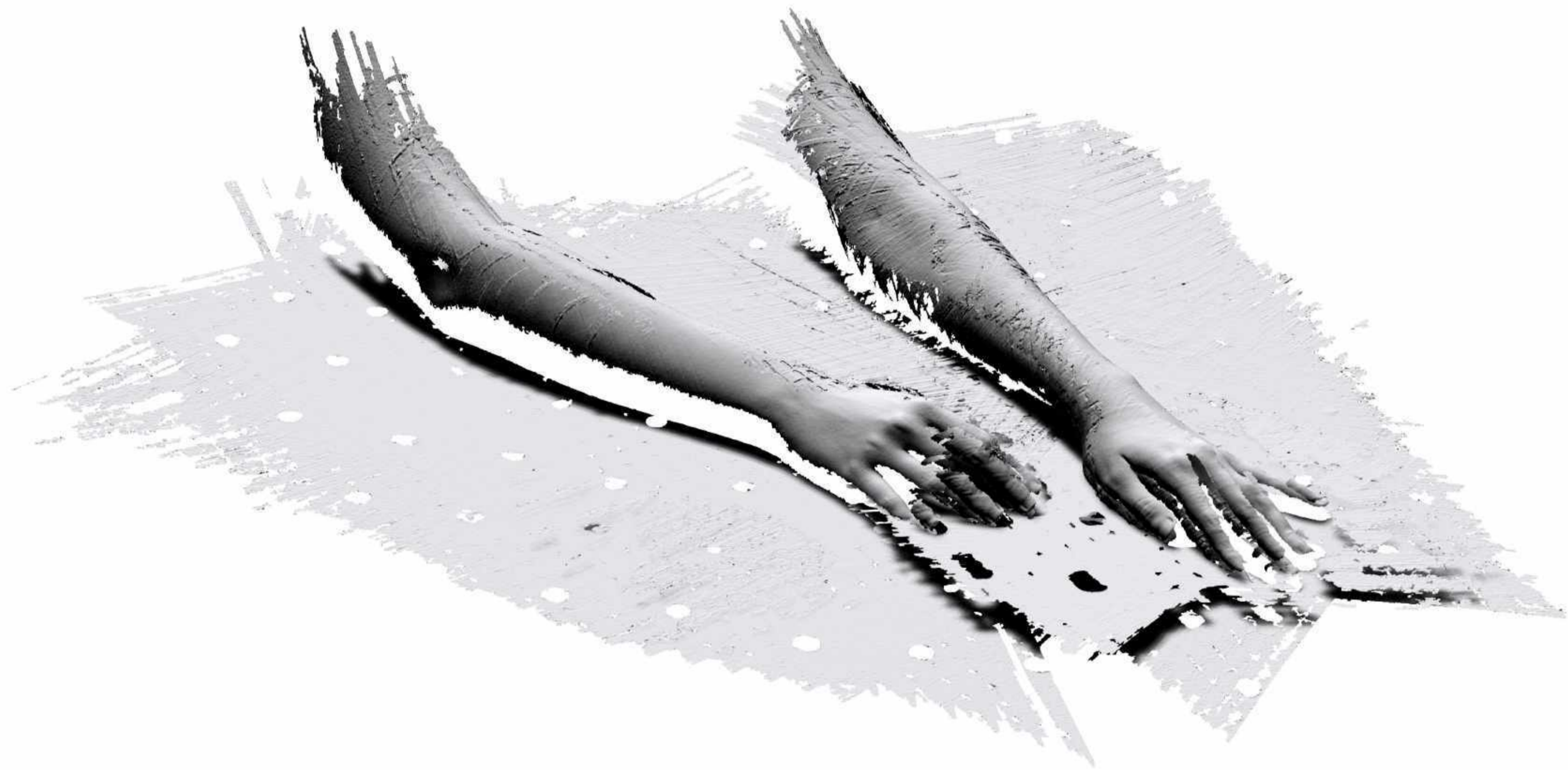


BODY



ARM





right



top

TWO ARMS

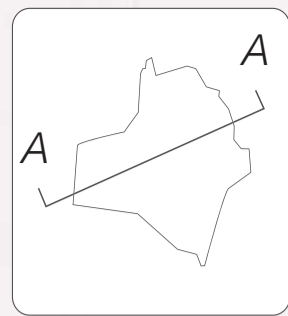
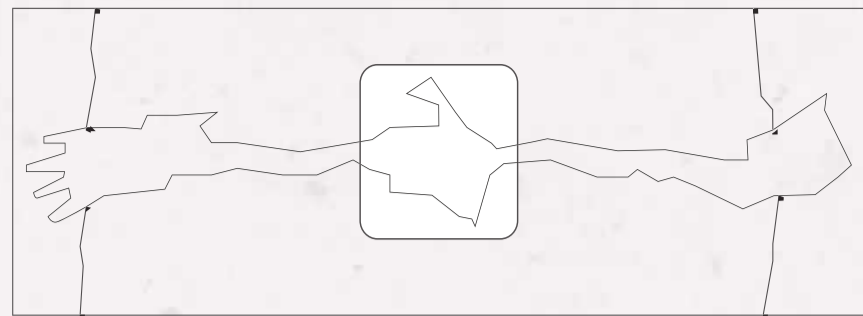


right



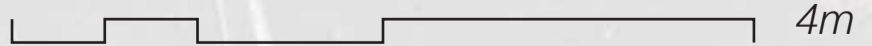
top

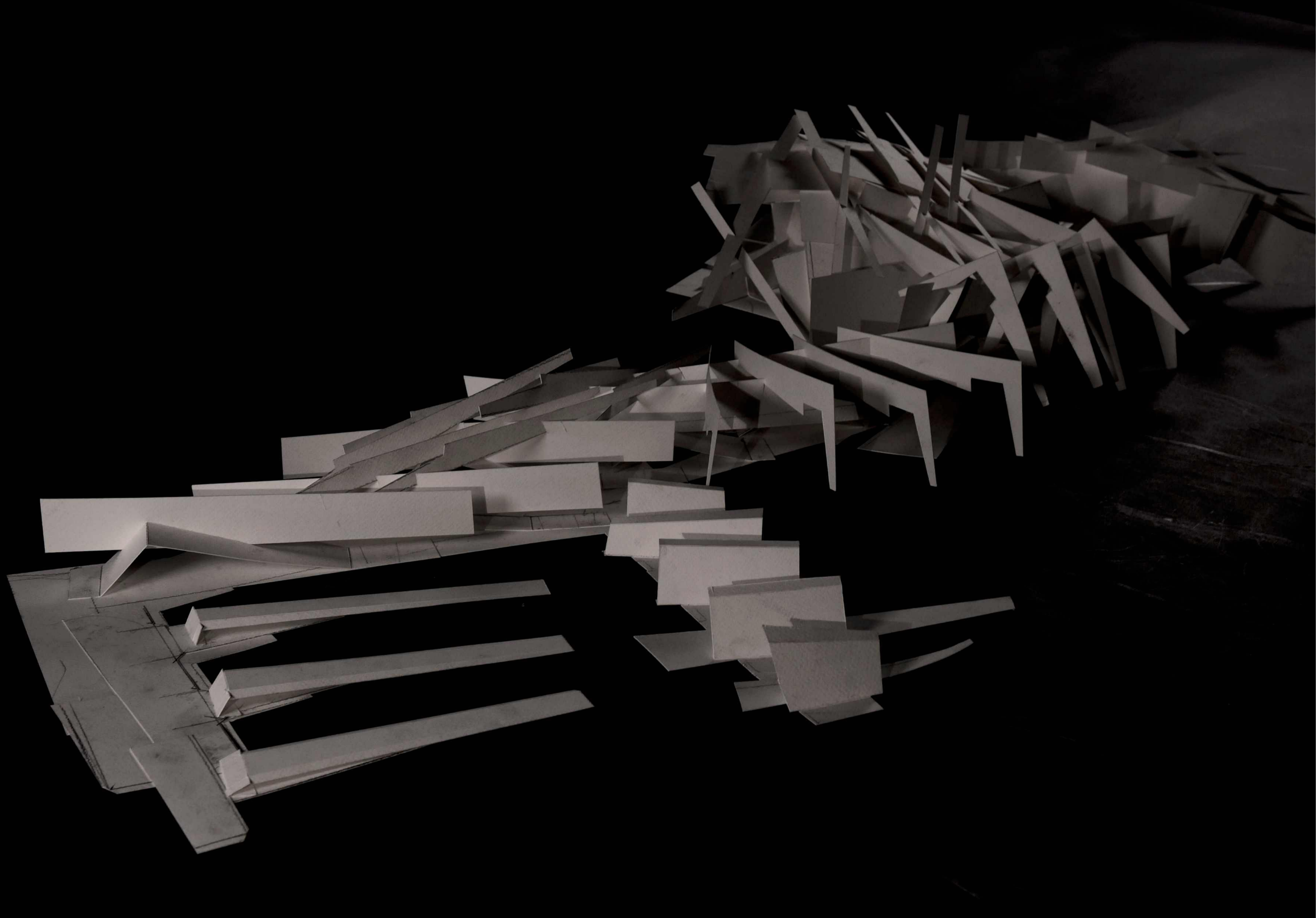
LEG

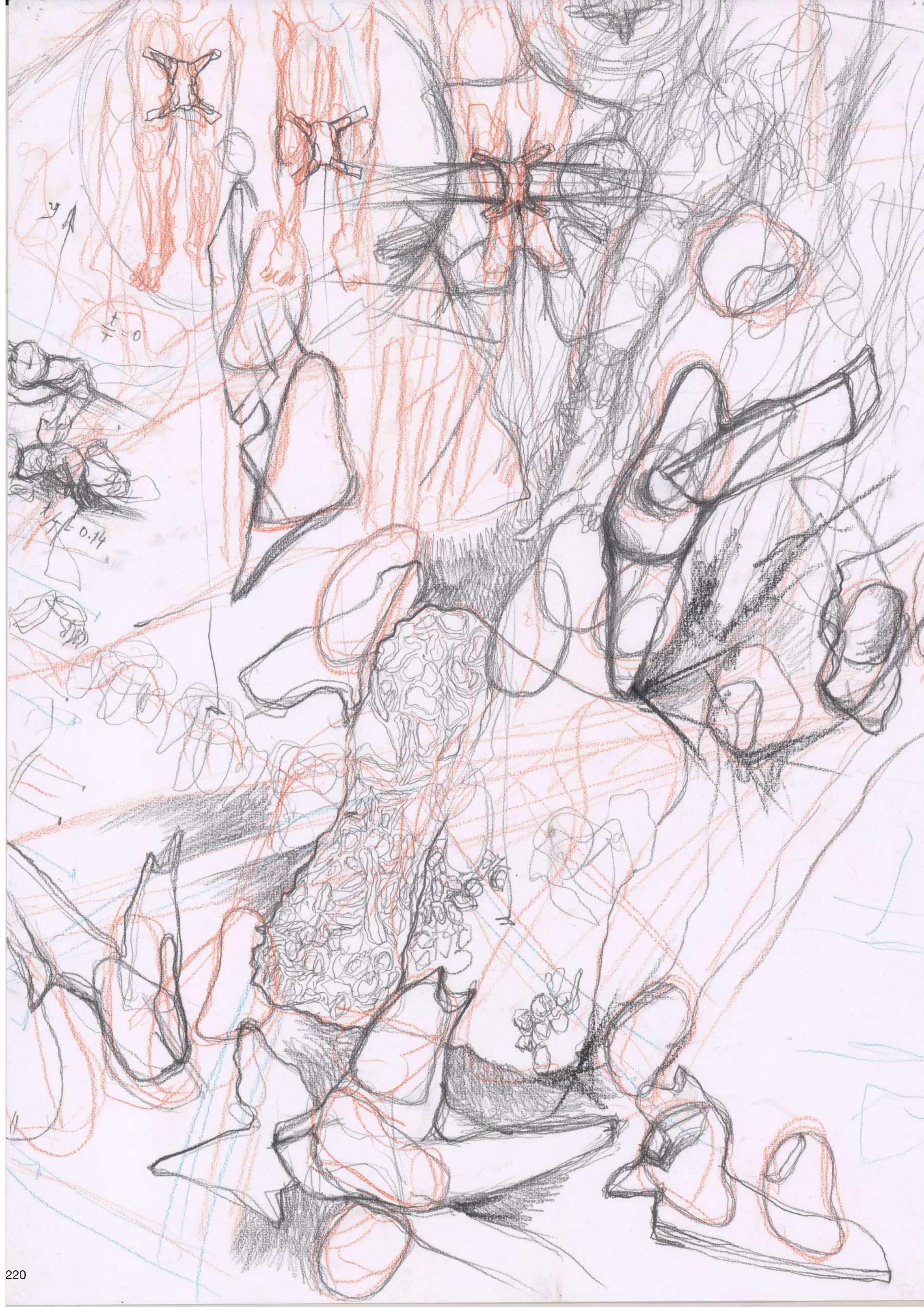


1. Delivering the art through swimming

1:50 on A1







Section 12. Fragment Design: Device

Continuing with swimming, the fragment design was based on a device that was linked to swimming. A device that would help people in the water as it contained the elements of buoyancy and relating to section 10, the swimming device would be created with the foam ceramics that would be upscaled. Then there would be a building fragment, a clamp to hold the clay coatings surrounding the metal. One of the techniques used would be the burnout technique in accordance with slip casting, which would non-conformist geometries that are still produced in a controlled manner. I found it very important that it related to the strain test, and the swimming device can be looked as another project within a project, closely interlinked with section 11, passive measures: swimming, as well as section 10 concerning foam ceramics.

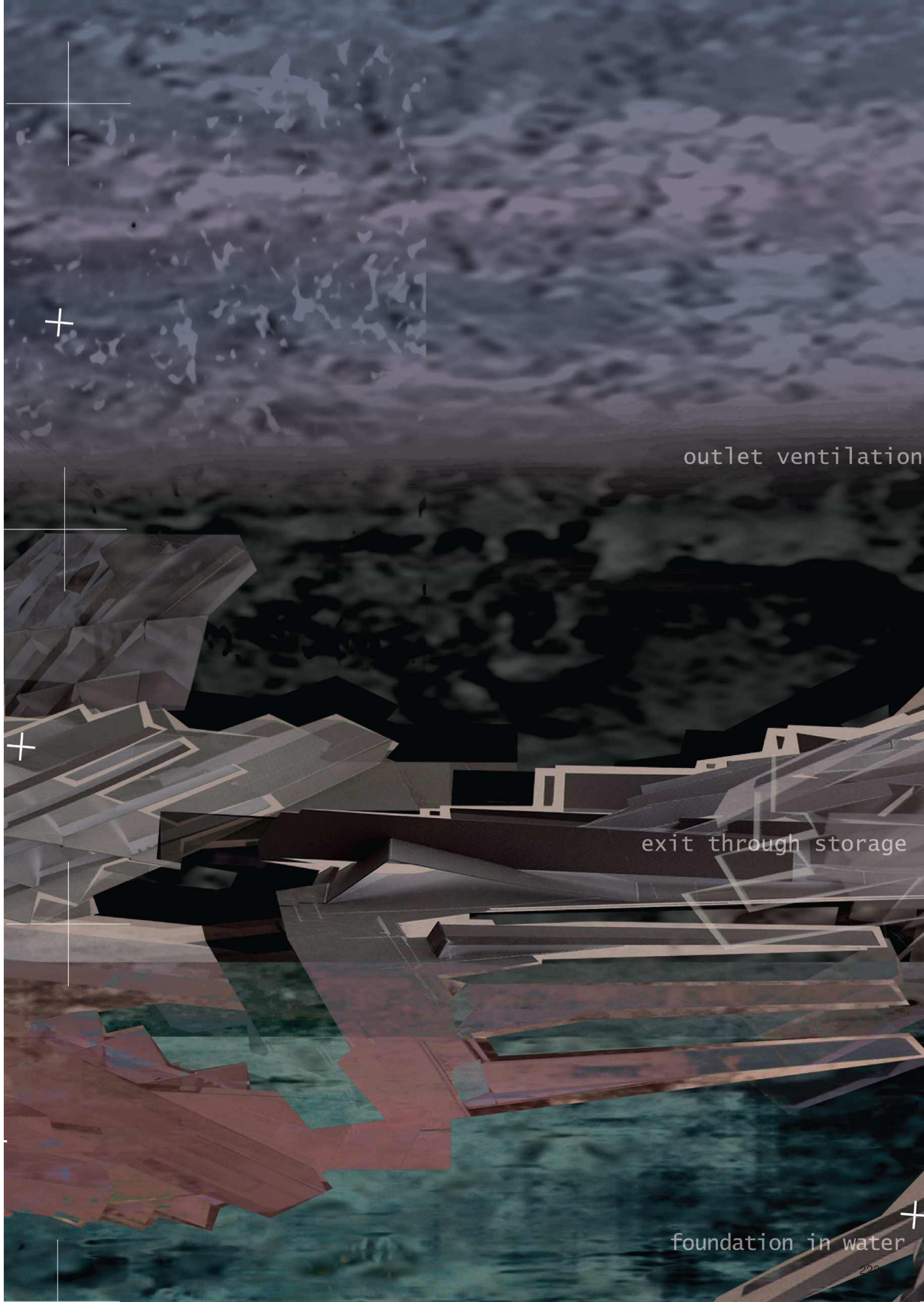
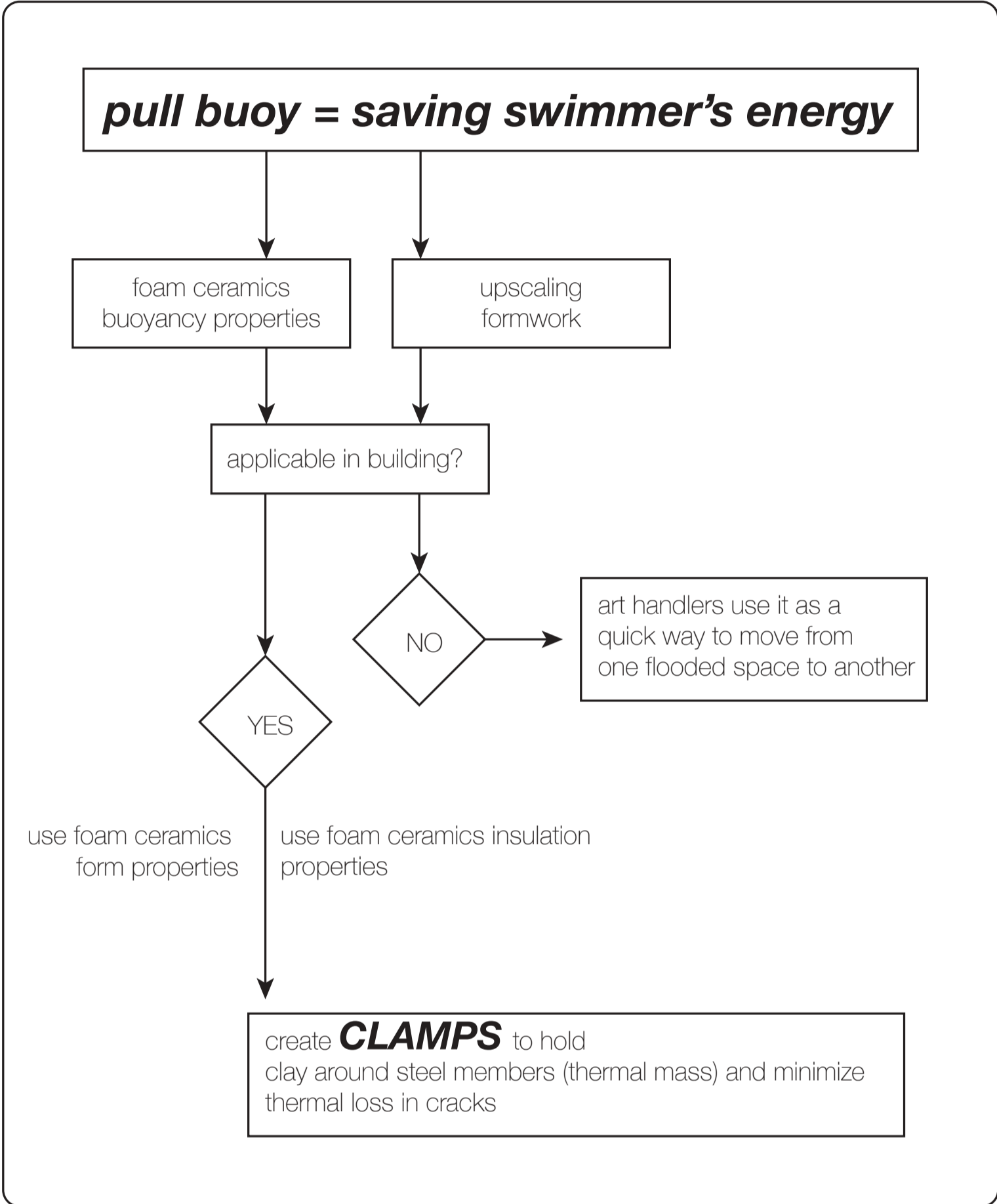




Figure 12.1

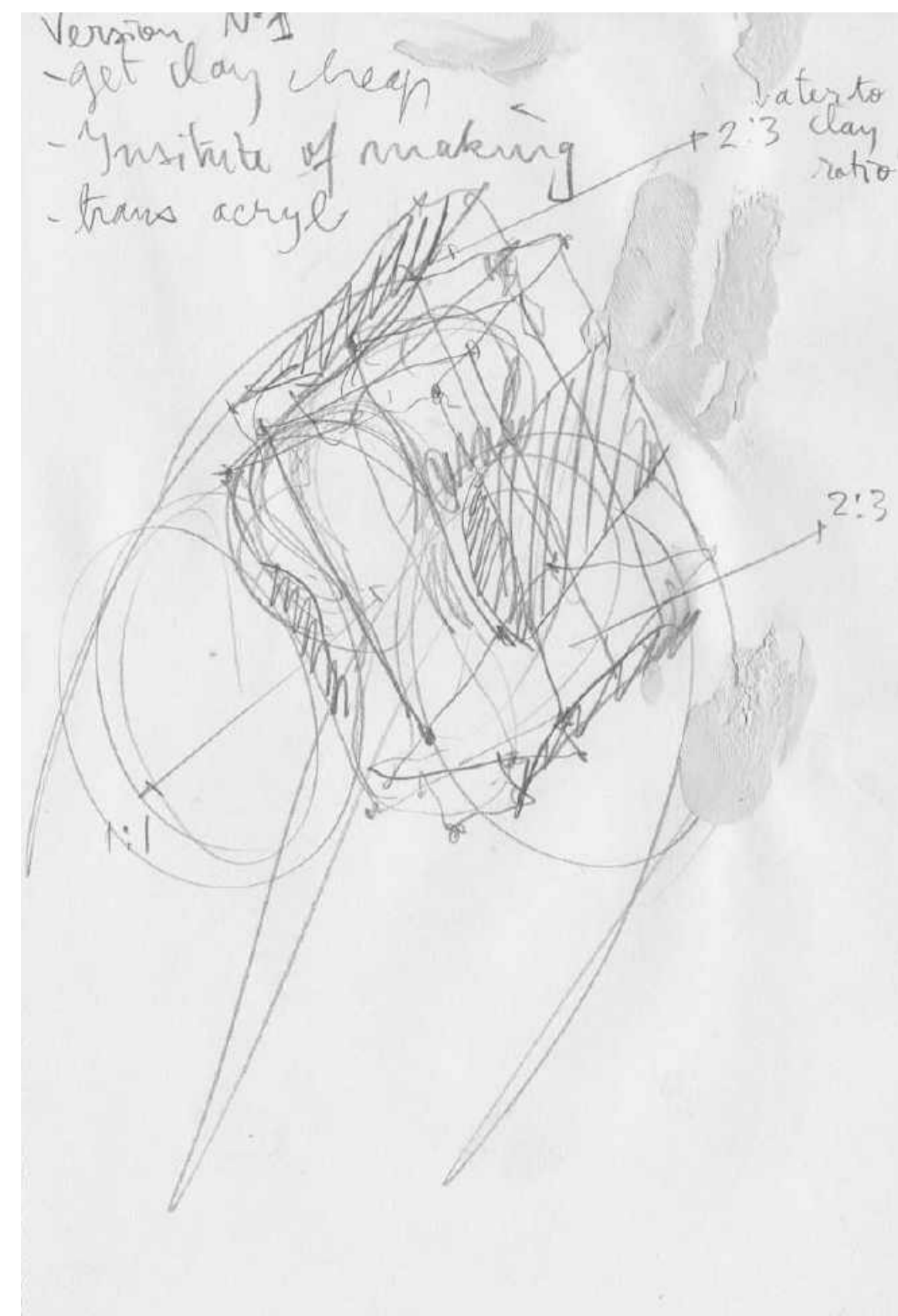


Figure 12.2

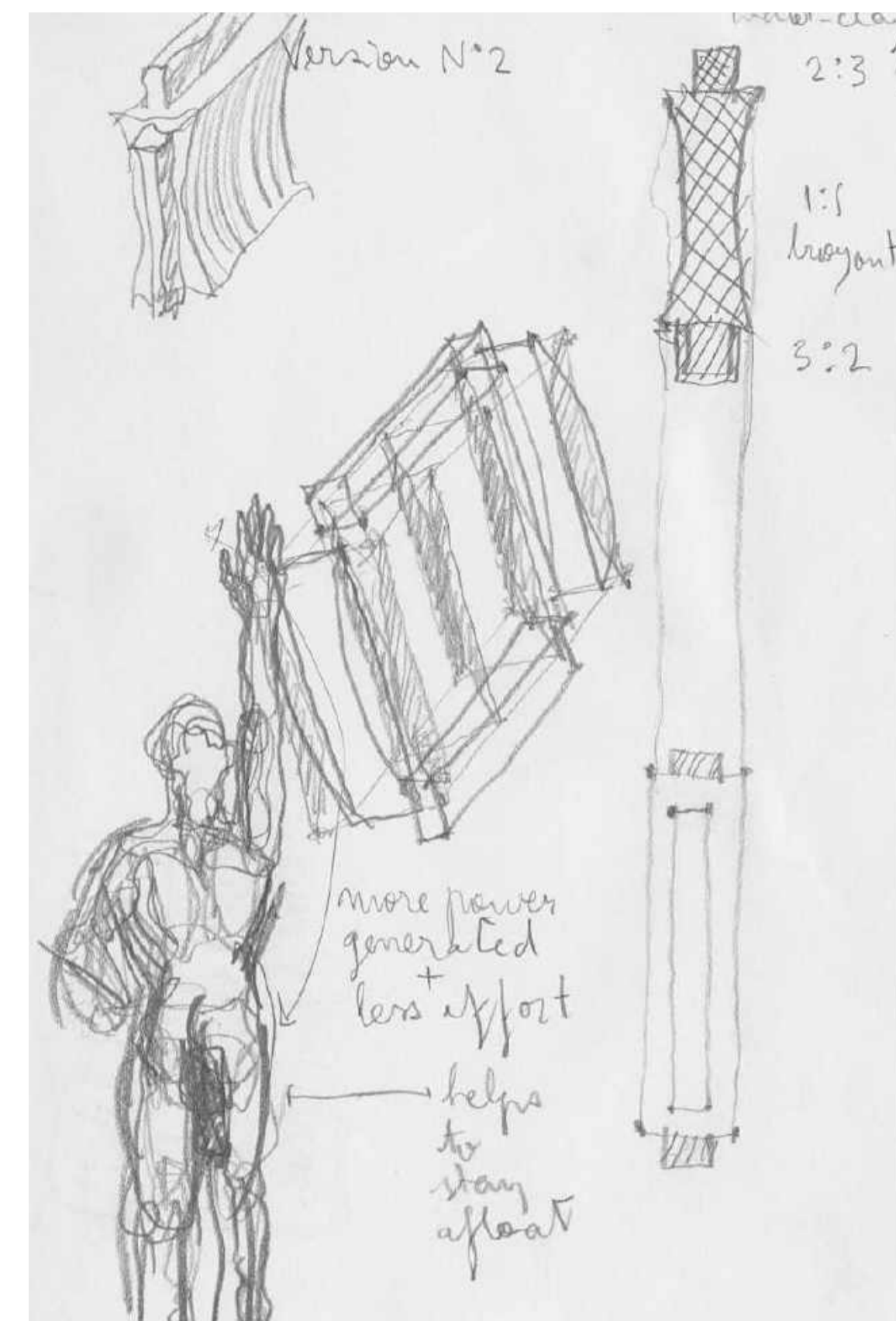


Figure 12.3

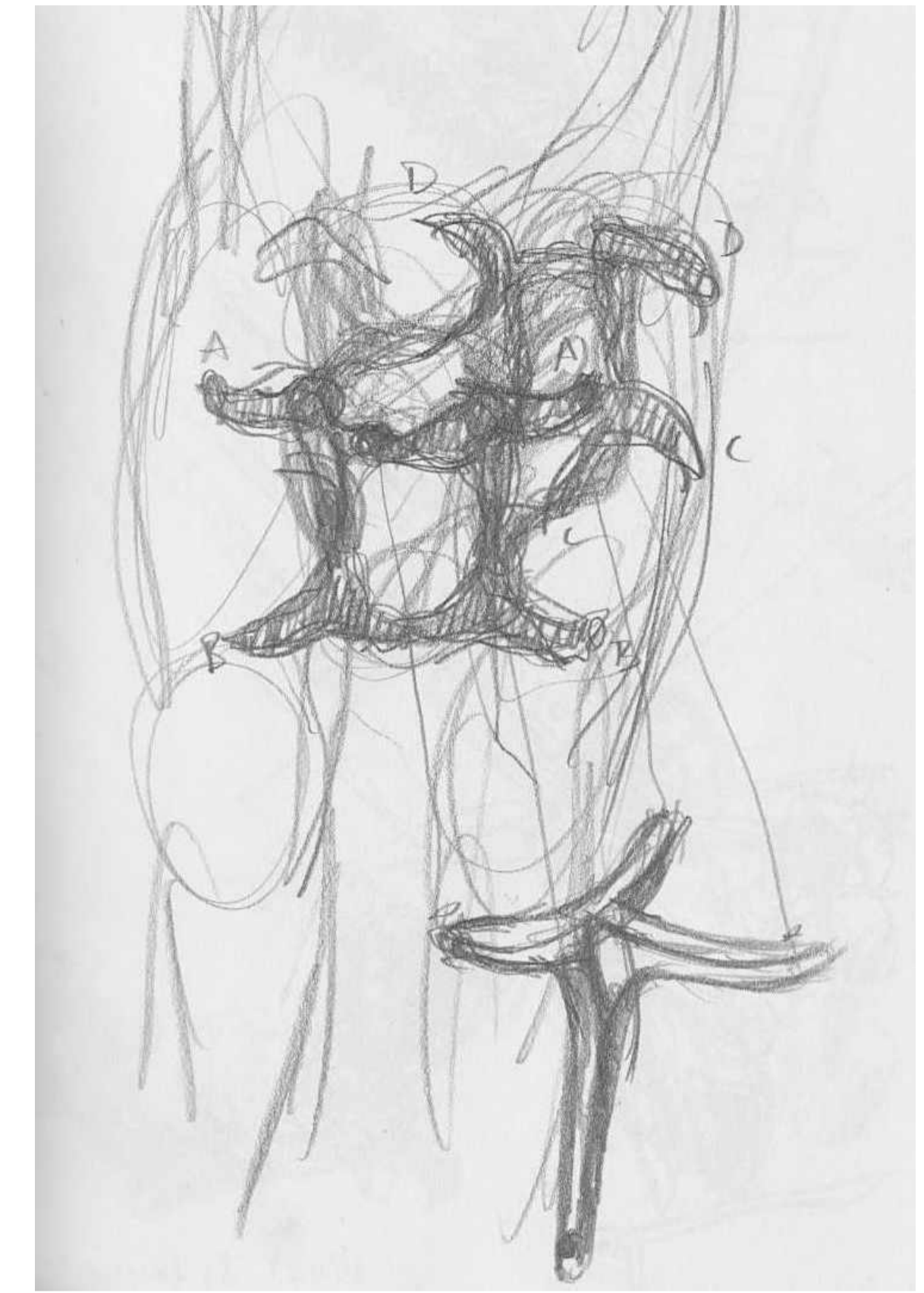


Figure 12.4

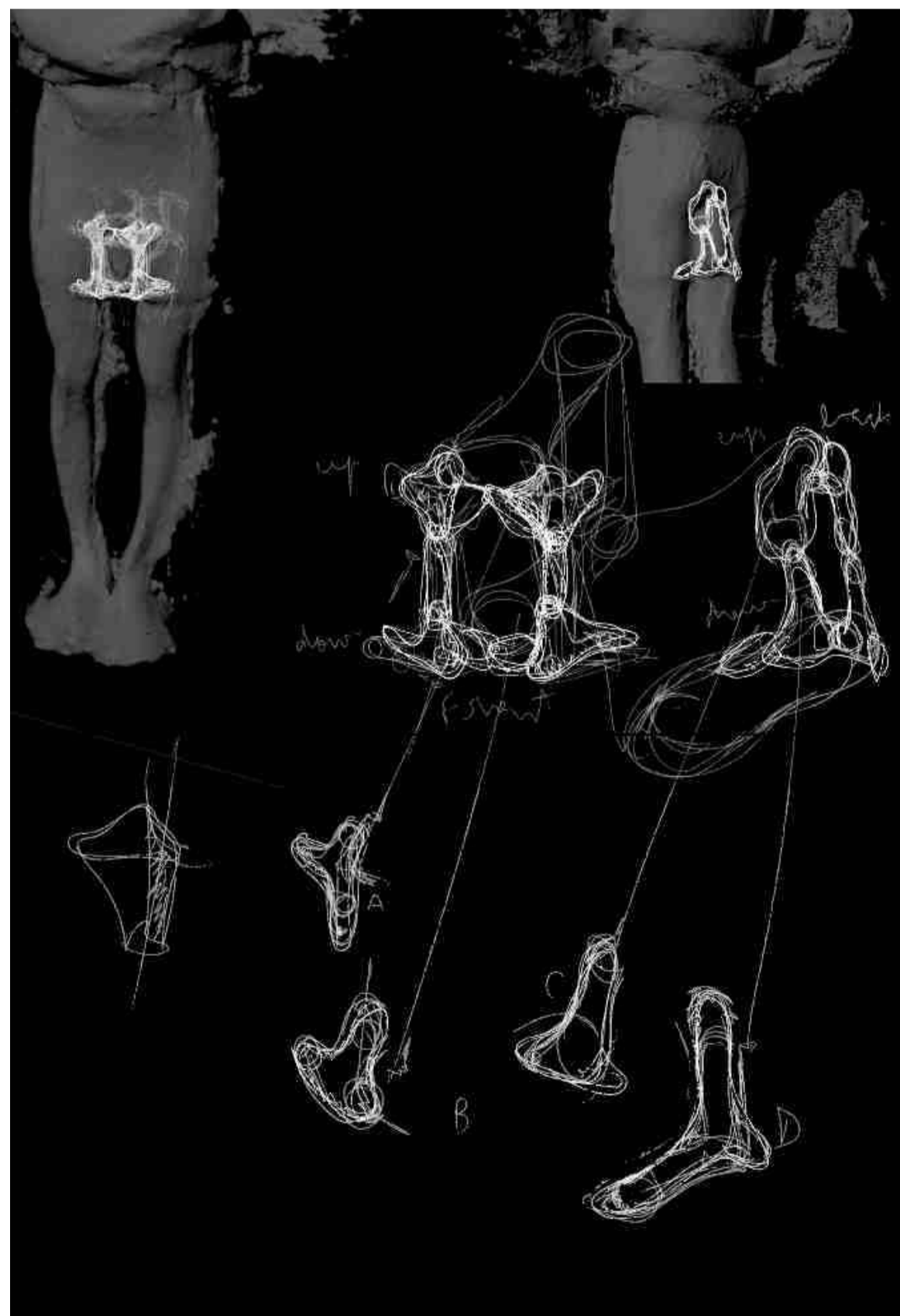


Figure 12.5

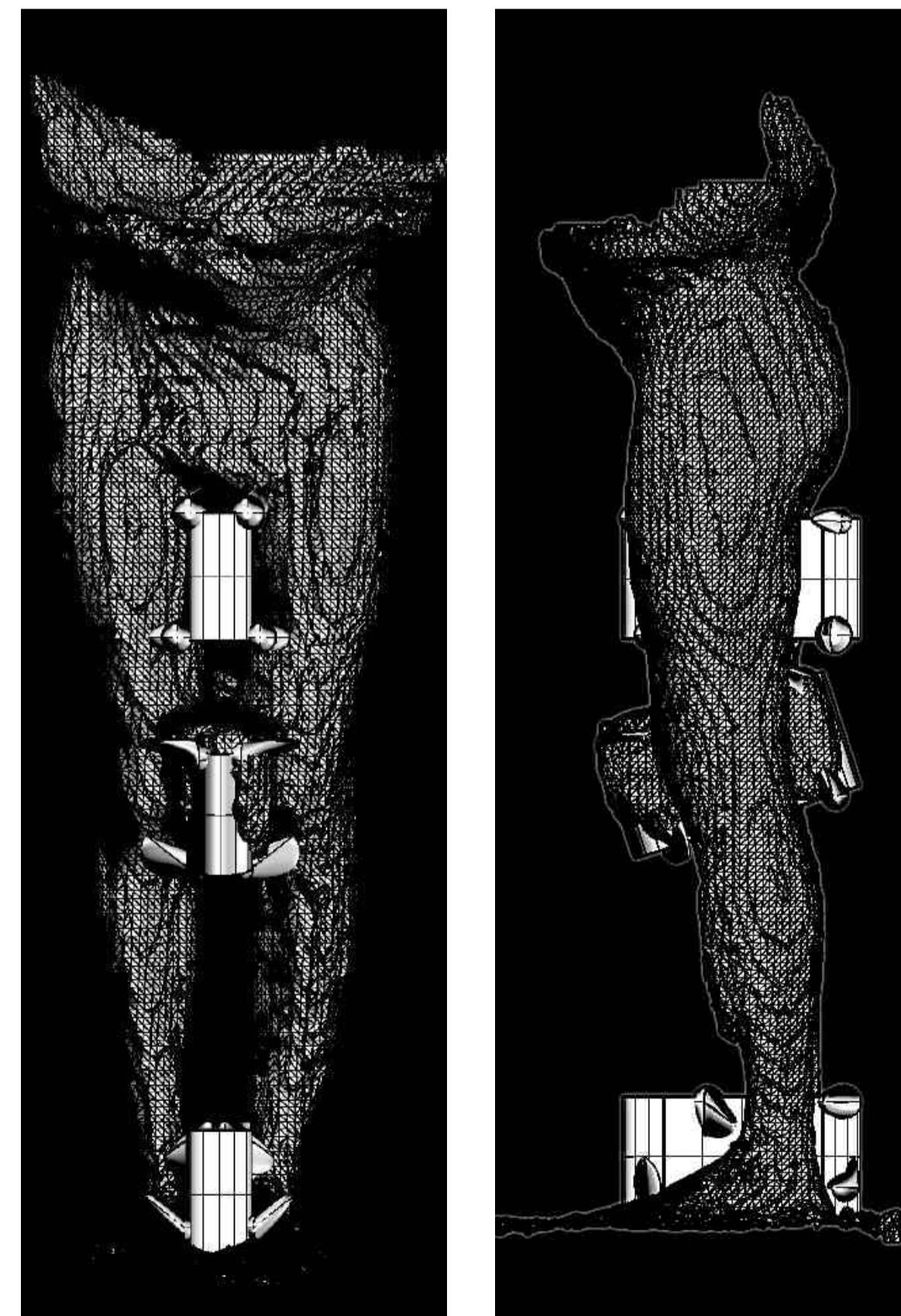


Figure 12.6

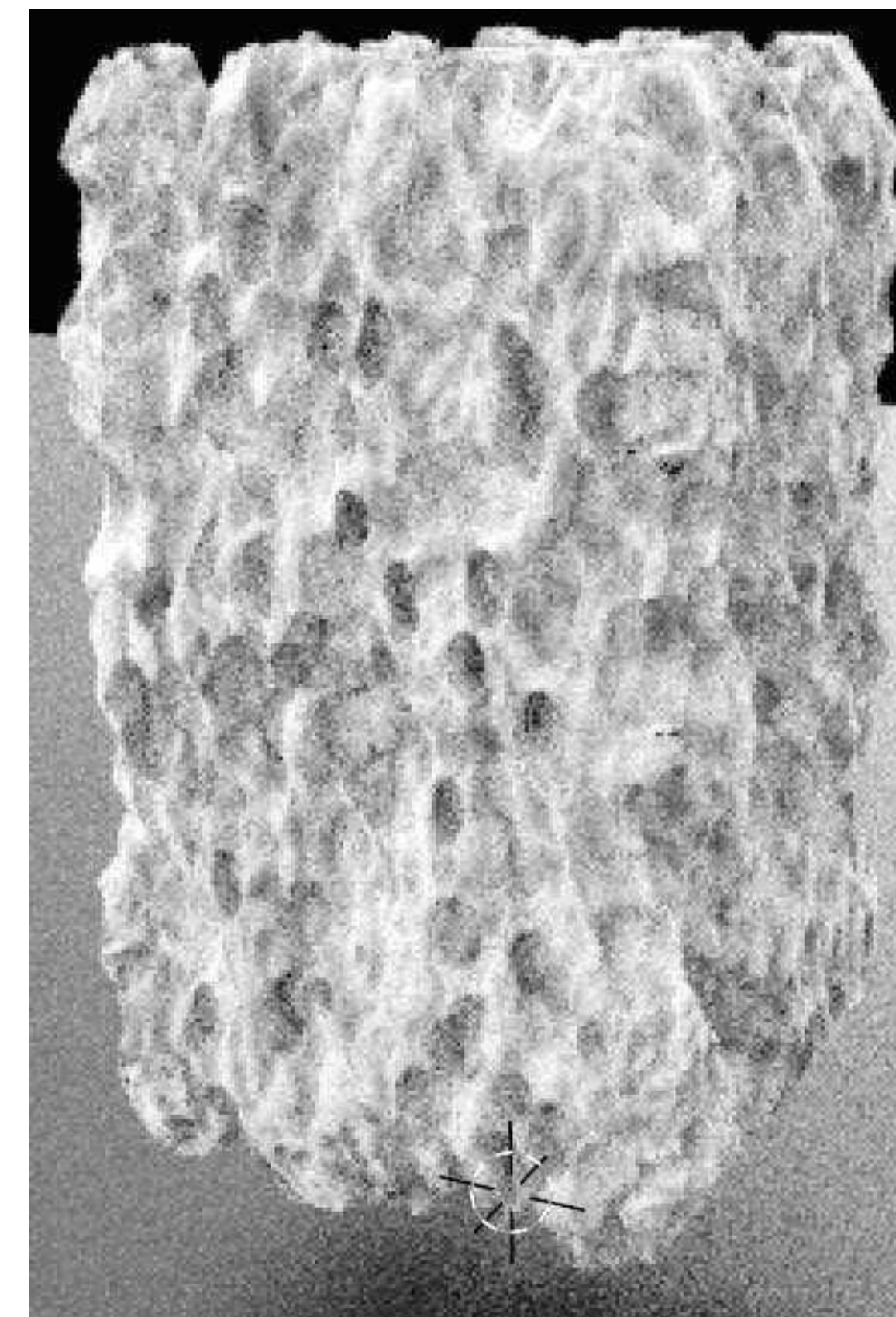


Figure 12.7

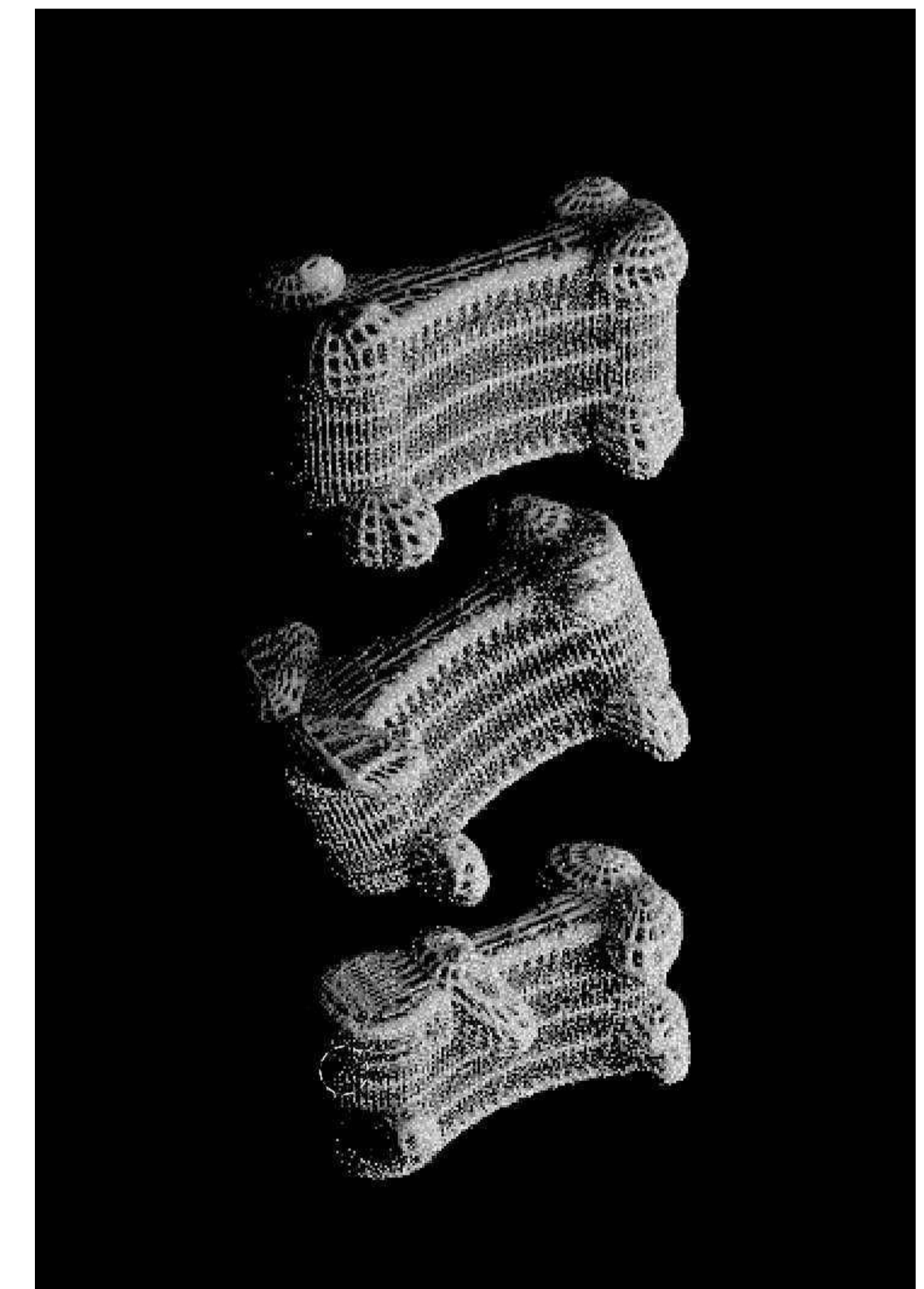
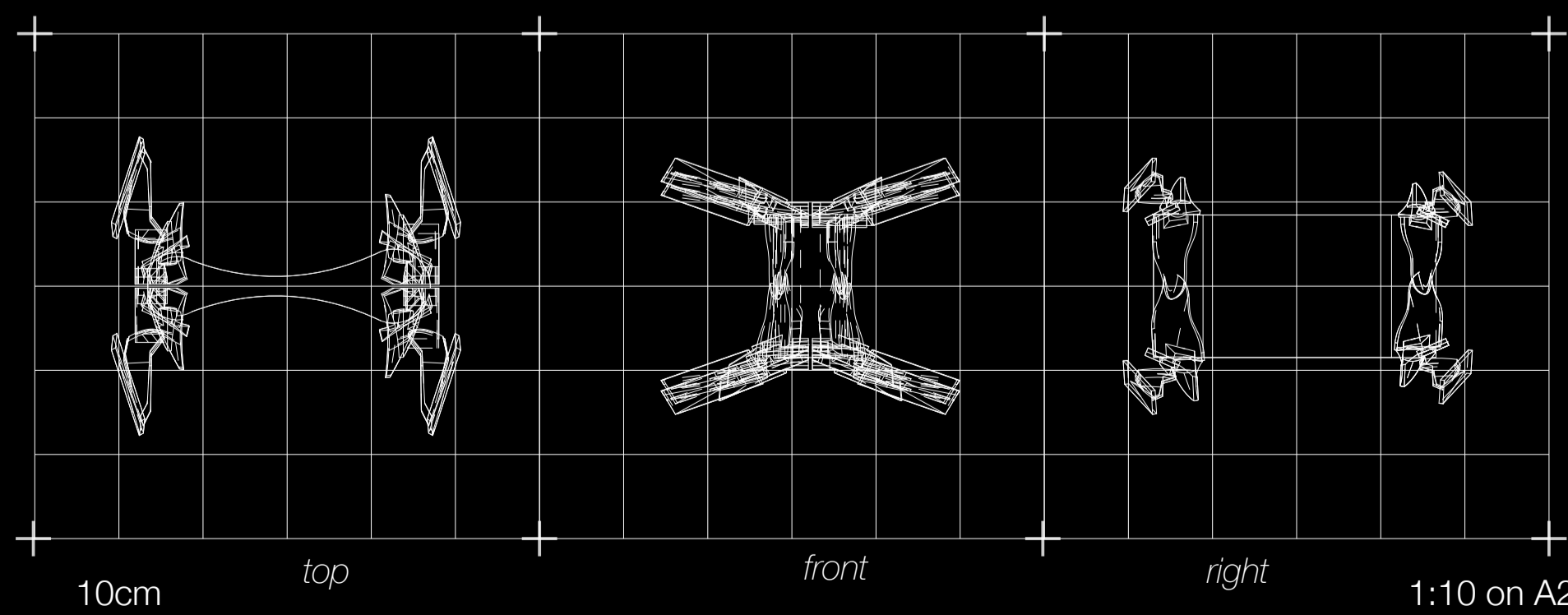
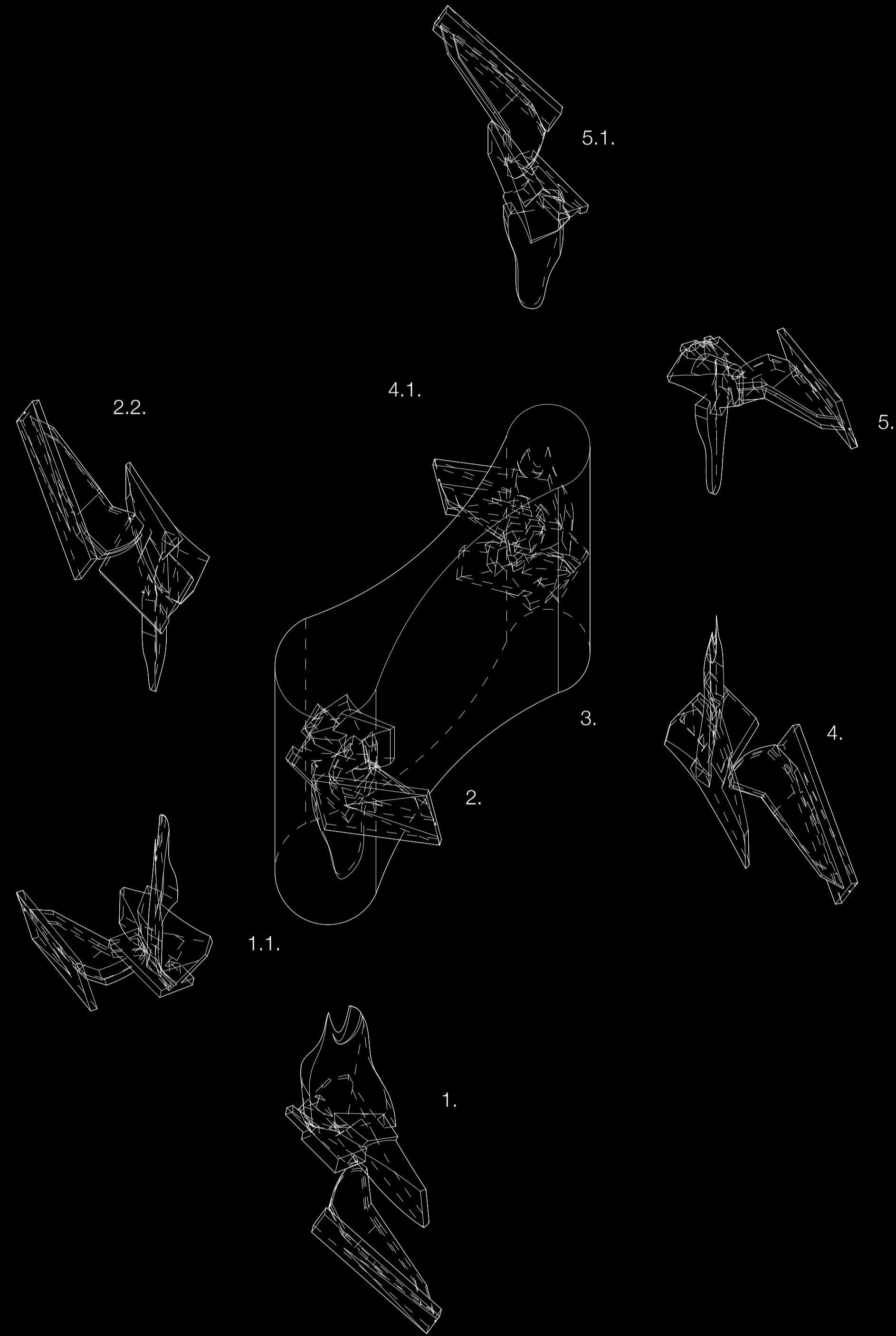


Figure 12.8

After doing preliminary sketches (fig.1 to 4), I was able to model versions of a personalized pull buoy that would fit my legs in different positions using an ipad scan of my legs (fig. 5 and 6).

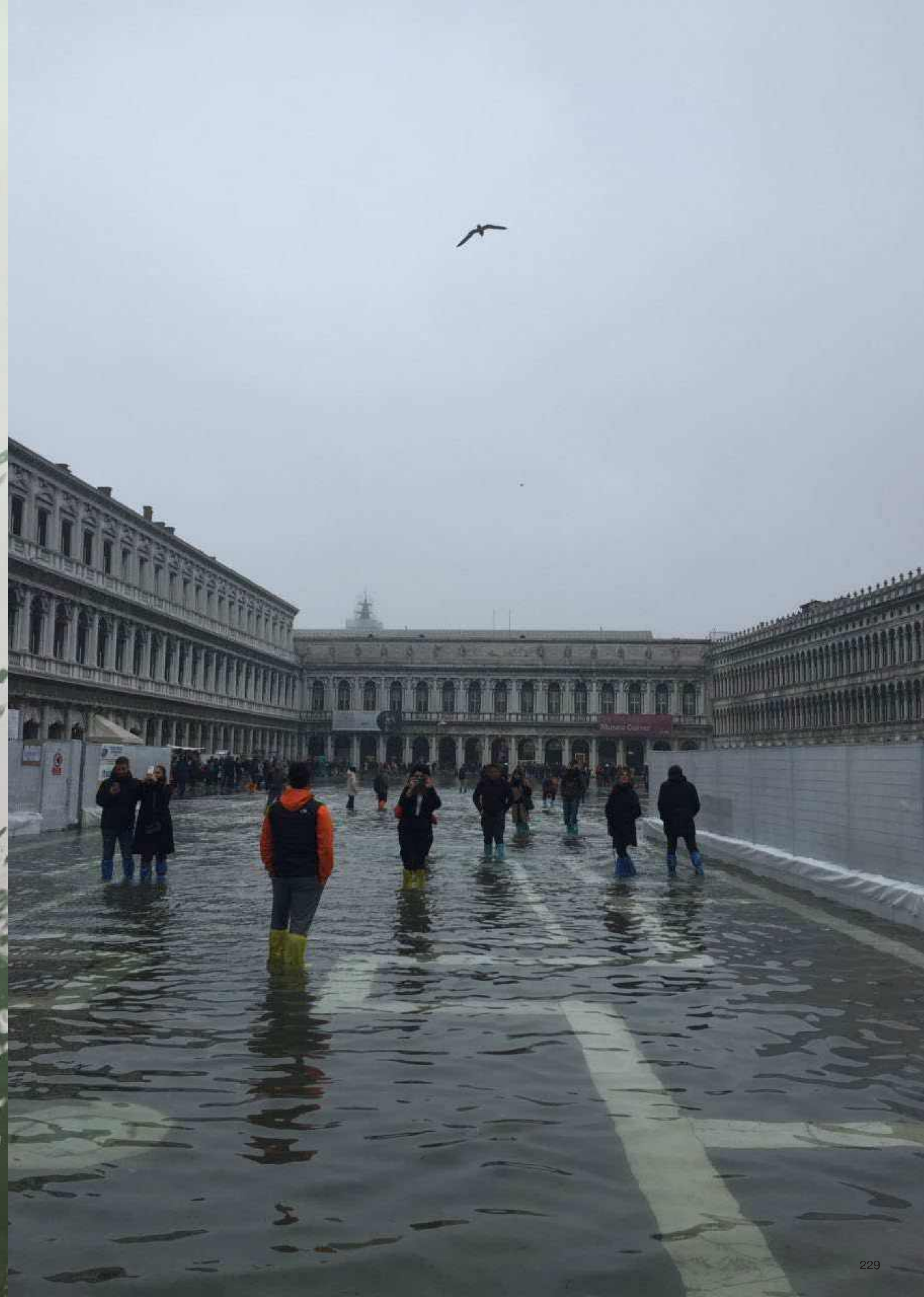
To model the desired foam clay, Blender was used as well as Voronoy textures (fig.7 and 8). It was a way to visualise the design before manufacturing it.

Upscaling



- 1. Top front adjusters
- 2. Bottom front adjusters
- 3. Pull buoy
- 4. Bottom back adjusters
- 5. Top back adjusters

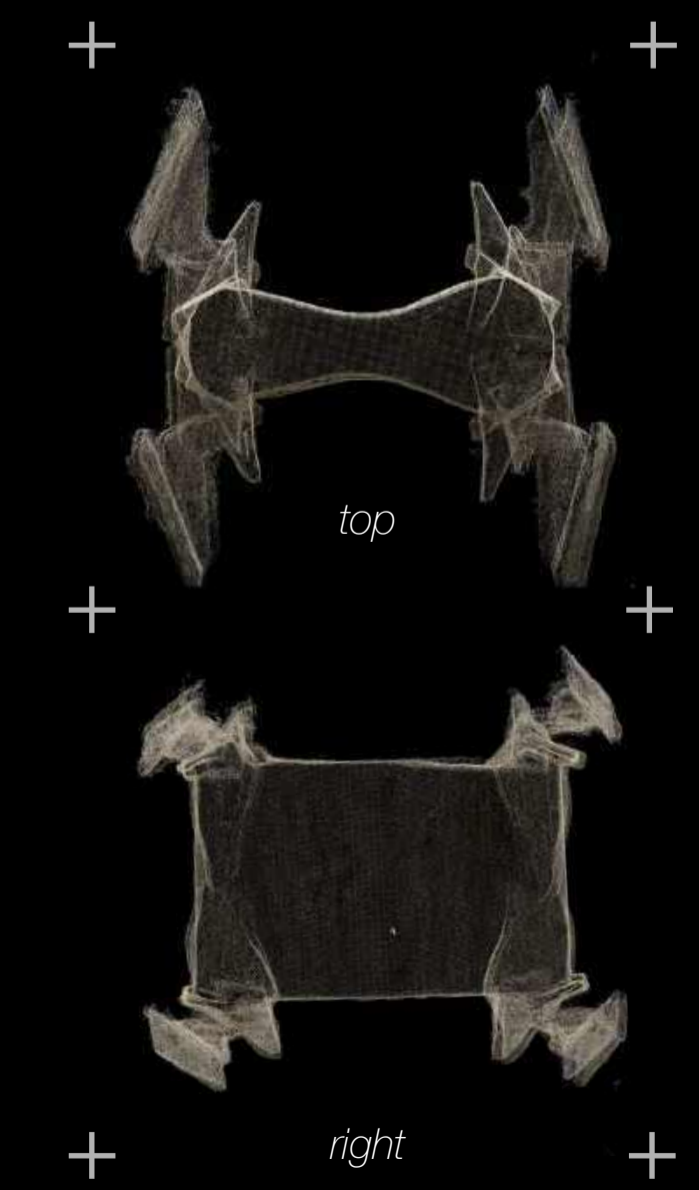




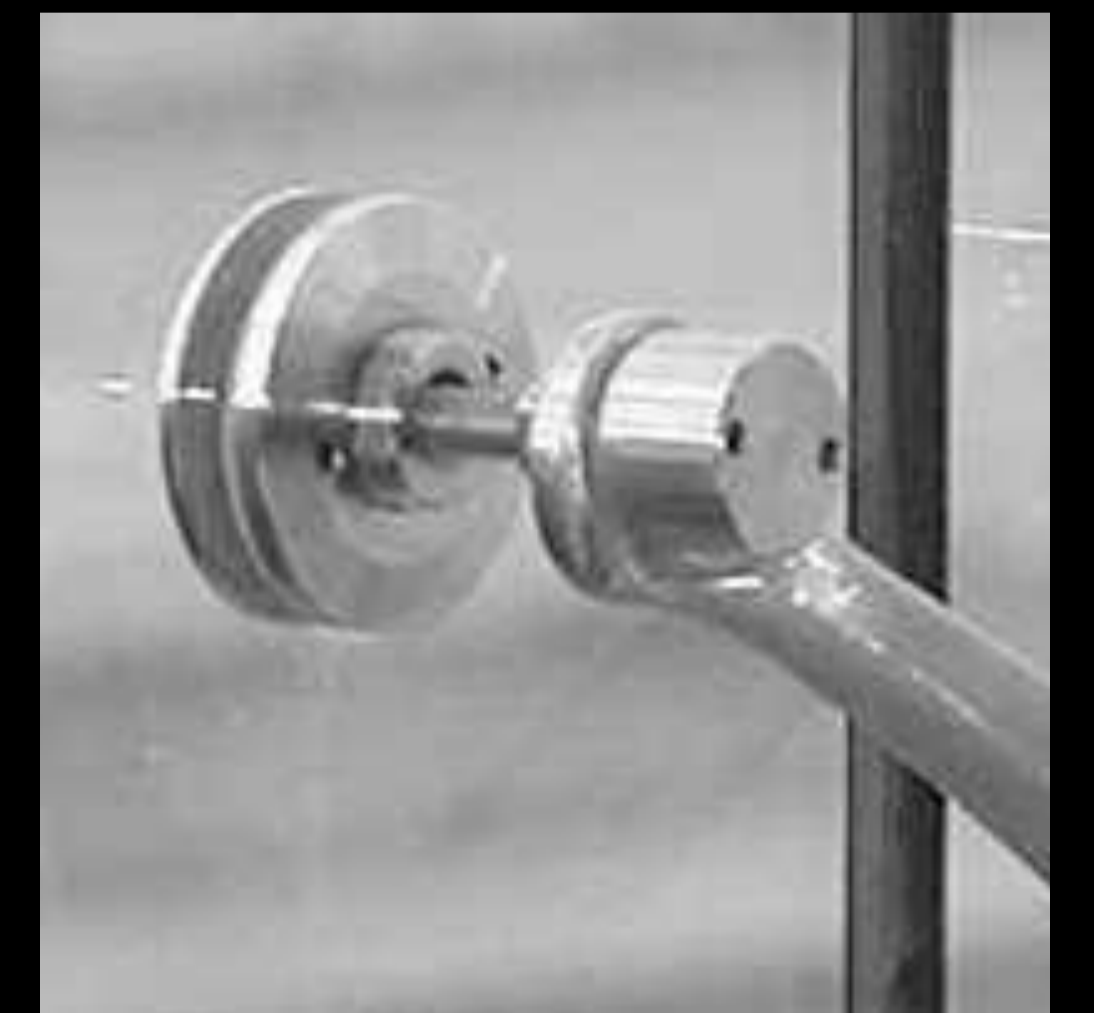




LEGS and SWIMMING DEVICE

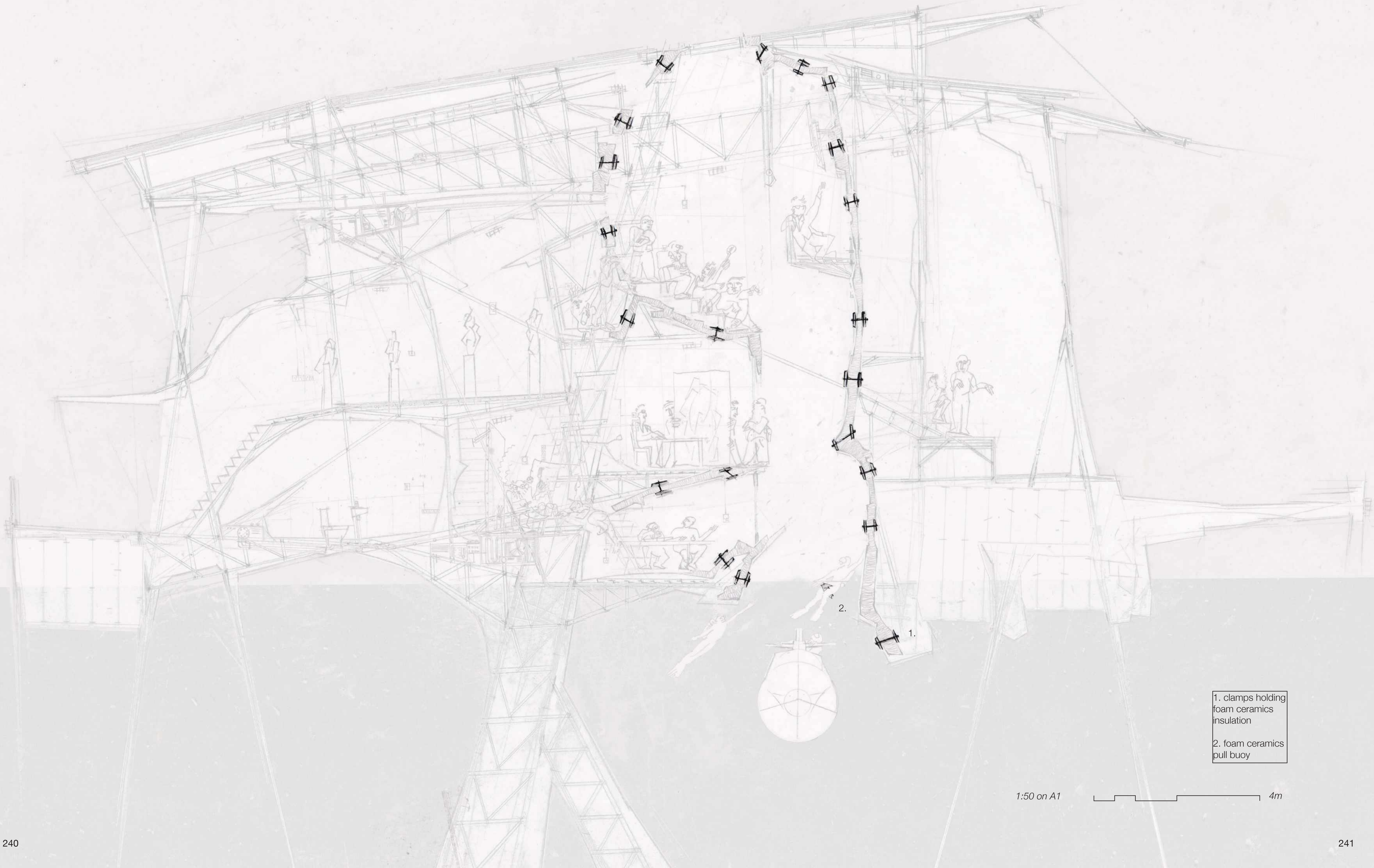
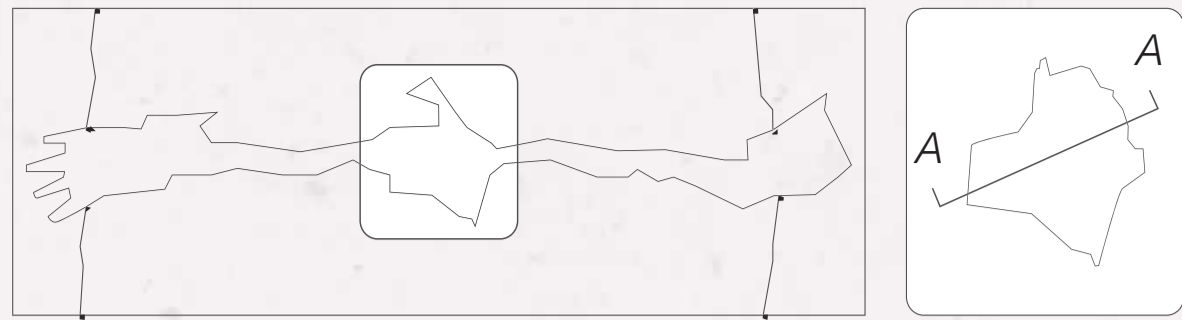


SWIMMING DEVICE



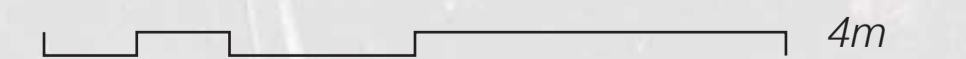


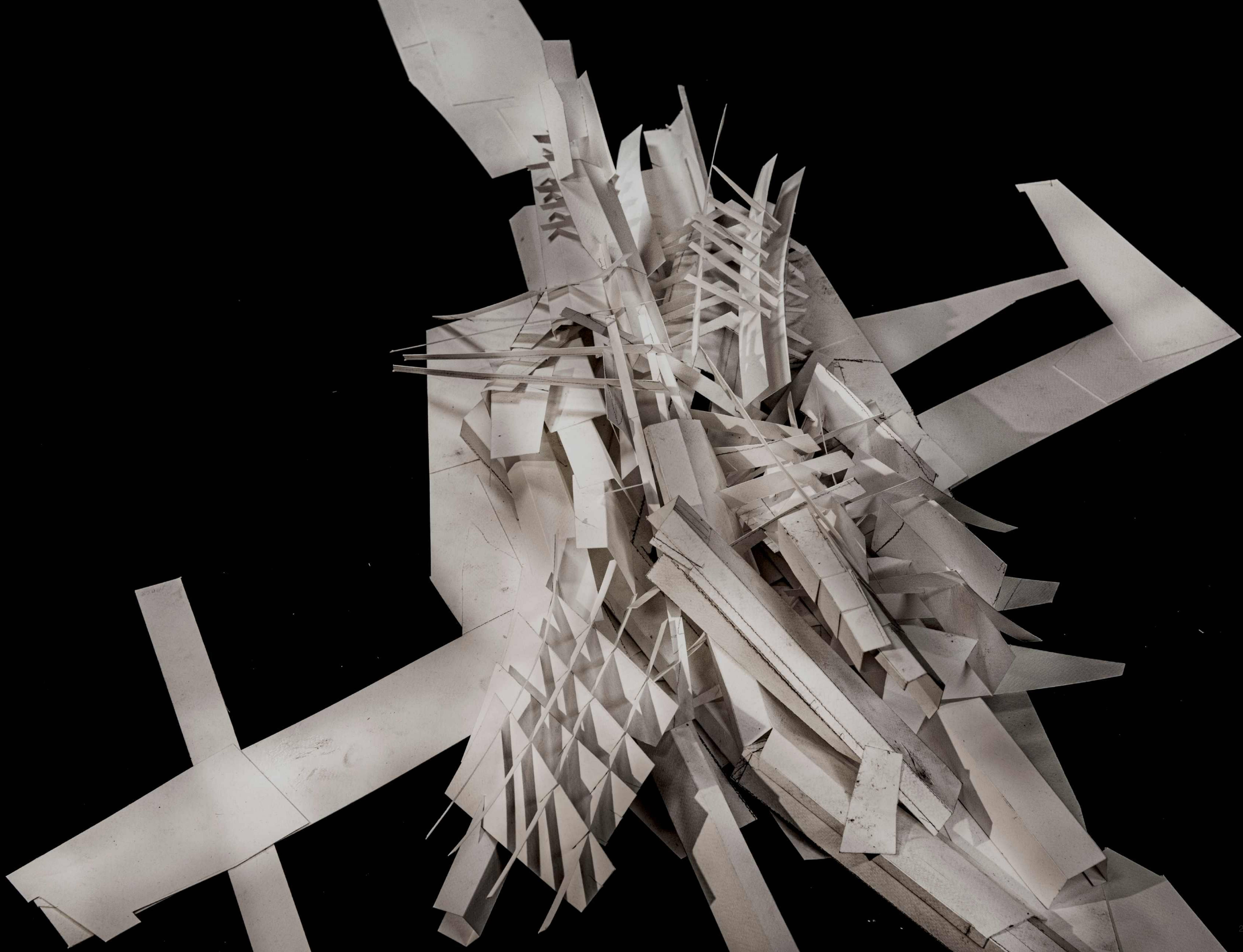


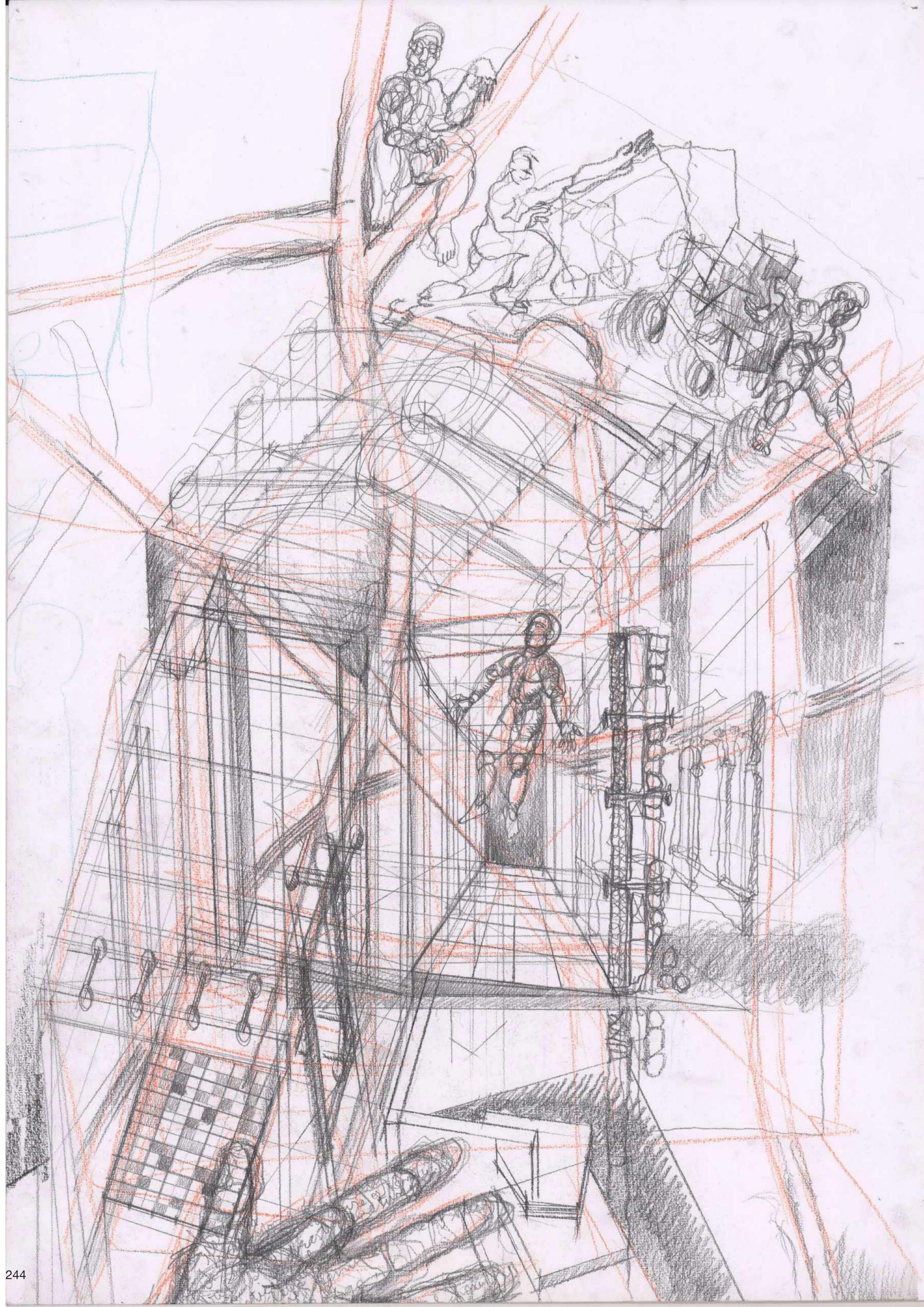


- 1. clamps holding foam ceramics insulation
- 2. foam ceramics pull buoy

1:50 on A1







Section 13. Abstraction: Ramps

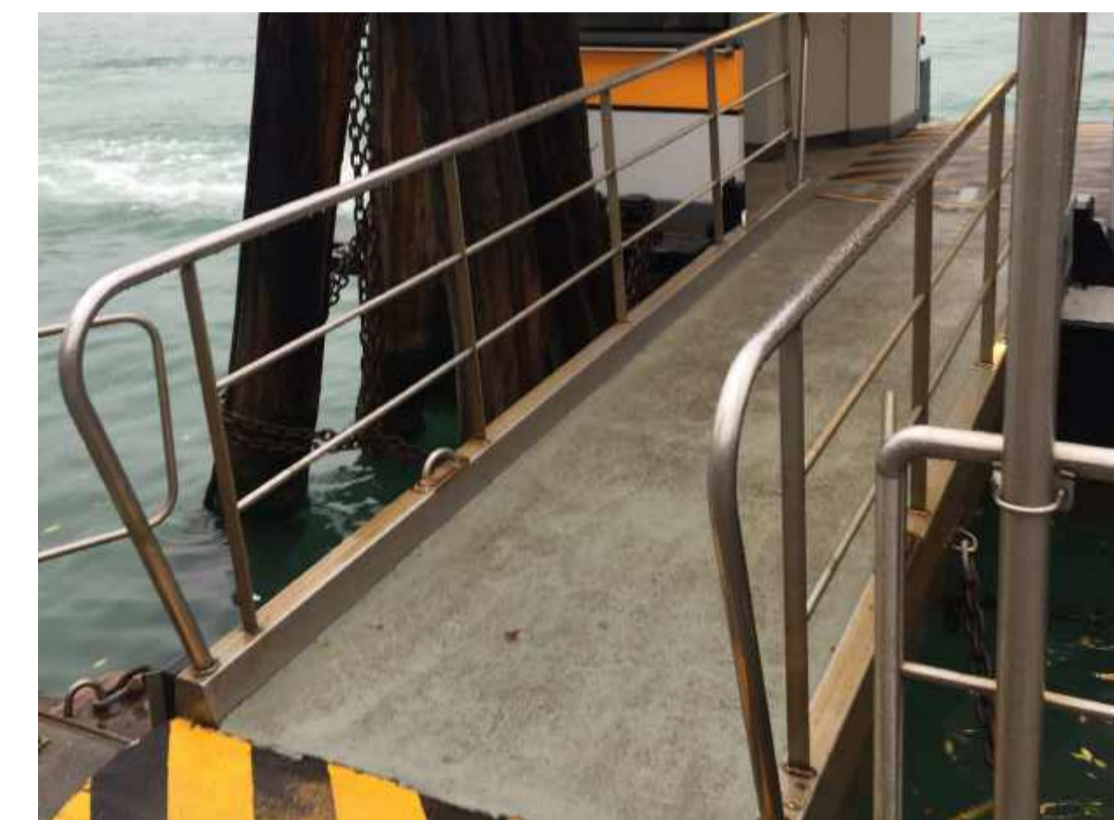
This section mainly focuses on the project's structural aspect. The building remains an ever sinking auction house and thus poses a huge structural challenge. The ever tilting aspect of the bridge makes it a huge ramp and the calculations contain some structural challenges that have to be dealt with accordingly. I was inspired by my trip to Venice, the sinking city, and photographed some of the ramps that were there. I looked extensively at the roof design, floors and foundation, sizing cable and braces, the main columns, and other aspects that had to be considered from a structural point of view. Concerning the cantilever system, I was inspired by the West Ham stadium system and researched how that worked so as to be able to apply some of its design to the auction house.

DEAD LOADS		
construction	material/element	unit load (kN/m2)
roof	snow/wind uplift	0.8
	kingspan steel roof panels	0.8
	services	0.1
	clay tiles finishes	0.1
TOTAL_roof		1.8
DEAD LOADS		
construction	material/element	unit load (kN/m2)
exterior walls	glass windows	0.15
	insulation	0.05
	secondary columns	0.5
	clay tiles finishes	0.1
TOTAL_walls		0.8
DEAD LOADS		
construction	material/element	unit load (kN/m2)
floor	partitions	0.1
	composite slab	1.2
	asphalt waterproofing	0.45
	celing and services	0.15
TOTAL_floor		1.9
LIVE LOADS		
construction	use	unit load (kN/m2)
floor 1	storage/private sale (type A)	2
floor 2	auction (type C)	3
floor 3	gallery (type B)	2
TOTALS		
	unit load (kN/m2)	tributary area (w*l = m2)
total roof	1.8	160
total floor 1	4.7	60
total floor 2	5.7	60
total floor 3	4.7	60
total floors	15.1	180
total all	16.9	340





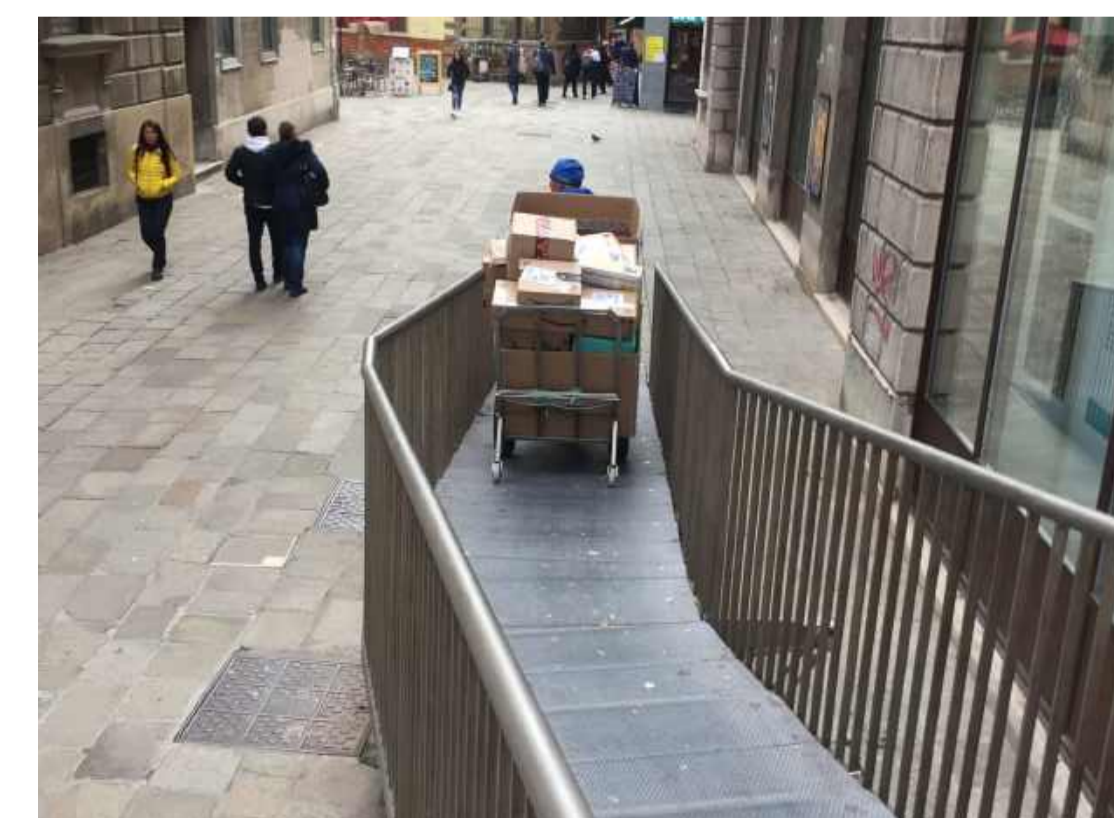
Venice Ramp 1



Venice Ramp 2



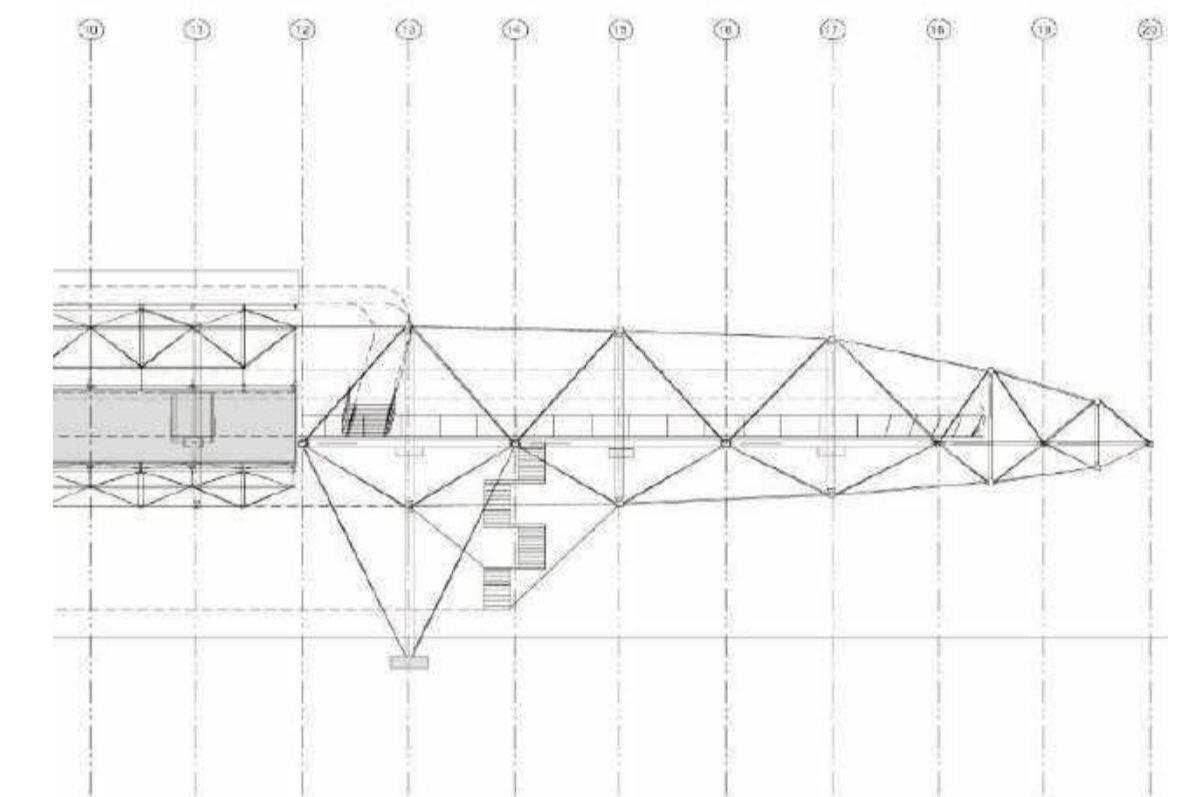
Venice Ramp 3



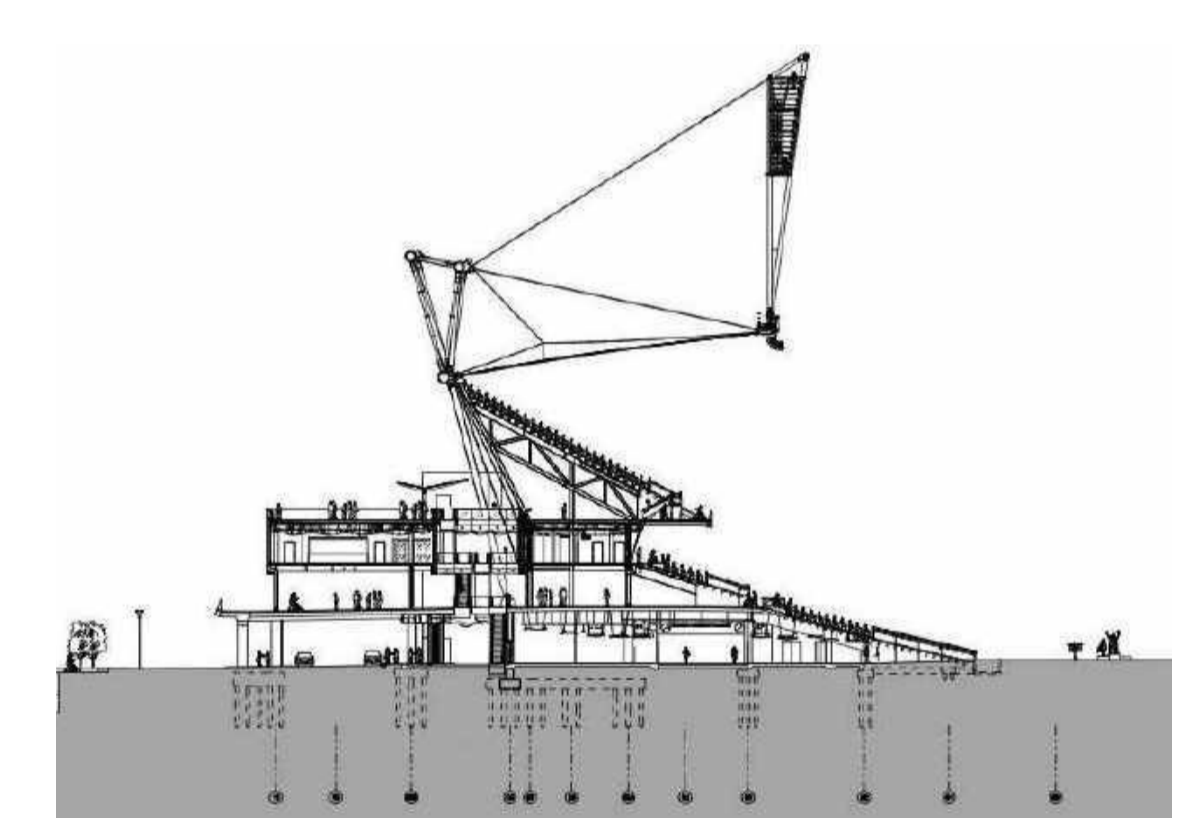
Venice Ramp 4

Structural Considerations

The concept of ramps, sinking cities (Venice), and tilting floors are taken as an inspiration to design cantilever systems, truss systems and members in tension and compression.



Case Study 1: Smoke Pavilion



Case Study 2: West Ham Stadium



Case Study 3: Oil Rigs

Main Columns Design

1.

Wind calculations:
 Let's only consider what is above the water

$V_w = q_p \times C_{pe}$
 where
 $q_p = C_e(z) \times q_b$
 $q_b = 0.613 V_w^2$
 $V_w = c_{dir} \times c_{season} \times c_{prob} \times c_{alt} \times V_{w,ref}$
 where
 $c_{dir} = 1$; $c_{season} = 1$; $c_{prob} = 1$; $c_{alt} = 1$

2.

$V_w = 1 \times 1 \times 1 \times 22 = 22 \text{ m/s}$
 $q_b = 0.613 (22)^2 = 296.7 \text{ N/m}^2$
 $q_p = C_e(z) \times 296.7$
 $C_e(z) = 2.9$
 $q_p = 2.9 \times 296.7 = 860.43 = 0.86 \sim 0.9 \text{ kN/m}^2$
 How much force does it apply on the wall?
 $0.9 \times 12 \times 10 = 108 \text{ kN}$

We have a member in tension and compression

As the roof is tilted, we still have an overturning moment even with the cable bracing so a cable is added

5.

we therefore get a moment of inertia of:
 $I = 6963200 \text{ cm}^4$
 Checking the Blue Book the maximum I cold formed circular hollow section we can get is:
 $I = 74900 \text{ cm}^4 \ll 6963200 \text{ cm}^4$
 As a check we can approximate a square column dimensions.
 $I = \frac{b^4}{12}$; assume $b = h$; $I = \frac{h^4}{12}$
 using found I we get
 $h = \sqrt[4]{12 \times 6.069} = 0.95 \sim 1 \text{ m}$
 so we would need filled $1 \text{ m} \times 1 \text{ m} \times 32 \text{ m}$ steel column to hold our structure which is not realistic at all.

We will then use a system of trusses to create the columns.

The circular shape follows the loading moment diagram as the maximum BM is along the length of the column at the base. For the simplification of calculations, parallel geometry is used.

For now we are ignoring the assumed pin in the bracing and just spreading two columns parallel to each other.

6.

We will use system C as it is the system that is able to adapt the most to forces acting in V, H and diagonal directions.

The sizing will be separated into 4 sections:

- Section 1 = holds just the roof
- 2 = roof + floor 3
- 3 = roof + floor 3 + floor 2
- 4 = holds all

The different compression loads and bending moments will determine the sizing of the hollow circular columns and bracing.

3.

CHECK DIAGRAMS

$UDL_1 = 28.8 \text{ kN/m} \div 4$
 $UDL_2 = 24.2 \text{ kN/m} \div 3$
 $UDL_3 = 24.2 \text{ kN/m} \div 3$
 $UDL_4 = 28.2 \text{ kN/m} \div 3$

Primary beams sectioned by roof beams connected roof panels

Floor Plan

Roof Plan

4.

COLUMNS:

$1194 \text{ kN} / 6 = 199 \text{ kN} \sim 200 \text{ kN}$

Let's model this column as a cantilever beam

$P = 200 \text{ kN}$
 $P_H = 200 \sin 20 = 68 \text{ kN}$
 $P_V = 200 \cos 20 = 188 \text{ kN}$
 $\delta_{max} = \frac{WL^3}{EI}$; $\delta_{limit} = \frac{L}{180}$

$\sum F_H = 0 \Rightarrow H_A = 188 \text{ kN}$
 $\sum F_V = 0 \Rightarrow V_A = 68 \text{ kN}$
 $\sum M = 0 \Rightarrow M_A = 32 \times 68 = 2176 \text{ kNm}$

Sizing it with F_r .

$\frac{W L^3}{EI} \leq \frac{L}{180} \Rightarrow I \geq \frac{60 W L^2}{E}$
 $\therefore I \geq \frac{60 \times 68 \times 18 \times 32^2}{200 \times 10^9}$
 $\therefore I \geq 0.669 \text{ m}^4 = 6693200 \text{ cm}^4$

7.

We will size the part with the least load and the one with the most, and average what is inbetween.

Smallest Load:

Assumptions:
 - roller pin
 - extra force on bottom left at A

As the structure is in equilibrium:

$\sum M_A = 0 \Rightarrow (1 \times 4) + (1 \times 9) - (1 \times V_D) = 0 \Rightarrow V_D = 13 \text{ kN}$
 $\sum M_D = 0 \Rightarrow (1 \times 4) + (1 \times V_A) - (1 \times 9) = 0 \Rightarrow V_A = 5 \text{ kN}$
 $\sum V = 0 \Rightarrow 5 + 13 = 9 + 9 \Rightarrow 18 \text{ kN} = 18 \text{ kN}$
 $\sum H = 0 \Rightarrow 4 \text{ kN} = H_D + H_A \Rightarrow H_D = 8 \text{ kN}$

8.

Resolve at A:

$\sum H = 0 \Rightarrow F_{AE} \cos 45 + 4 = 0 \Rightarrow F_{AE} = -5.7 \text{ kN} \sim -6 \text{ kN}$
 $\sum V = 0 \Rightarrow F_{AB} \sin 45 + V_A = 0 \Rightarrow F_{AB} = -5.7 \text{ kN} \sim -6 \text{ kN}$

Resolve at B:

$\sum V = 0 \Rightarrow -F_{AB} - F_{BE} \sin 45 - 9 = 0 \Rightarrow F_{BE} = -1.41 \text{ kN} \sim -2 \text{ kN}$
 $\sum H = 0 \Rightarrow 4 + F_{BC} + F_{BE} \cos 45 = 0 \Rightarrow F_{BC} = -3.5 \text{ kN} \sim -4 \text{ kN}$

Resolve at C:

$\sum H = 0 \Rightarrow -F_{BC} - F_{CE} \cos 45 = 0 \Rightarrow F_{CE} = 2.5 \text{ kN} \sim 3 \text{ kN}$
 $\sum V = 0 \Rightarrow -9 - F_{CD} - F_{CE} \sin 45 = 0 \Rightarrow F_{CD} = -13.5 \text{ kN} \sim -14 \text{ kN}$

Resolve at D:

$\sum H = 0 \Rightarrow -F_{DE} \cos 45 - 8 = 0 \Rightarrow F_{DE} = -11.31 \text{ kN} \sim -12 \text{ kN}$

9.

all in kN

Sizing element AB and DC from the Blue Book:
 For axial compression of 8 kN (AB); with length 1m:
 $d = 42.4, t = 3 \text{ mm}$
 For axial compression of 13 kN (DC); with length 1m:
 $d = 18.3, t = 3 \text{ mm}$

Sizing central elements, taking the largest force (12 kN):
 $d = 48.3, t = 3 \text{ mm}$

It seems that element BE and EC don't hold much. In addition, the members AB and DC go all the way to the base, so have a much larger moment and need to be continuous. Therefore we will redo a calculation for the large corner columns.

10.

Learning from the previous iteration, the trusses with the largest stress will be sized.

A new truss system is implemented to make the trusses more efficient.

$\sum M_A = 0 \Rightarrow (1 \times 68) + (1 \times 188) - (1 \times V_E) = 0 \Rightarrow V_E = 256 \text{ kN}$
 $\sum M_E = 0 \Rightarrow (1 \times V_A) + (1 \times 68) - (1 \times 188) = 0 \Rightarrow V_A = 120 \text{ kN}$
 $H_E = 2 \times 68 = 136 \text{ kN}$

Resolving at B:
 $\sum H = 0 \Rightarrow F_{BC} + 68 = 0 \Rightarrow F_{BC} = -68 \text{ kN}$
 $\sum V = 0 \Rightarrow -188 - F_{BA} = 0 \Rightarrow F_{BA} = -188 \text{ kN}$

Resolving at A:
 $\sum V = 0 \Rightarrow F_{AD} + F_{AC} \sin 60 + 120 = 0$
 $+188 + F_{AC} \sin 60 + 120 = 0 \Rightarrow F_{AC} = -68 \text{ kN}$
 $\sum H = 0 \Rightarrow 68 + F_{AE} + F_{AC} \cos 60 = 0 \Rightarrow F_{AE} = -120 \text{ kN}$

Solving Cantilever System

13.

So the final member sizing is:

Location	A	B
Floor 3	A = 76.1 B = 18.3	4 OR 7
Floor 2	A = 114.3 B = 28.7	5 OR 7
Floor 1	A = 168.3 B = 139.7	5 OR 5
Base	A = 219.1 B = 131.7	5

water 32m

SKETCH

column or trusses in any case they will be thin

14.

We can now size the cable and front cantilever.

cantilever stadium like system

UDL = 18 kN/m

Max Shear = 22 kN

TRUSSES

TRUSS COMPRESSION

CABLE TENSION

WEST HAM STADIUM core study

11.

Resolving at E:
 $\sum H = 0 \Rightarrow -F_{AE} - F_{EC} \cos 60 - 136 = 0$
 $93 - F_{EC} \cos 60 - 136 = 0 \Rightarrow F_{EC} = -86 \text{ kN}$
 $\sum V = 0 \Rightarrow 256 + F_{BE} + F_{EC} \sin 60 = 0$
 $256 - 86 \sin 60 + F_{BE} = 0 \Rightarrow F_{BE} = -182 \text{ kN}$

Resolving at C:
 $\sum H = 0 \Rightarrow F_{CD} - F_{BC} \cos 20 + F_{EC} \cos 60 = 0$
 $F_{CD} = F_{BC} \cos 20 - F_{EC} \cos 60$
 $F_{CD} = 68 \cos 20 + 86 \cos 60 = 44 \text{ kN}$

all in kN

12.

* applying SF = 15%

Blue Book comparing axial compression relative to buckling length.

Member	Force (kN)	Length (m)	d (mm)	t (mm)
FAD	188	2.67	76.1	4
FBC	68	0.991	48.3	3
FCD	44	0.762	42.4	3
FDE	182	2.67	76.1	4
FEA	93	1.27	48.3	4
FAC	73	0.991	48.3	3
FCE	86	1.27	48.3	4

So we have:
 $[76.1; 4] \times 2$ taking into account slenderness length $[508; 12] \times 2$
 $[48.3; 4] \times 2$ $[139.7; 3] \times 5$
 $[48.3; 3] \times 2$ - one and simplifying
 $[42.2; 3] \times 1$

Checking for buckling:
 $F = \frac{\pi^2 EI}{L^2}$
 $I = \frac{FL^2}{\pi^2 E}$; $m = 0.25$
 $I = \frac{(188 \times 10^3)(2.67^2)}{(0.25)(\pi^2)(190 \times 10^9)}$
 $I = 4.335 \times 10^{-4} \text{ m}^4$
 $I = 43.345.7 \text{ cm}^4$
 Hollow circular section is $d = 508; t = 12 \text{ mm}$

15.

Quick sizing of cantilever beam:
 From Blue Book:
 $\frac{WL^3}{8EI} \leq \frac{L}{180}$
 $I \geq \frac{WL^3 \times 180}{8E}$ where $E = 200 \times 10^9$
 $I \geq \frac{22.5(18)^3 \times 10^3}{180 \times 10^9}$
 $I \geq 1.44 \times 10^{-4} \text{ m}^4 = 14.400 \text{ cm}^4$
 taking $I = 19.500 \text{ cm}^4$; SF = 15%
 we can use UB 356 x 171 x 67
 where $h = 365.4 \text{ mm}$ and $b = 173.2 \text{ mm}$
 But we need to make structure lighter and use slightly less material but make it more substantial.

Option 1 = full thick beam and large span
 total kg = 268.4 kg of steel

Option 2 = truss cantilever, smaller span
 total kg for 2 beams < 40 kg

Option 3 = not used

16.

Resolving at A:
 $\sum M_A = 0 \Rightarrow (2 \times 18) + (1 \times 18) - (1 \times H_B) = 0 \Rightarrow H_B = 108 \text{ kN}$
 $\sum M_B = 0 \Rightarrow (2 \times 18) + (4 \times 18) - (1 \times H_A) = 0 \Rightarrow H_A = 108 \text{ kN}$
 $\sum V = 0 \Rightarrow V_A = 54 \text{ kN}$

Resolving at A:
 $\sum H = 0 \Rightarrow F_{AB} = -108 \text{ kN}$
 $\sum V = 0 \Rightarrow F_{AB} = -54 \text{ kN}$

Resolving at B:
 $\sum V = 0 \Rightarrow 19 - F_{AB} \cos 65 - F_{BC} \sin 15 = 0$
 $-19 + 54 - F_{BC} \cos 5 - F_{BD} \sin 15 = 0$
 $F_{BC} = 36 - F_{BD} \cos 5$
 $\sum H = 0 \Rightarrow -108 + F_{BC} \sin 65 + F_{BD} \cos 15 = 0$
 $F_{BC} = 108 - F_{BD} \cos 15$

$108 - F_{BD} \cos 15 = 36 - F_{BD} \cos 5$
 $108 \sin 15 - F_{BD} \sin 15 \cos 15 = 36 \cos 15 - F_{BD} \cos 5 \cos 15$
 $F_{BD} = 36 \cos 15 - 108 \sin 15$
 $F_{BD} = 39 \text{ kN}$
 $F_{BC} = 75 \text{ kN}$

Sizing Cable and Braces

17. Resolving at C:

$$\sum H = 0 \Rightarrow -F_{BC} \cos 15^\circ + F_{CE} \cos 15^\circ = 0$$

$$F_{BC} = F_{CE} = 75 \text{ kN}$$

Resolving at D:

$$\sum V = 0 \Rightarrow -18 + F_{CD} = 0$$

$$F_{CD} = 18 \text{ kN}$$

Resolving at A:

$$\sum H = 0 \Rightarrow -F_{BC} \cos 15^\circ + F_{AD} + F_{DE} = 0$$

$$-39 \cos 25^\circ + 108 + F_{DE} = 0$$

$$F_{DE} = -108 + 39 \cos 25^\circ$$

$$F_{DE} = -72 \text{ kN}$$

Resolving at E:

$$\sum H = 0 \Rightarrow -F_{BC} \cos 15^\circ + F_{DE} = 0$$

$$-75 \cos 15^\circ + F_{DE} = 0$$

$$F_{DE} = 75 \cos 15^\circ = 72.7 \text{ kN}$$

Final Force Diagram:

all in kN

Limit: $307 \text{ steel} = 110 \text{ MPa}$

$$\sigma = \frac{F}{A} \Rightarrow A = \frac{F}{\sigma}$$

$A_{AB} = \frac{54 \times 10^3}{110 \times 10^6} = 490.9 \text{ mm}^2 \rightarrow 48.3, 3$

$A_{BC} = \frac{75 \times 10^3}{110 \times 10^6} = 681.8 \text{ mm}^2 \rightarrow 76.1, 3$

$A_{CD} = \frac{18 \times 10^3}{110 \times 10^6} = 163.6 \text{ mm}^2 \rightarrow 33.7, 3$

$A_{DE} = \frac{72 \times 10^3}{110 \times 10^6} = 654.5 \text{ mm}^2 \rightarrow 76.1, 3$

$A_{AD} = \frac{108 \times 10^3}{110 \times 10^6} = 981.8 \text{ mm}^2 \rightarrow 88.9, 3$

$A_{DE} = \frac{72 \times 10^3}{110 \times 10^6} = 654.5 \text{ mm}^2 \rightarrow 76.1, 3$

Roof Design

19. Sizing Roof:

We will choose Kingspan metal insulated roof sheets. They will be 11mm span. change: 10mm

Sizing primary beams:

SPAN = 20

DEPTH = 20

depth = $\frac{16000}{20} = 800$

Looking for beams with depth = 300mm in the Blue Book:

UB 914 x 419 x 388 onwards.

$I = 720000 \text{ cm}^4$

We will therefore consider using a portal truss.

as UB 914 x 419 x 388 is too large of a beam.

Portal Truss:

Primary x-direction beams: UDL = 18 kN/m

Let's size the secondary beams.

There are 8 of them, spanning 5m each in y direction.

span = 5000 = 250

depth = 20

\therefore UB 305 x 102 x 33 with $I = 6500 \text{ cm}^4$

then divide the structure into 3 sections, 4 of 6x5m and 2 of 4x5m.

18. Sizing the cable:

$18 \times 4 = 72 \text{ kN}$

So the cable has to withstand a force of 72 kN.

$$A_{\text{cable}} = \frac{F}{\sigma_{\text{limit}}} = \frac{72 \times 10^3}{110 \times 10^6}$$

$$A_{\text{cable}} = 6.54 \text{ cm}^2$$

$$6.54 \div 4 = 1.63 \text{ cm}^2$$

$$1.63 = \pi r^2$$

$$r = 0.7 \text{ cm}$$

$$d = 1.4 \text{ cm}$$

at each cantilever will need 4 cables

Parallel wire strands diameter 1.4 cm

It will be a regular hexagonal arrangement of 4 strands

Sizing the side braces:

Total axial compression = $\sqrt{4} \times 5382 = 539 \text{ kN}$

Sizing from the Blue Book:

Looking at stress, moment and axial compression.

$\therefore d = 223 \text{ mm}$ and $t = 10 \text{ mm}$ where

$M = 1312 \text{ kNm}$

20. Therefore the Kingspan roof single span panels will be the following:

thickness = 115 mm, imposed load = 0.74 kN/m² span = 6m

and thickness = 60 mm, imposed load = 0.98 kN/m² span = 4m

Let's see if the chosen secondary beams can hold them.

For 6m span: $L = 6 \text{ m}$

UDL = $0.74 \times 6 = 4.4 \text{ kN/m}$

where $\sigma_{\text{max}} = \frac{5wL^4}{384EI}$ and $\sigma_{\text{limit}} = \frac{L}{180}$

Checking if chosen UB 305 x 102 x 33 works

$$\sigma_{\text{max}} \leq \sigma_{\text{limit}}$$

$$\frac{5(4.4 \times 10^3)(6^4)}{384(180 \times 10^6)(6.5 \times 10^{-5})} \leq \frac{6}{180}$$

$$0.003 \leq 0.027$$

therefore the beam is suitable.

Floors and Foundations

21. For 4m span:

UDL = $0.98 \times 4 = 3.92 \text{ kN/m}$

$\sigma_{\text{max}} \leq \sigma_{\text{limit}}$

$$\frac{5wL^4}{384EI} \leq \frac{L}{180}$$

$$\frac{5(3.92 \times 10^3)(4^4)}{384(180 \times 10^6)(6.5 \times 10^{-5})} \leq \frac{4}{180}$$

$$0.0027 \leq 0.027$$

Therefore it works too.

22. Designing floor beams and slabs:

span = 20

depth = $\frac{6000}{20} = 300 \text{ mm}$

\therefore UB 356 x 127 x 33 $I = 8250 \text{ cm}^4$

with a UDL = $4.7 \text{ k/m}^2 \times 5 \text{ m} = 23.5 \text{ kN/m}$

$\sigma_{\text{max}} \leq \sigma_{\text{limit}}$

$$\frac{5wL^4}{384EI} \leq \frac{L}{180}$$

$$\frac{5(23.5 \times 10^3)(6^4)}{384(180 \times 10^6)(8.25 \times 10^{-5})} \leq \frac{6}{180}$$

$$0.027 \leq 0.03$$

It works but is too close to the limit. \therefore UB 356 x 171 x 67, $I = 19100 \text{ cm}^4$ is chosen which gives:

$\sigma_{\text{max}} = 0.011 \leq 0.03$ in some cases, trusses will be used.

Sizing composite slabs:

span = 20 for single spans

depth = $\frac{5000}{20} = 250 \text{ mm}$

and for end spans $\frac{L}{5} = 4 \text{ m}$

depth = $\frac{5000}{26} = 192 \text{ mm}$

Foundations:

Pile foundations are used as it is little for clay soils.

1000×9

CURATOR'S NOTE:

Go to Appendix to see the Blue Notes to see the full scope of the 'back of the envelope' calculations.

8th June 2023 London, UK
Maxime Ostroverhyy

1:20 CLAY EXTRUSIONS THAT ARE SIZED BASED ON CALCULATIONS



trusses in columns:
1 - d = 508mm, t = 12mm
2 - d = 219.1mm, t = 5mm
3 - d = 76.1mm, t = 4mm

cable :
4 - d = 14mm, x7

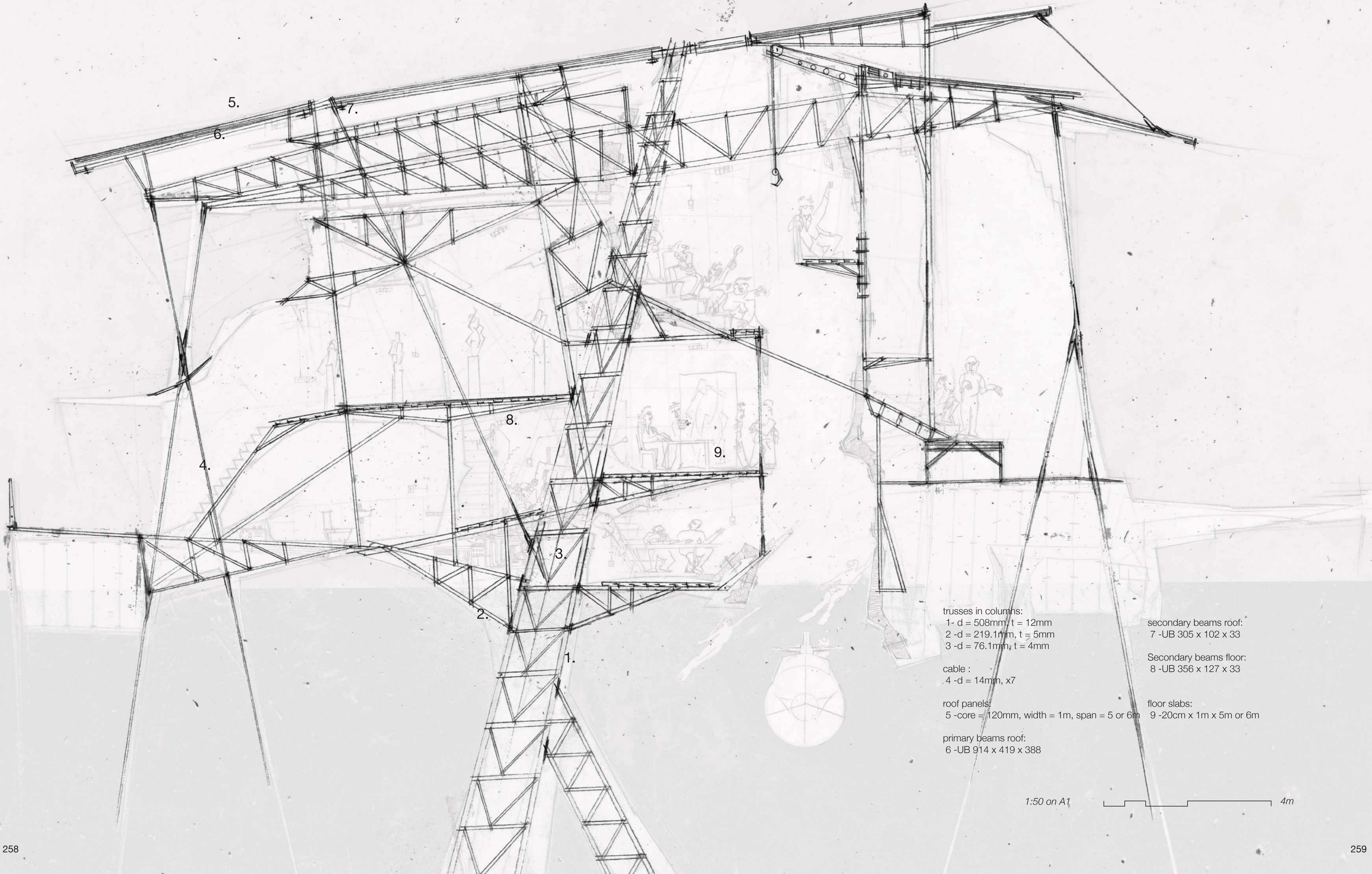
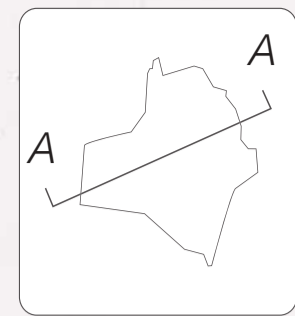
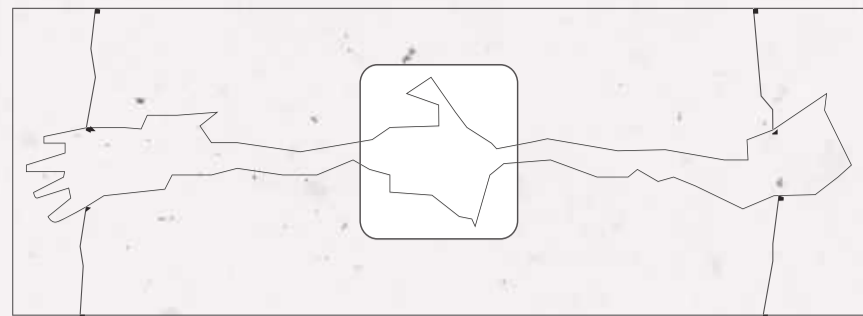
roof panels:
5 - core = 120mm, width = 1m, span = 5 or 6m

primary beams roof:
6 - UB 914 x 419 x 388

secondary beams roof:
7 - UB 305 x 102 x 33

Secondary beams floor:
8 - UB 356 x 127 x 33

floor slabs:
9 - 20cm x 1m x 5m or 6m



trusses in columns:
1 - d = 508mm, t = 12mm
2 - d = 219.1mm, t = 5mm
3 - d = 76.1mm, t = 4mm

cable :
4 - d = 14mm, x7

roof panels:
5 - core = 120mm, width = 1m, span = 5 or 6m

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6 - UB 914 x 419 x 388

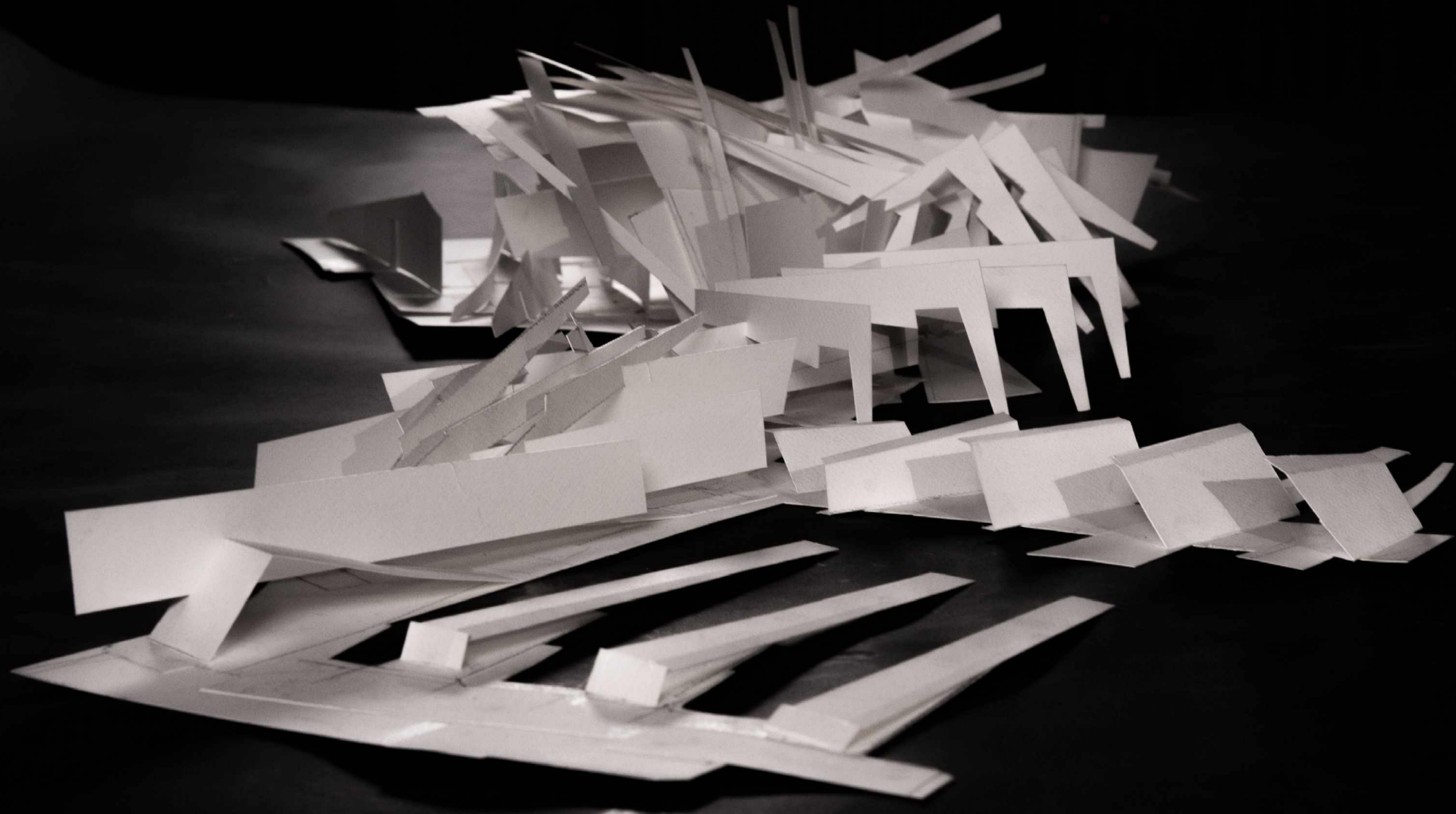
secondary beams roof:
7 - UB 305 x 102 x 33

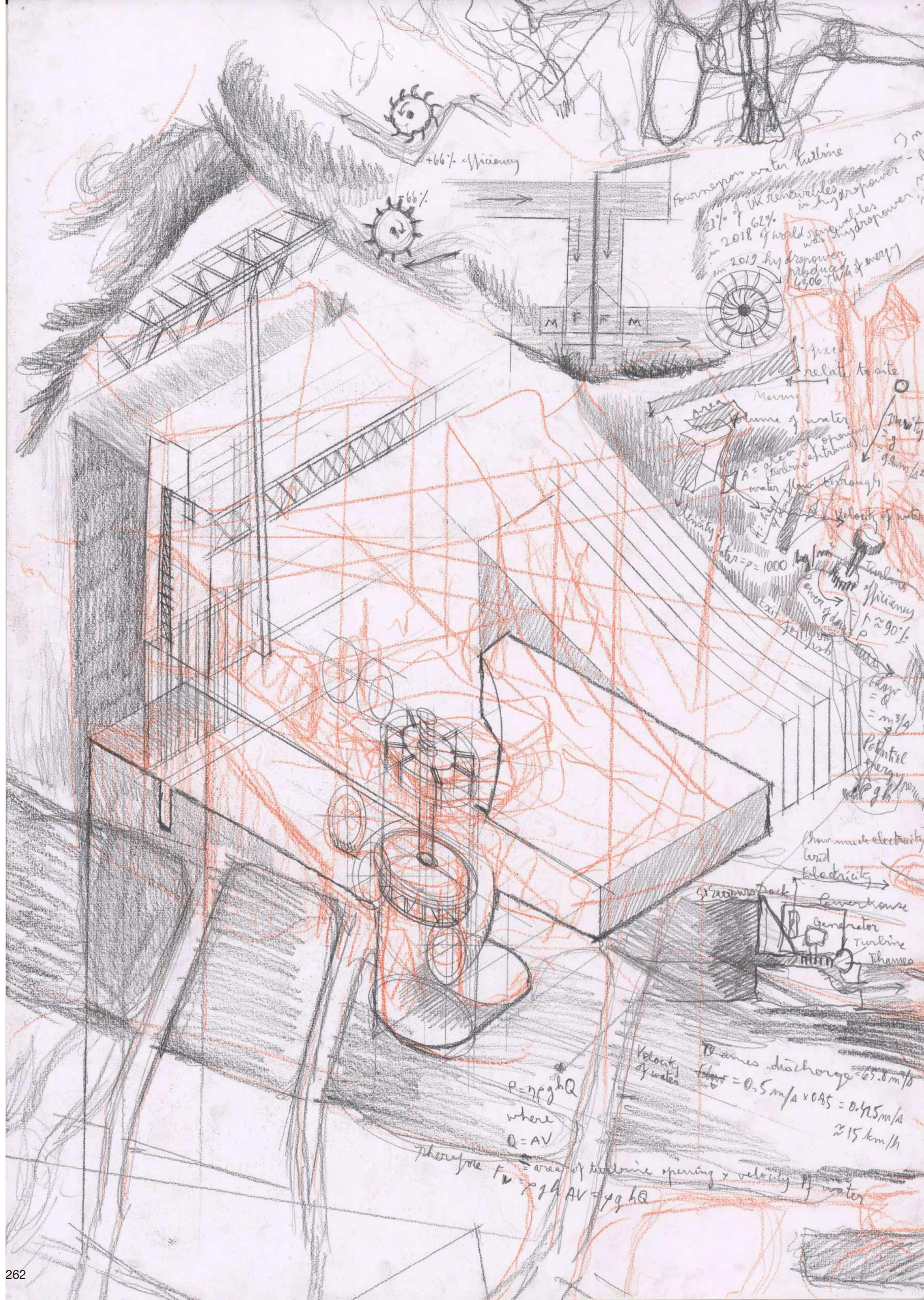
Secondary beams floor:
8 - UB 356 x 127 x 33

floor slabs:
9 - 20cm x 1m x 5m or 6m

1:50 on A1



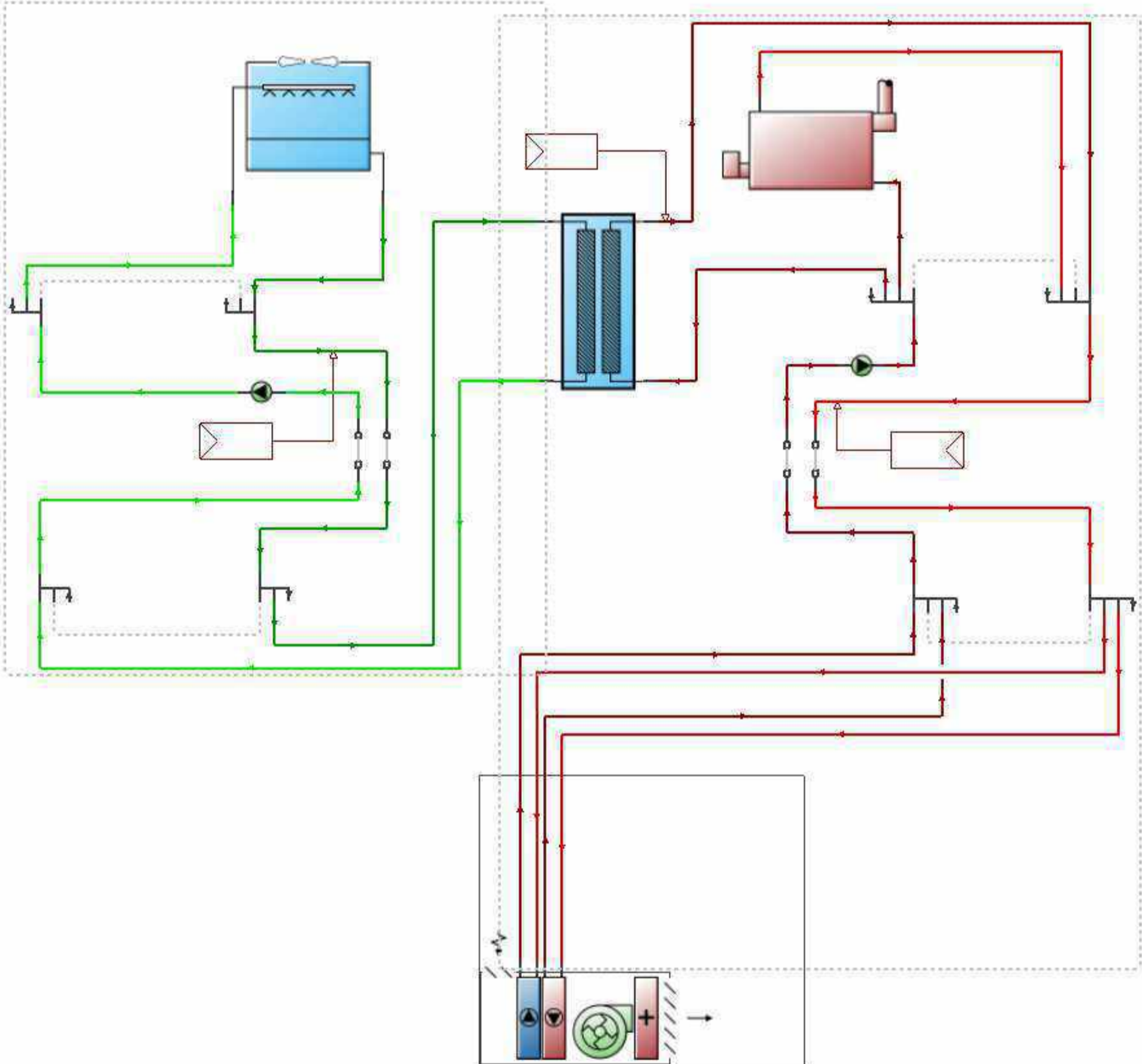




Section 14. Environmental Design Strategy: Hydropower

The main environmental renewable is hydropower, which helps the building to reach net carbon zero. Sustainability remains very important. The auction house is over the River Thames, and therefore hydropower would be the most convenient and sustainable source of energy for the building to be able to function properly. As such, whenever a building is created, its environment had to be studied in order to determine which course of action would best. A detailed HVAC and relevant collected data concerning everything that is needed to create an energy design for the auction house had to be completed with meticulous care and attention. Overall, the result was satisfactory and I remain confident that the environmental design strategy would work if it is ever to be applied. The building must strive to be sustainable and the best way to do that is to see which renewable energy is best for the auction house through a matrix of iterations using Design Builder.

Detailed HVAC



Detailed HVAC iteration 51: GSHP

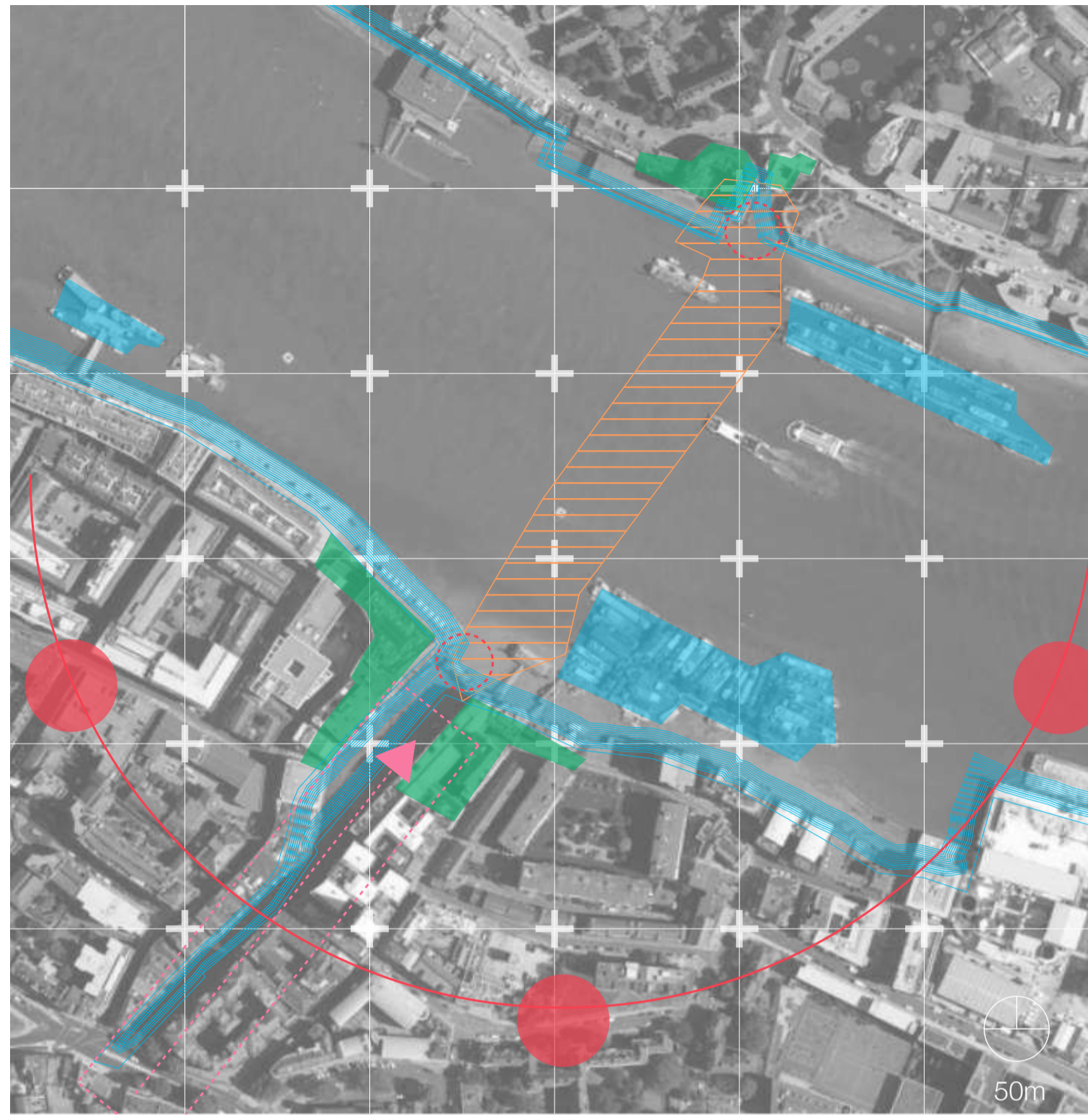
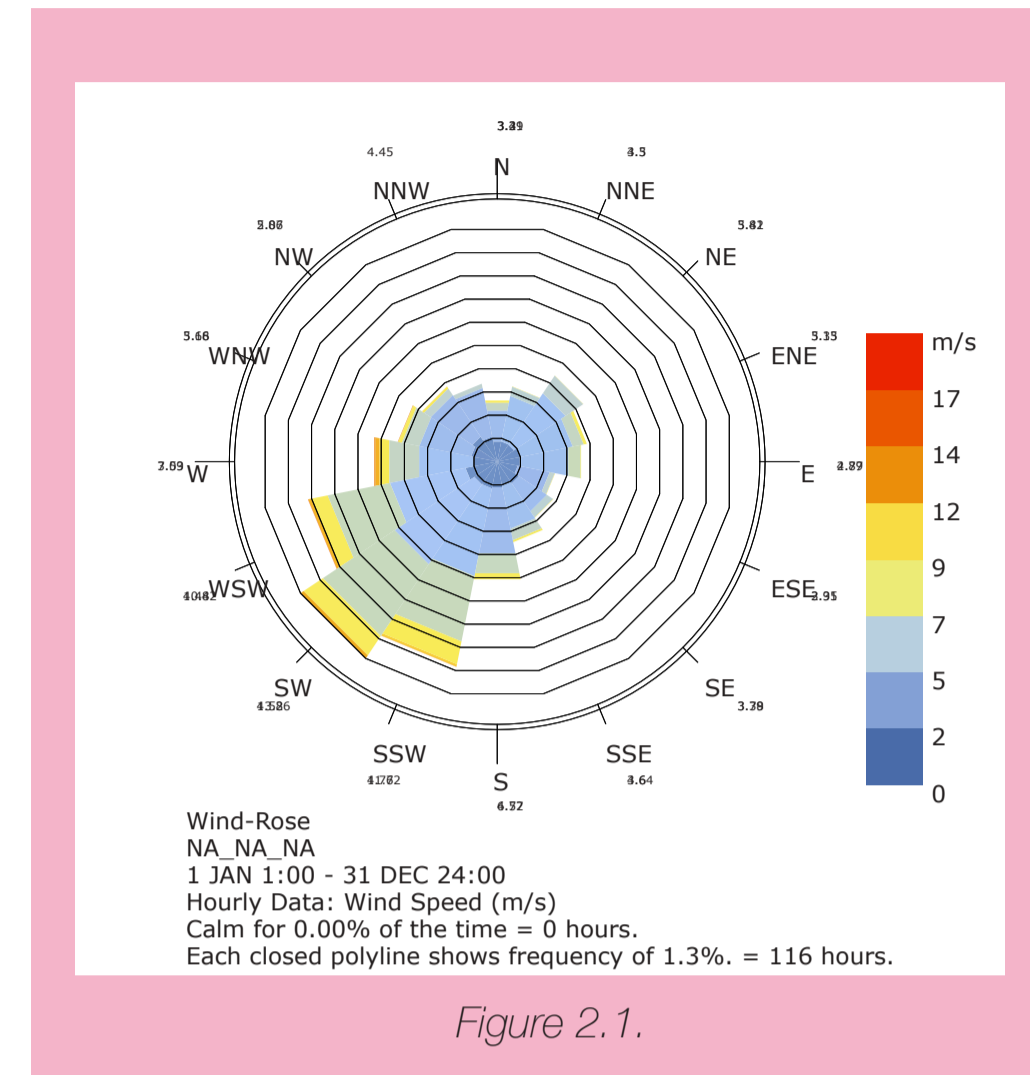


Environmental Site Analysis

Introduction:

Site coordinates:
 51.5021° N, 0.0714° W (St Saviours Dock)
 Closest data taken was:
 54.9756° N, 1.6217° W (St James Park)

Wind



Tides

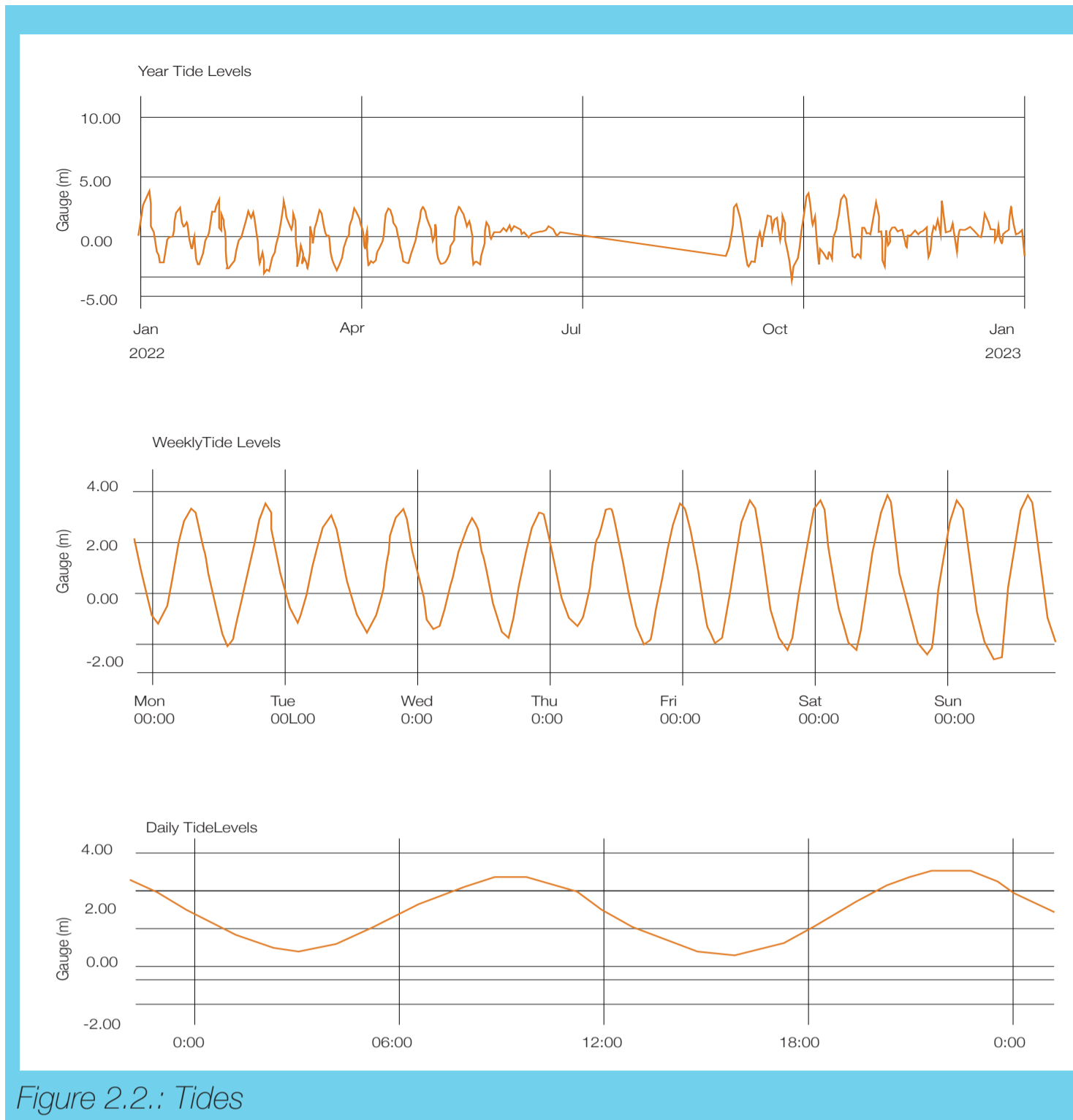
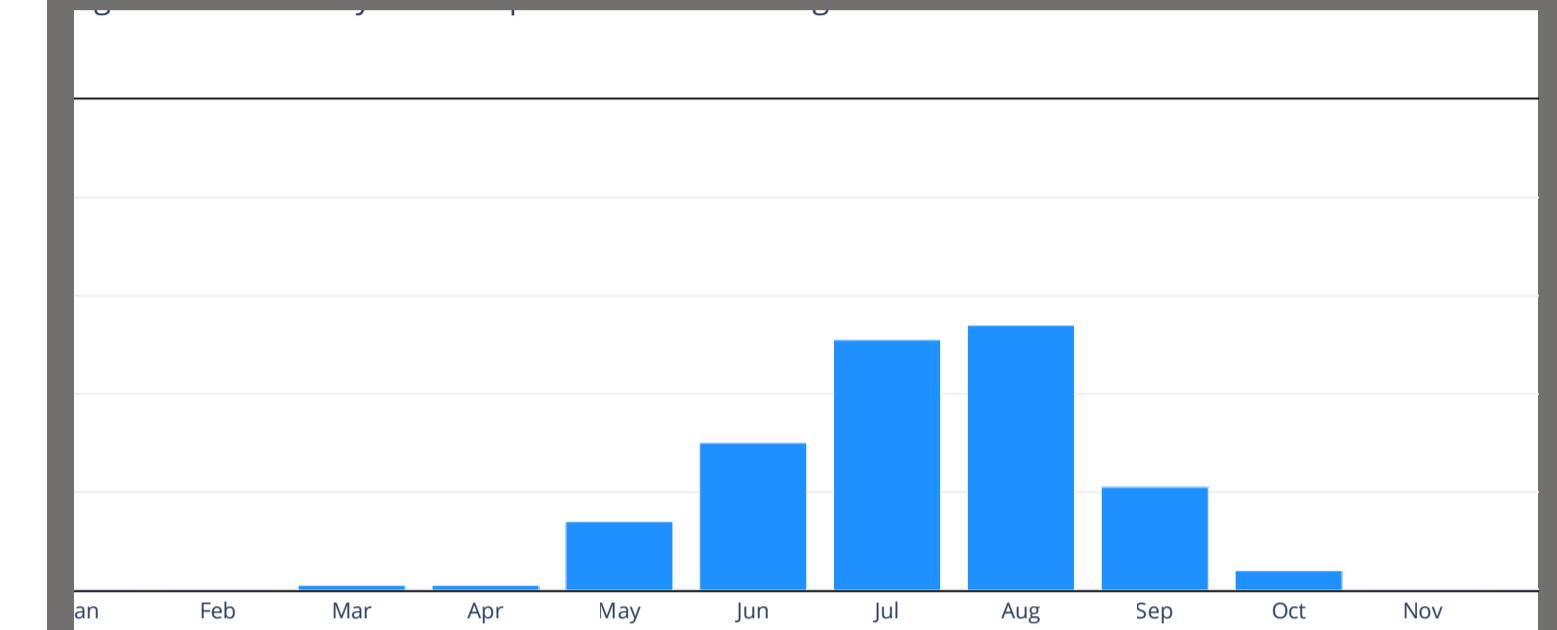
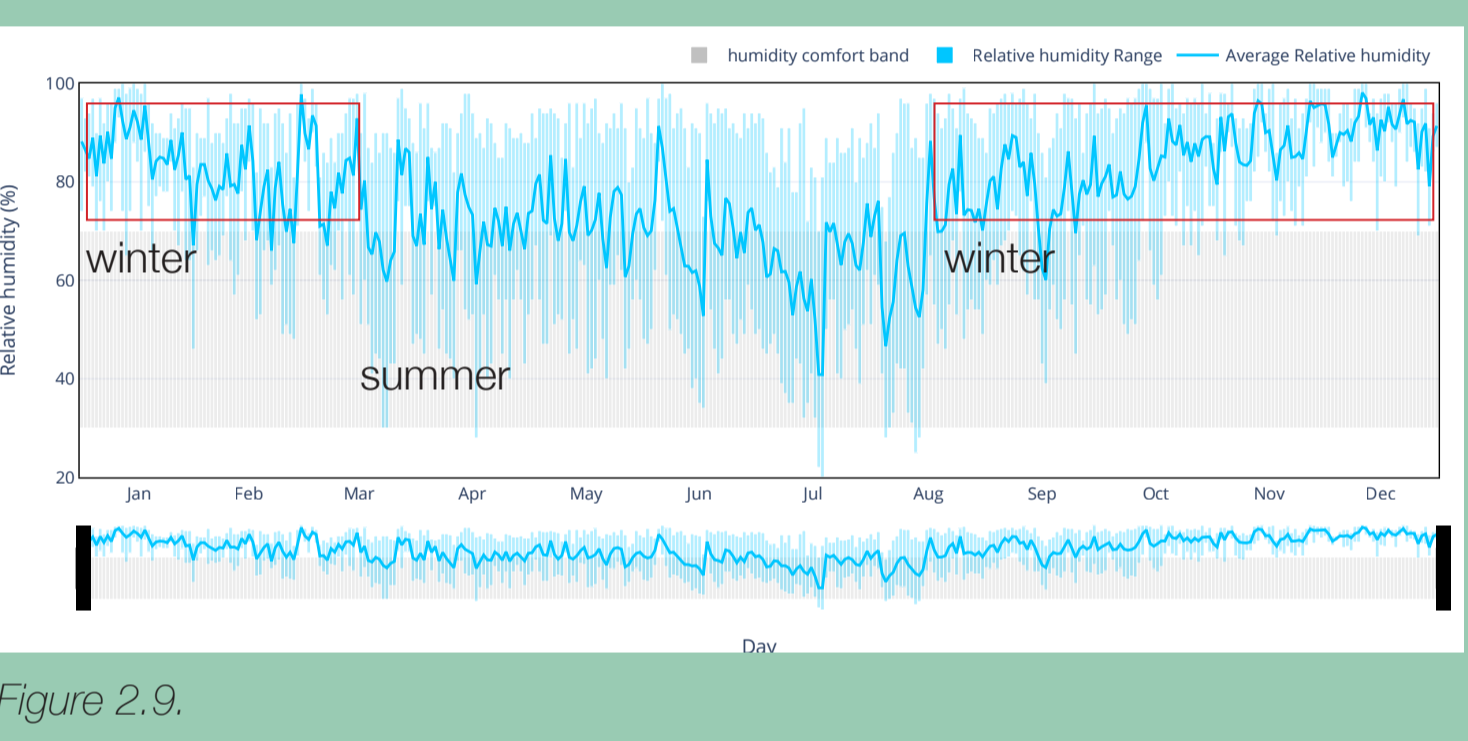
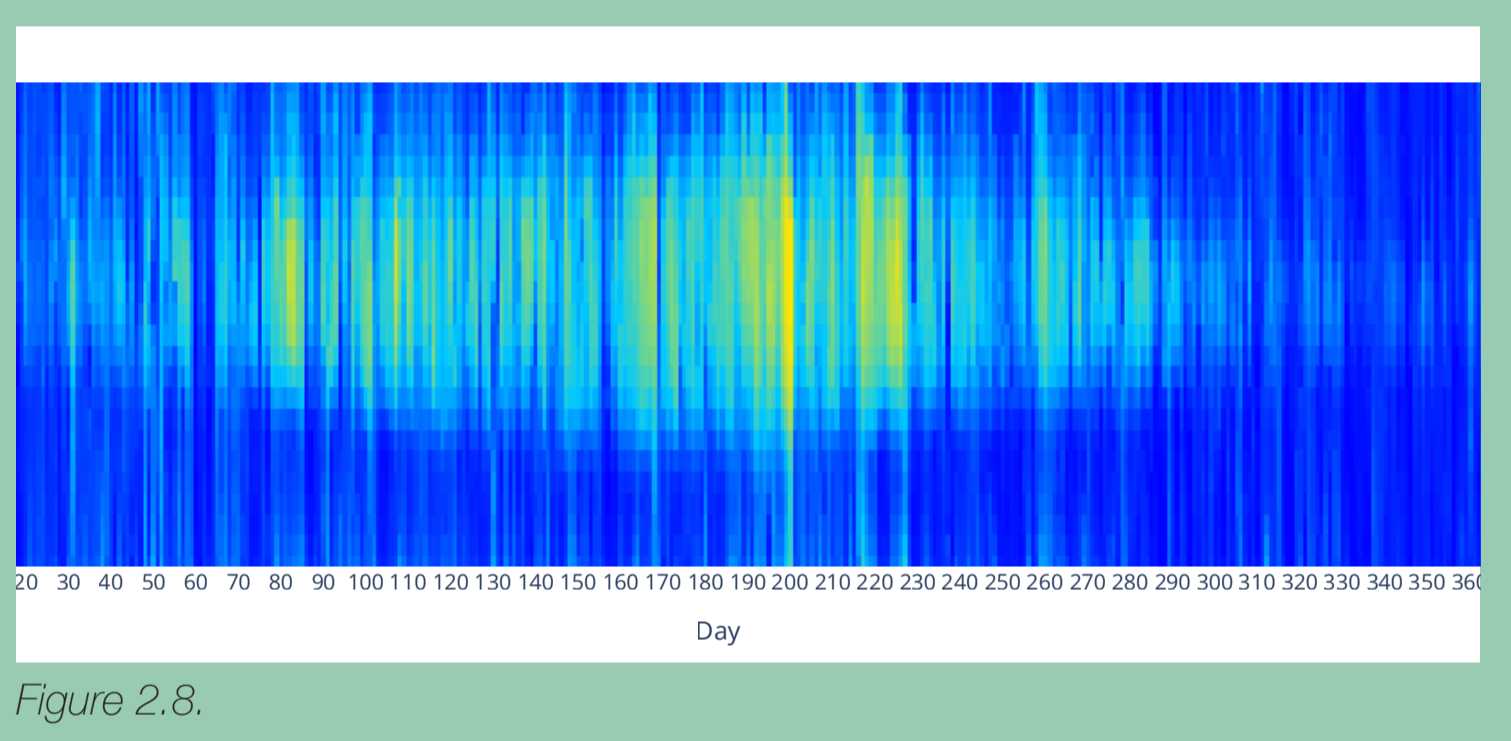
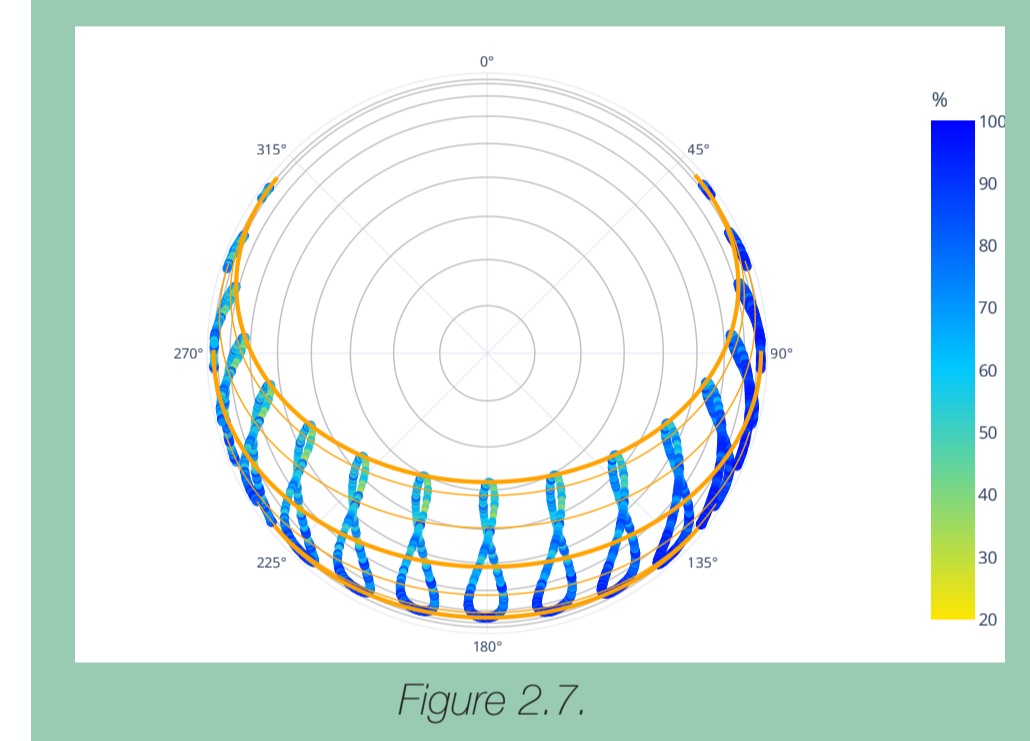


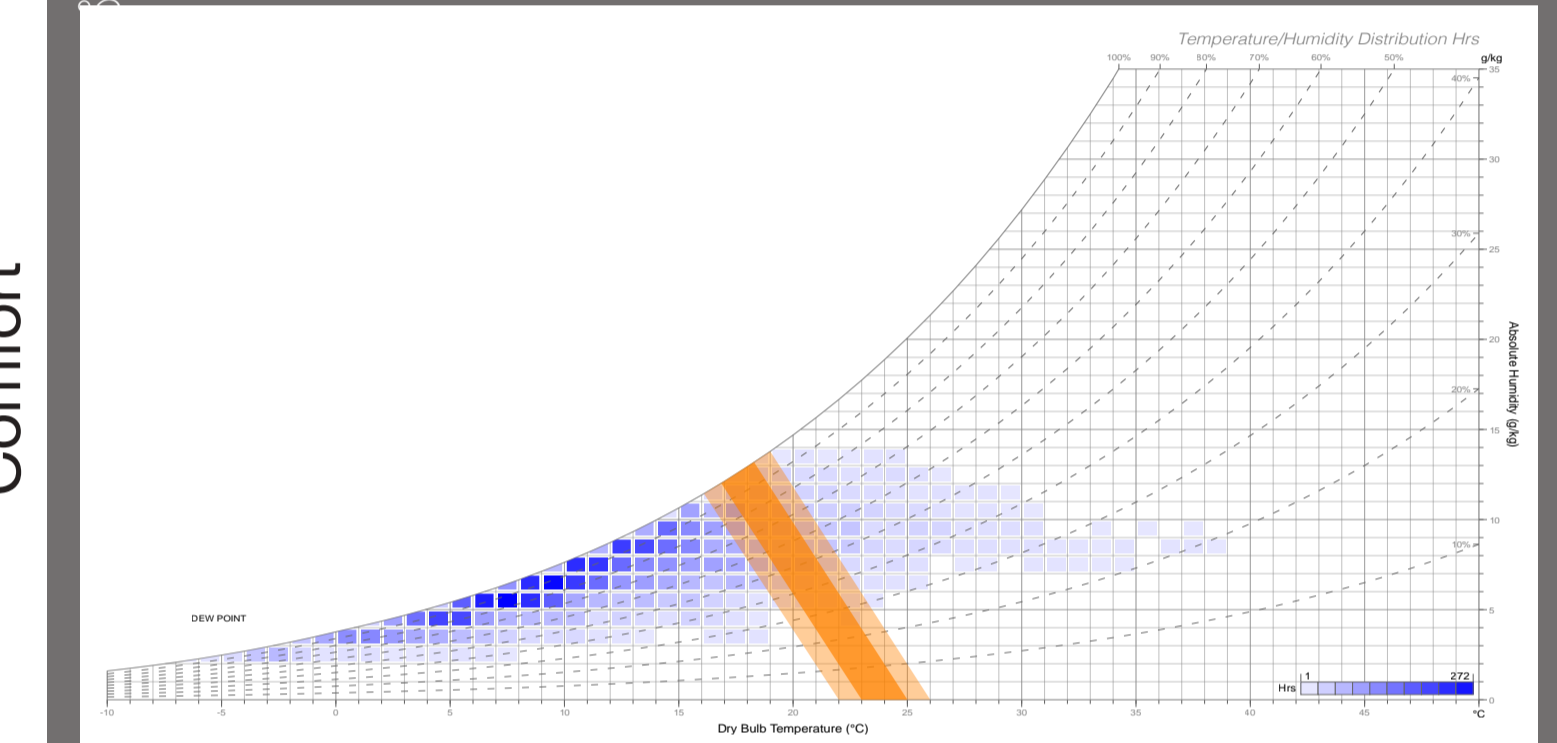
Figure 2.3.: Heating and cooling days



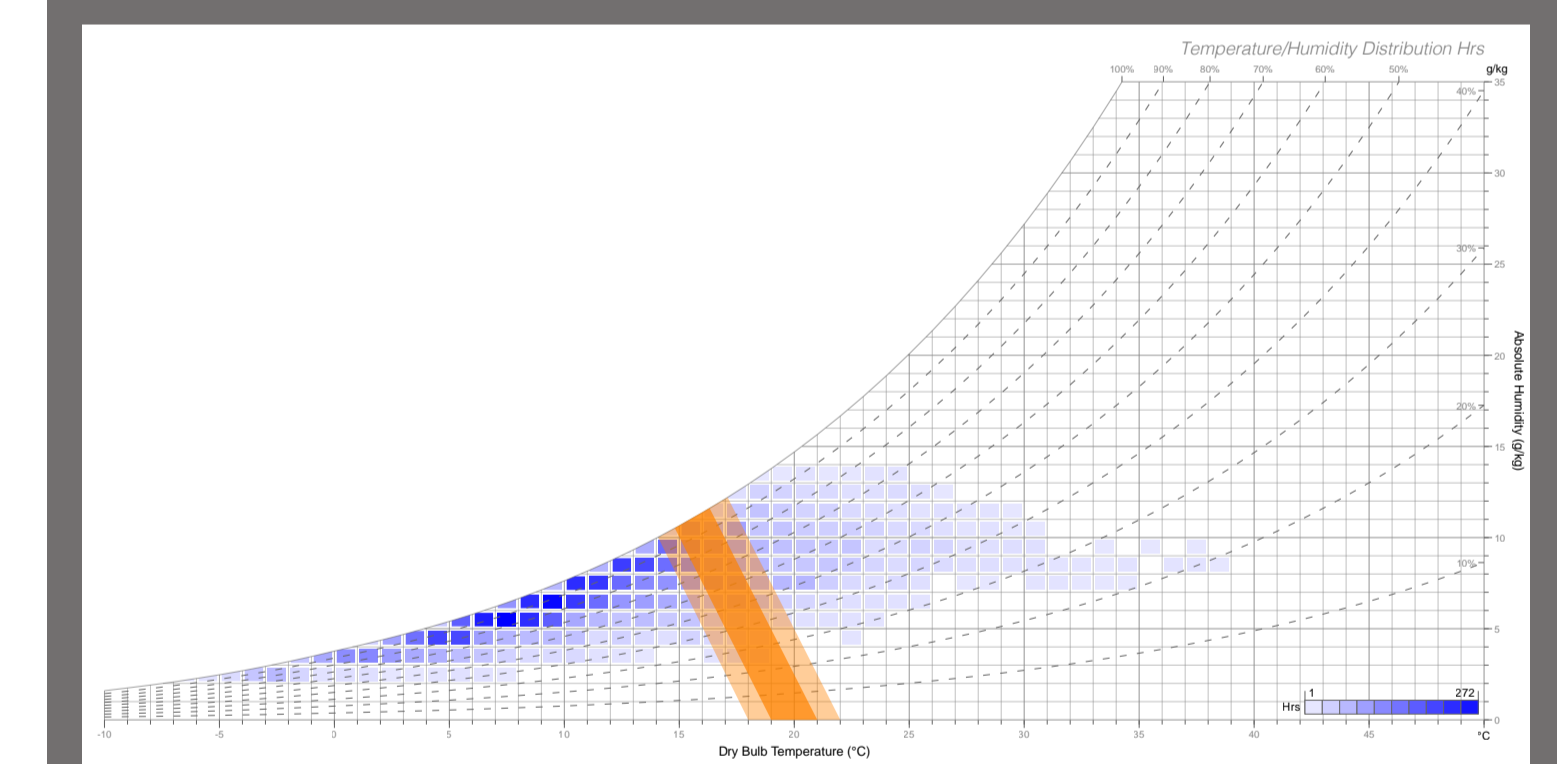
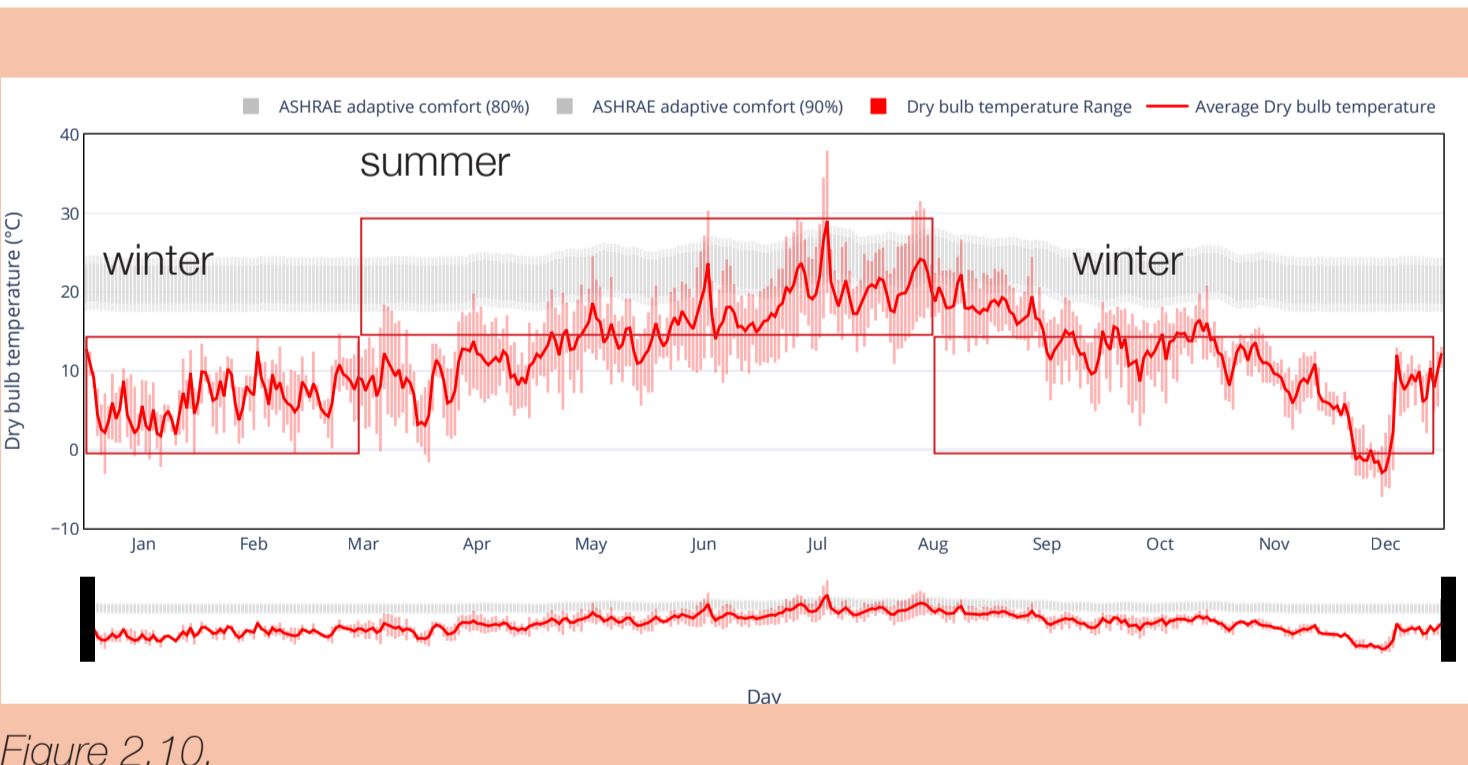
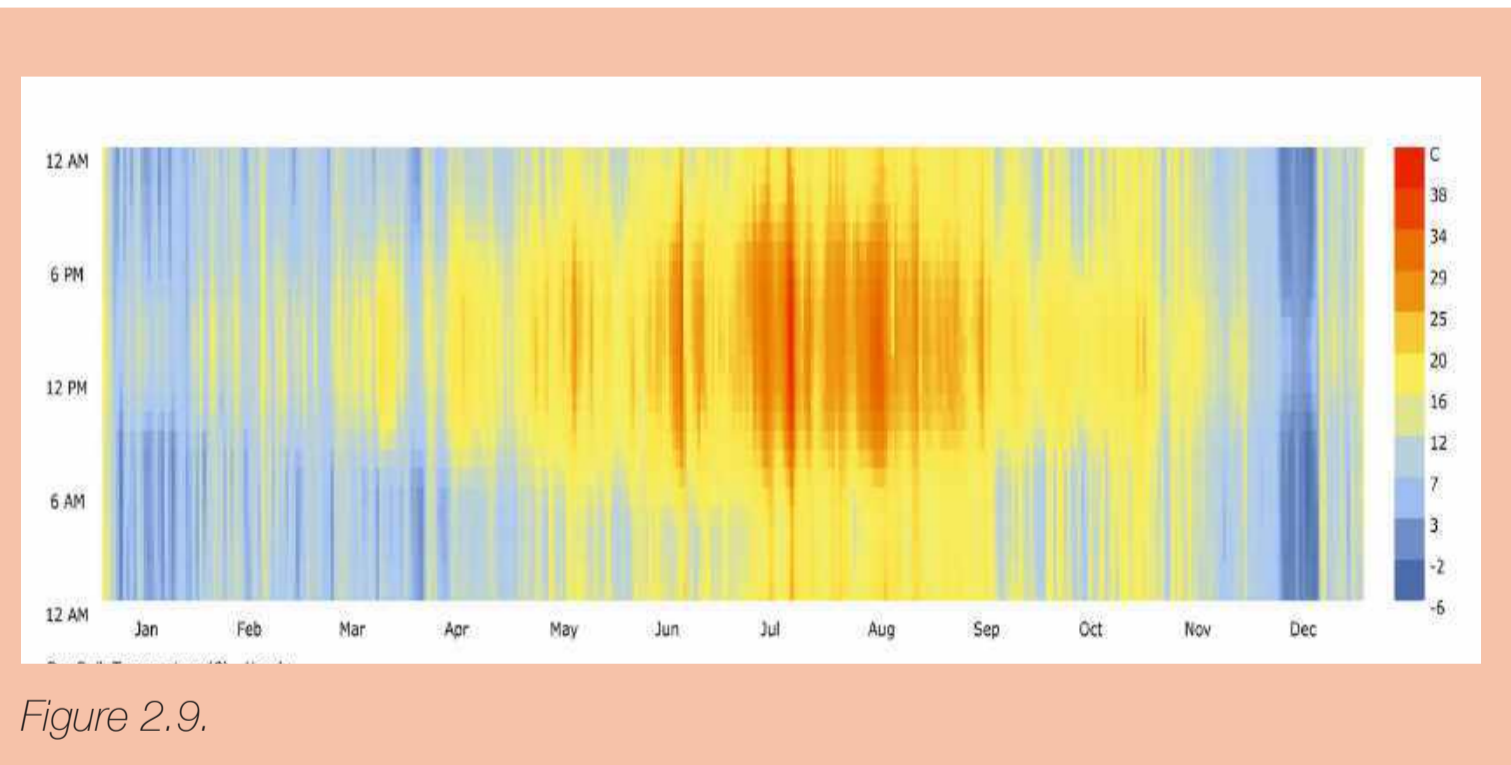
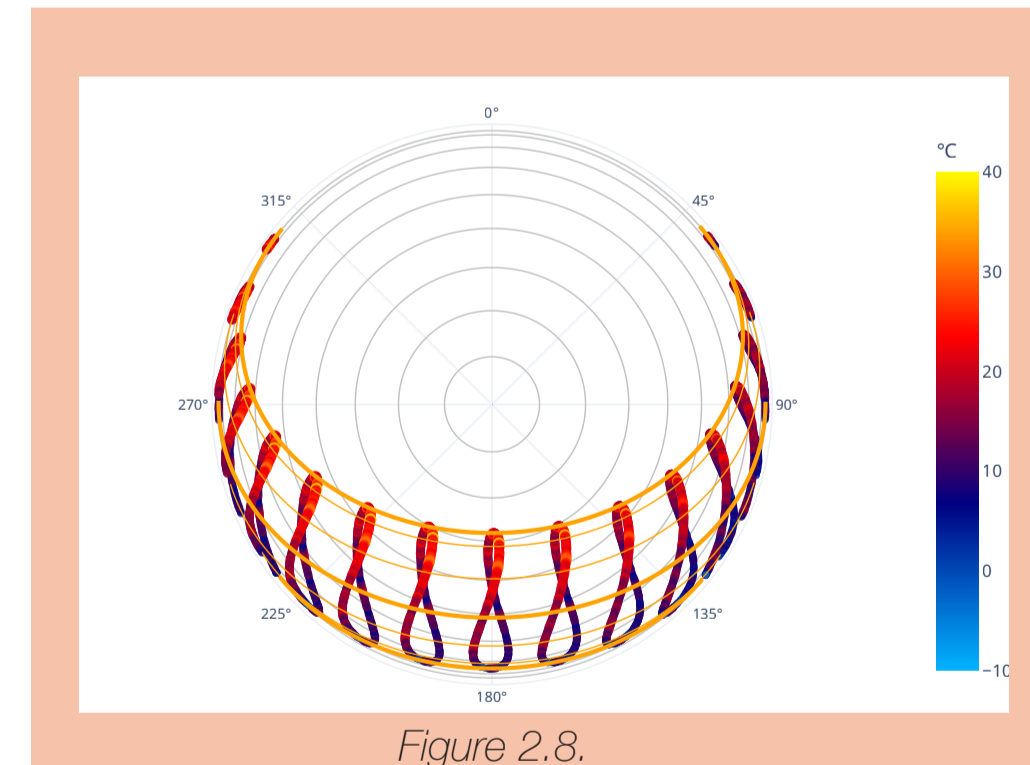
Relative humidity



Comfort



Dry bulb temperature



Conclusion:

Wind:
 A wind tunnel is created with the adjacent buildings and prevalent winds coming from the south (fig.2.1.). Potential opportunity for wind powered energy source.

Comfort:
 It is clear from fig.2.3. as well as 2.5. and 2.6. that the building requires much more heating strategies than cooling strategies. However, some storage room require a more stable temperature so slightly more cooling might be necessary.

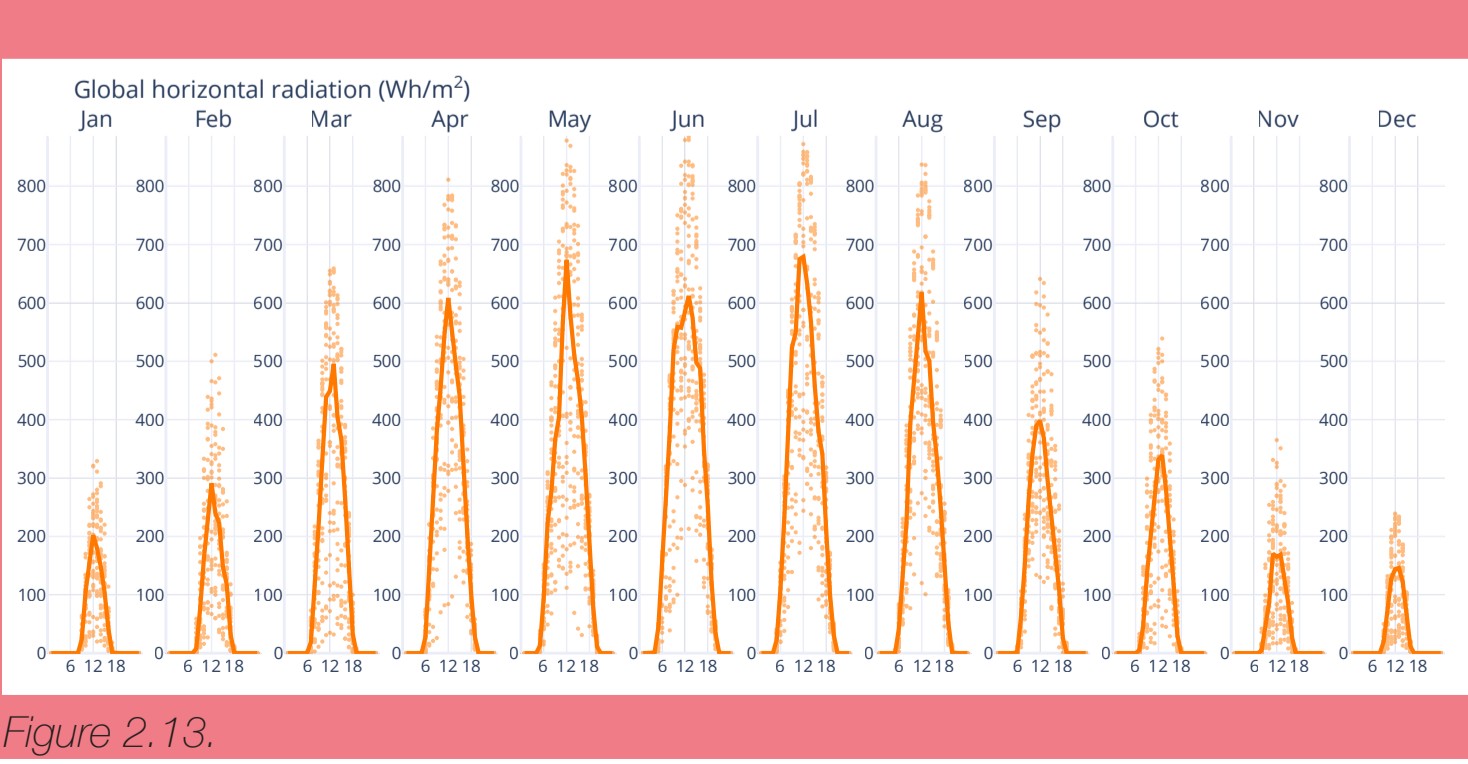
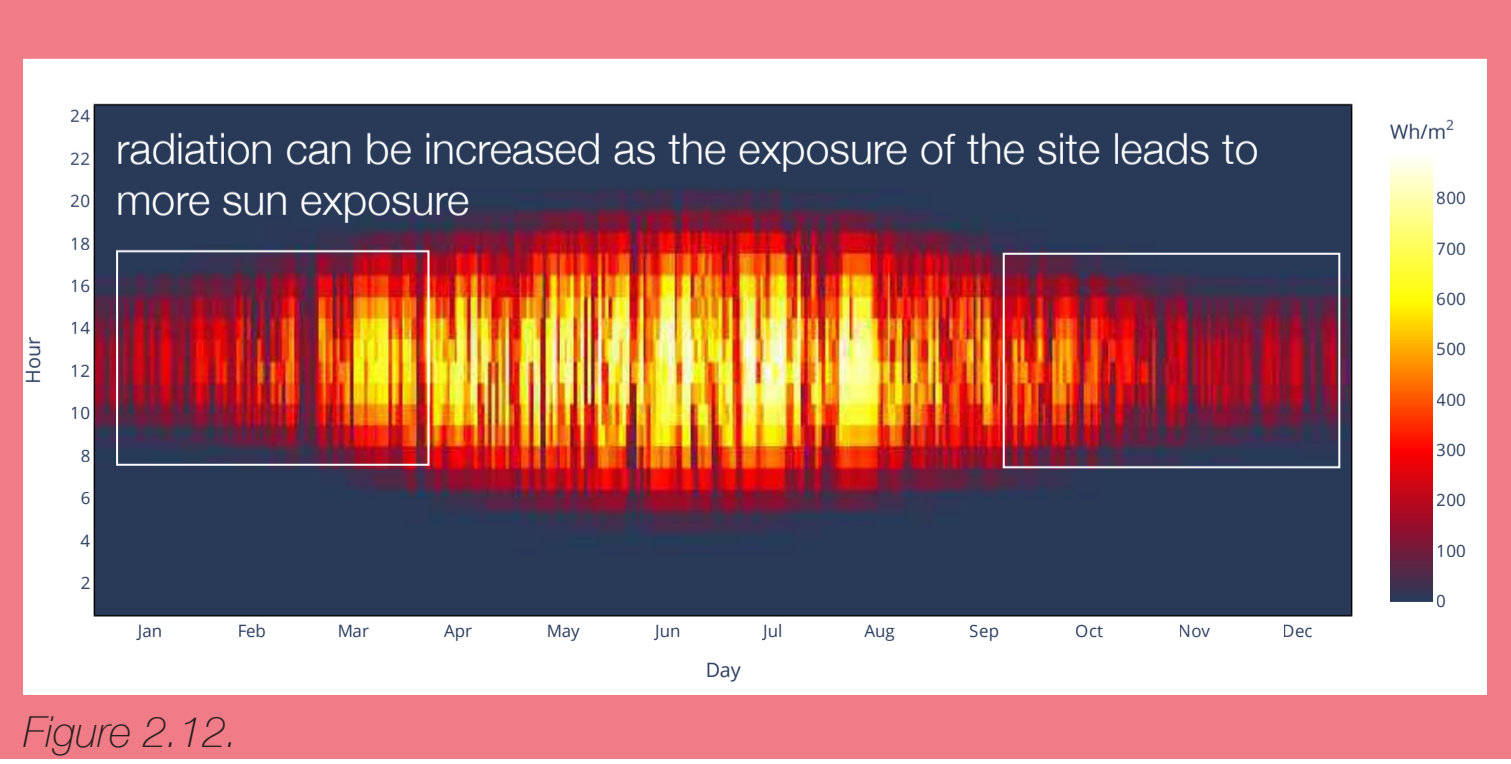
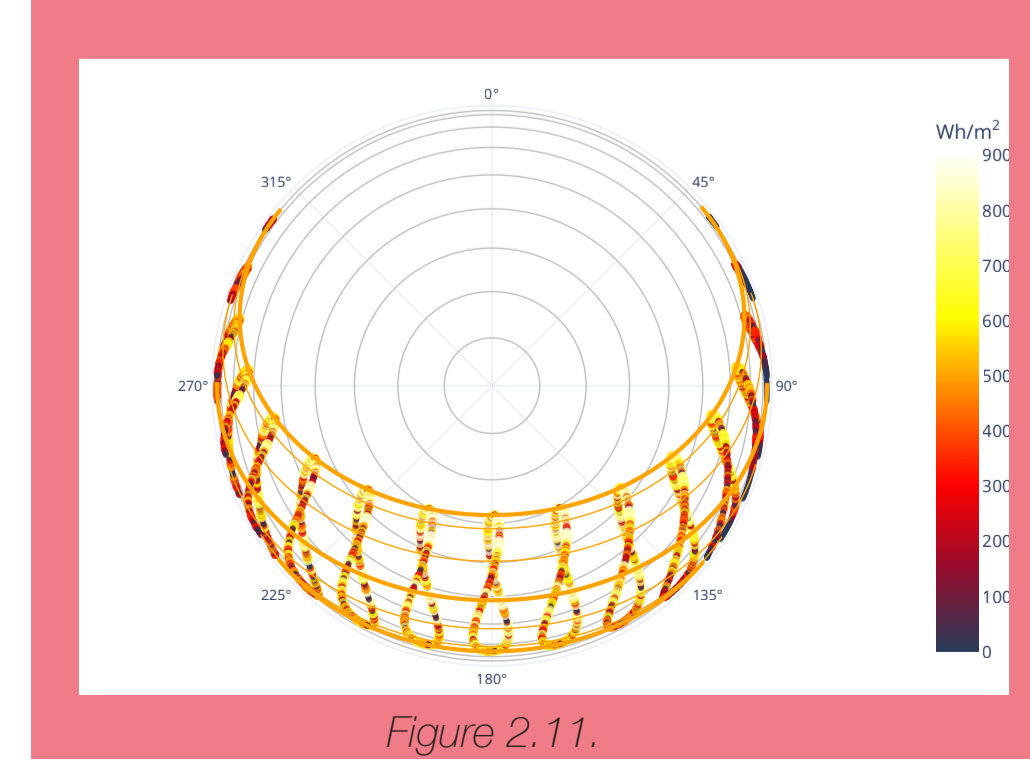
Tides:
 Highlighted on the map are the communities affected by the tide, they live with the tide. It is a great opportunity for hydropower, and dam designs that can be filled up up to twice per day (fig. 2.3.).

Relative Humidity:
 The site being on water, nearby buildings can be taken into account in terms of how they deal with high humidity.

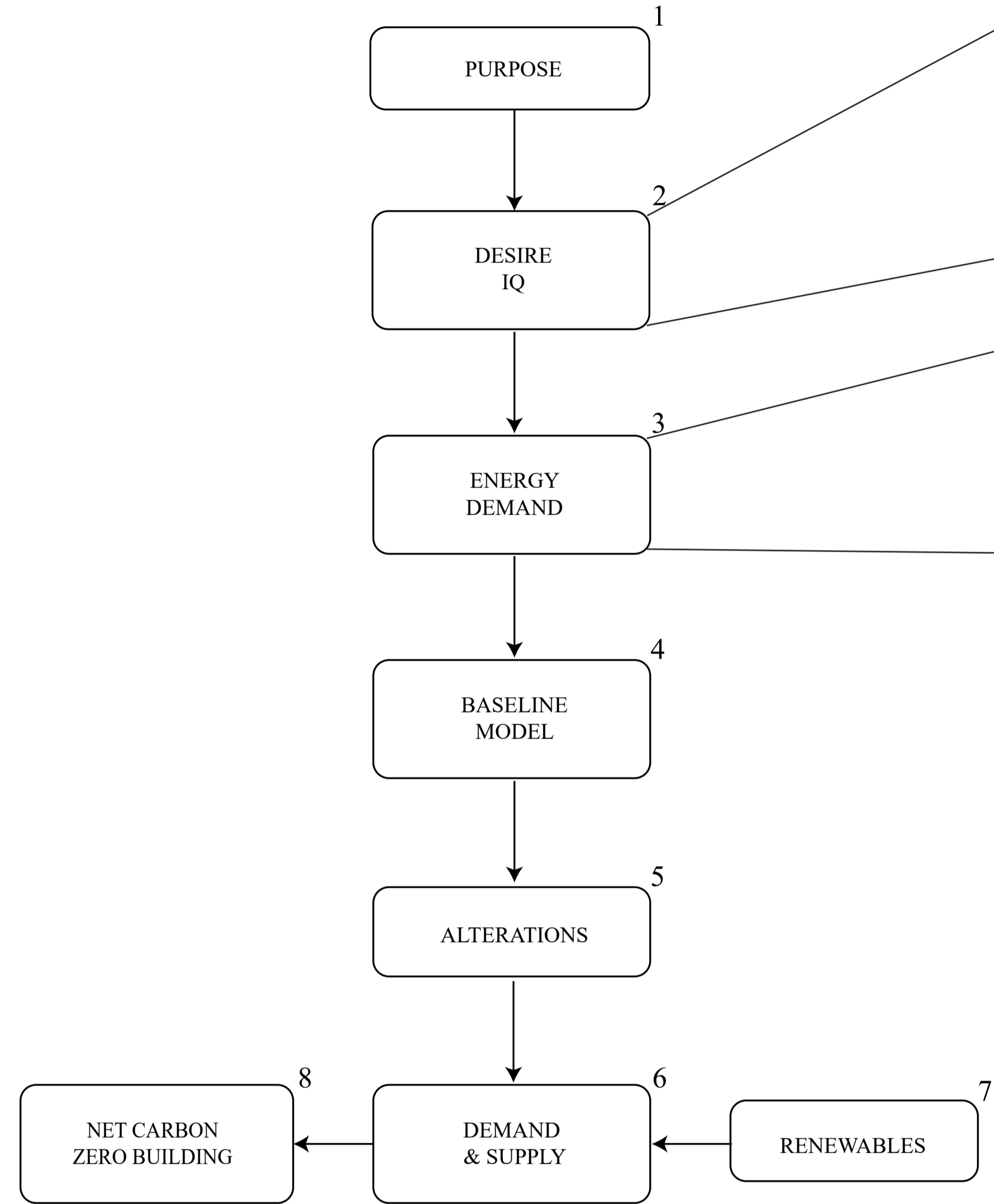
Dry bulb temperature:
 Fig. 2.8., 2.9. and 2.0. do not show high temperatures, and it seems that extremely low temperatures are not very common either. There is a tendency for the site to be in the range of being in the range of 12 to 16 degrees.

Direct Solar Radiation:
 The site being on the Thames it is constantly exposed to solar radiation, so the graphs could be changed and thus radiation increased by 10%. No need for shadow and radiance analysis, as the center of the river is constantly exposed to sun. So for air drying clay, vessels can be sent onto the Thames for it to dry on the roof.

Direct normal radiation



Environmental Design Goals



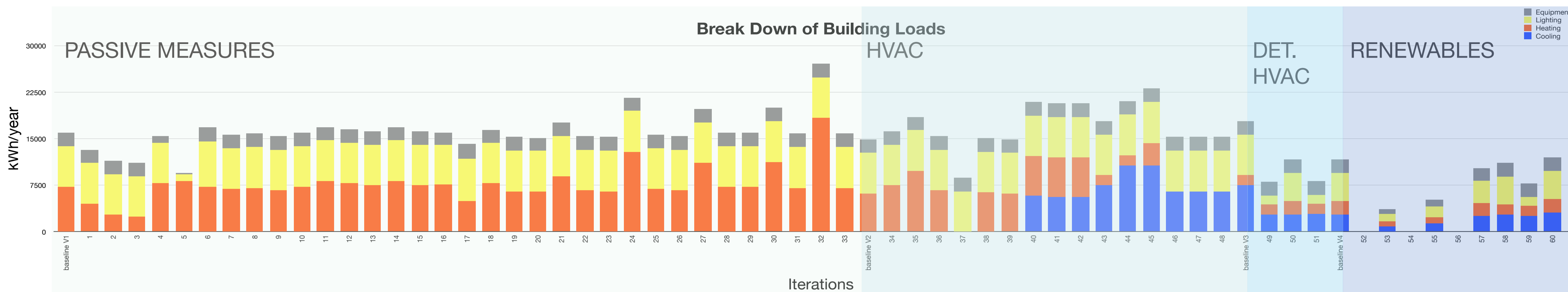
KEYSPACES NEEDS and DEMANDS								
	total building area = 182m ²	zone 1	zone 2	zone 3	zone 4	zone 5	zone 6	zone 7
ROOM TYPE	AUCTION ROOM	BOILER ROOM	CORRIDOR	GALLERY	PRIVATE SALE ROOM	STORAGE ROOM	TOILETS	
floor area (m2)	81	1.2	28	67	14	17	2.5	
people	97.2	1	3	10	2	4		
occupancy (people/m2)	1.2	0.833	0.107	0.149	0.143	0.235	0.800	
temp (°C)	21-23	19-21	19-21	19-21	21-23	19-21	19-21	
RH (%)	40-60	40-60	40-60	40-60	40-60	40-60	40-60	
activity (met)	1.2	n/a	1.4	1.4	1.4	1.4	n/a	
clothing (clo)	0.9	n/a	1	1	1	1	n/a	
temp (°C)	22-25	21-25	21-25	21-25	21-25	21-25	21-25	
RH (%)	40-60	40-60	40-60	40-60	40-60	40-60	40-60	
activity (met)	1.1	n/a	1.3	1.3	1.3	1.3	n/a	
clothing (clo)	0.6	n/a	0.6	0.6	0.6	0.6	n/a	
suggested air supply rate (L-s ⁻¹ per person)	11.5	11.5	11.5	11.5	11.5	11.5	11.5	
maintained illuminance (lux)	200	100	100	200	200	50	100	
noise criterion (NR)	30	n/a	40	35	30-35	30-35	n/a	
noise criterion (dBA)	35	n/a	45	40	35-40	35-40	n/a	
noise criterion (dBC)	60	n/a	70	65	60-65	60-65	n/a	

GOALS AND BENCHMARKS									
ROOM TYPE	zone 1 AUCTION ROOM	zone 2 BOILER ROOM	zone 3 CORRIDOR	zone 4 GALLERY	zone 5 PRIVATE SALE ROOM	zone 6 STORAGE ROOM	zone 7 TOILETS	TOTAL	PASSIV HAUS
electricity good practice benchmark (kWh/m2/yr)	52	n/a	39	39	39	37	n/a	41	24
electricity typical benchmark (kWh/m2/yr)	60	n/a	55	55	55	39	n/a	52.8	24
fossil thermal good practice benchmark (kWh/m2/yr)	125	n/a	79	79	79	283	n/a	129	15
fossil thermal typical benchmark (kWh/m2/yr)	190	n/a	121	121	121	311	n/a	172.8	15
total electricity (kWh/yr)	4212	0	1092	2613	546	629	0	9092	



On Site Renewables and Storage

situation		iteration 1	iteration 2	iteration 3	iteration 4	iteration 5	iteration 6	iteration 7	iteration 8	iteration 9	iteration 10	iteration 11	iteration 12	iteration 13	iteration 14	iteration 15	iteration 16	iteration 17	iteration 18	iteration 19	iteration 20	iteration 21	iteration 22	iteration 23	iteration 24	iteration 25	iteration 26	iteration 27	iteration 28	iteration 29	iteration 30	iteration 31	iteration 32	iteration 33	iteration 34	iteration 35	iteration 36	iteration 37	iteration 38	iteration 39	iteration 40	iteration 41	iteration 42	iteration 43	iteration 44	iteration 45	iteration 46	iteration 47	iteration 48	iteration 49	iteration 50	iteration 51	iteration 52	iteration 53	iteration 54	iteration 55	iteration 56	iteration 57	iteration 58	iteration 59	iteration 60	iteration 61	iteration 62
PROBIE MEASURES	explained	[Detailed explanation of simulation iterations]																																																													
	occupancy	[Occupancy data for each iteration]																																																													
	activity	[Activity data for each iteration]																																																													
	habitation	[Habitation data for each iteration]																																																													
	roof shading (%)	[Roof shading data for each iteration]																																																													
	walls thickness (increase thermal mass)	[Walls thickness data for each iteration]																																																													
	external wall u-value (W/m2.K)	[External wall u-value data for each iteration]																																																													
	roof u-value (W/m2.K)	[Roof u-value data for each iteration]																																																													
	weight (tonnes/m2) (Wind or Wind ratio %)	[Weight data for each iteration]																																																													
	mechanical ventilation	[Mechanical ventilation data for each iteration]																																																													
HVAC	cooling	[Cooling data for each iteration]																																																													
	heating	[Heating data for each iteration]																																																													
	humidity control	[Humidity control data for each iteration]																																																													
	fan coil unit, air cooled chiller	[Fan coil unit, air cooled chiller data for each iteration]																																																													
	fan coil unit, air cooled chiller	[Fan coil unit, air cooled chiller data for each iteration]																																																													
	fan coil unit, air cooled chiller	[Fan coil unit, air cooled chiller data for each iteration]																																																													
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	fan coil unit, air cooled chiller	[Fan coil unit, air cooled chiller data for each iteration]																																																													
DETAILED HVAC	Heat pumps	[Heat pumps data for each iteration]																																																													
	Hydropower	[Hydropower data for each iteration]																																																													
	Solar panels roof coverage	[Solar panels roof coverage data for each iteration]																																																													
	Geothermal energy	[Geothermal energy data for each iteration]																																																													
	total energy use (kWh/year)	[Total energy use data for each iteration]																																																													
	cooling load (kWh/year)	[Cooling load data for each iteration]																																																													
	heating load (kWh/year)	[Heating load data for each iteration]																																																													
	interior lighting electricity	[Interior lighting electricity data for each iteration]																																																													
	interior lighting electricity	[Interior lighting electricity data for each iteration]																																																													
	interior lighting electricity	[Interior lighting electricity data for each iteration]																																																													
RENEWABLES	total energy (kWh/year)	[Total energy data for each iteration]																																																													
	cooling load (kWh/year)	[Cooling load data for each iteration]																																																													
	heating load (kWh/year)	[Heating load data for each iteration]																																																													
	interior lighting electricity	[Interior lighting electricity data for each iteration]																																																													
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	interior lighting electricity	[Interior lighting electricity data for each iteration]																																																													
PEOPLE	CONCORDIA (E)	[CONCORDIA (E) data for each iteration]																																																													
	ACTIVATION (E)	[ACTIVATION (E) data for each iteration]																																																													
	STORAGE (E)	[STORAGE (E) data for each iteration]																																																													
	PRINCEALBERT (E)	[PRINCEALBERT (E) data for each iteration]																																																													
	TOILETS (E)	[TOILETS (E) data for each iteration]																																																													
	GALLERY (E)	[GALLERY (E) data for each iteration]																																																													
	BOILER ROOM (E)	[BOILER ROOM (E) data for each iteration]																																																													
	total time of discomfort (level 1) (h/year)	[Total time of discomfort (level 1) data for each iteration]																																																													
	total time of discomfort (level 2) (h/year)	[Total time of discomfort (level 2) data for each iteration]																																																													
	percentage of discomfort (level 1) (%)	[Percentage of discomfort (level 1) data for each iteration]																																																													
percentage of discomfort (level 2) (%)	[Percentage of discomfort (level 2) data for each iteration]																																																														

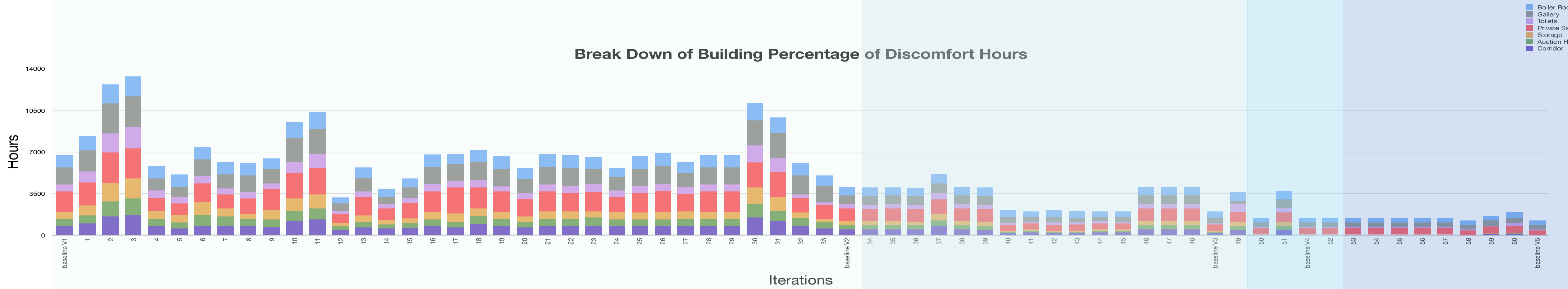


BASELINE V1

The baseline has already some good parameters implemented. However, its energy consumption are way to high and discomfort hours too large too.

updated BASELINE V2

Mainly the discomfort hours have gone down in that iteration, from 41% to 25%, according to ASHRAE standard 55-2007, with a 90% acceptability limit. This is achieved just using passive measures. There is also a small decrease in heating loads, which could be due to the mixed ventilation.

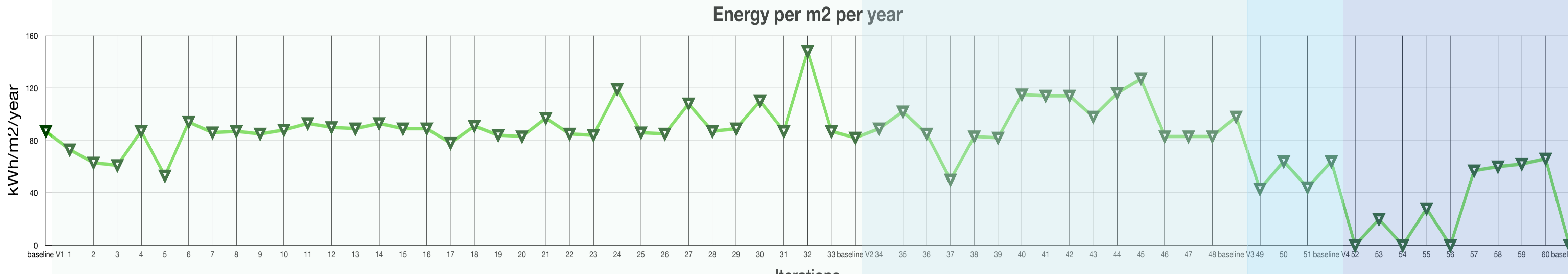


updated BASELINE V3

The added cooling significantly decreases the discomfort hours (from 25 to 12%), and the HVAC system significantly decreased the heating loads by 74%. The lighting and equipment stays the same in terms of loads. It is still not enough to reach net carbon zero or the desired occupant comfort range.

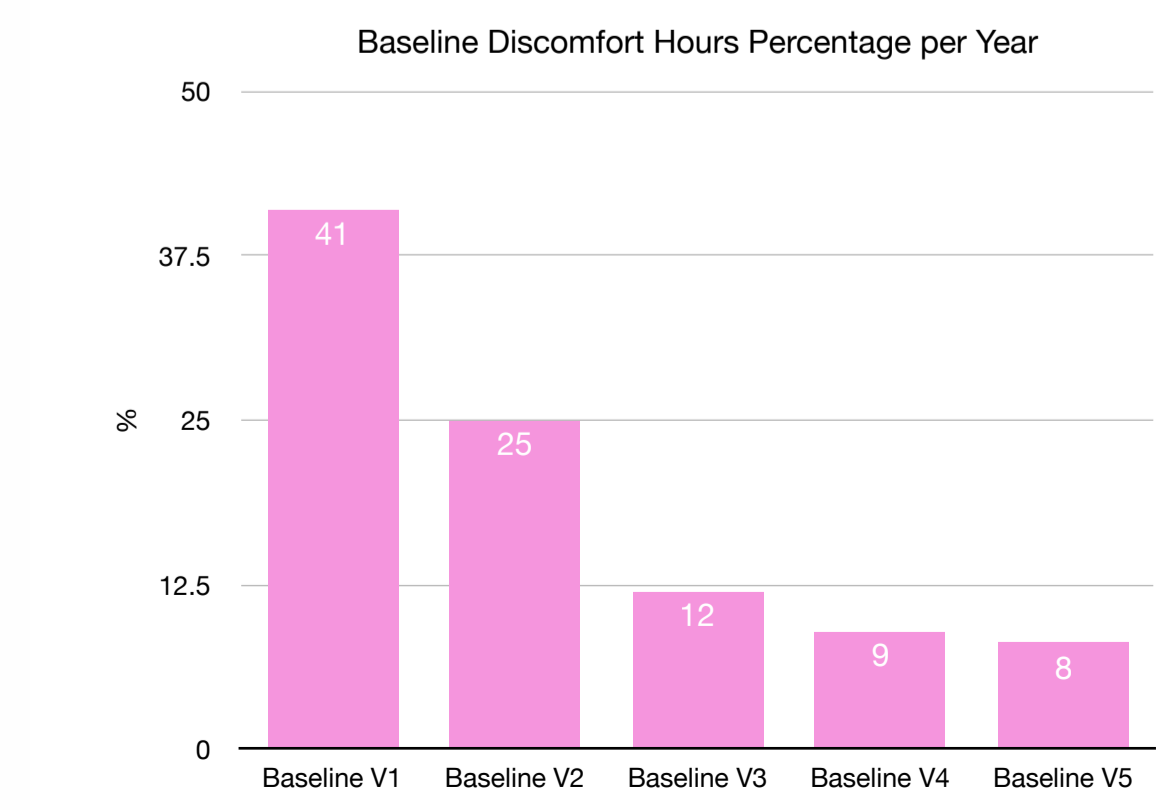
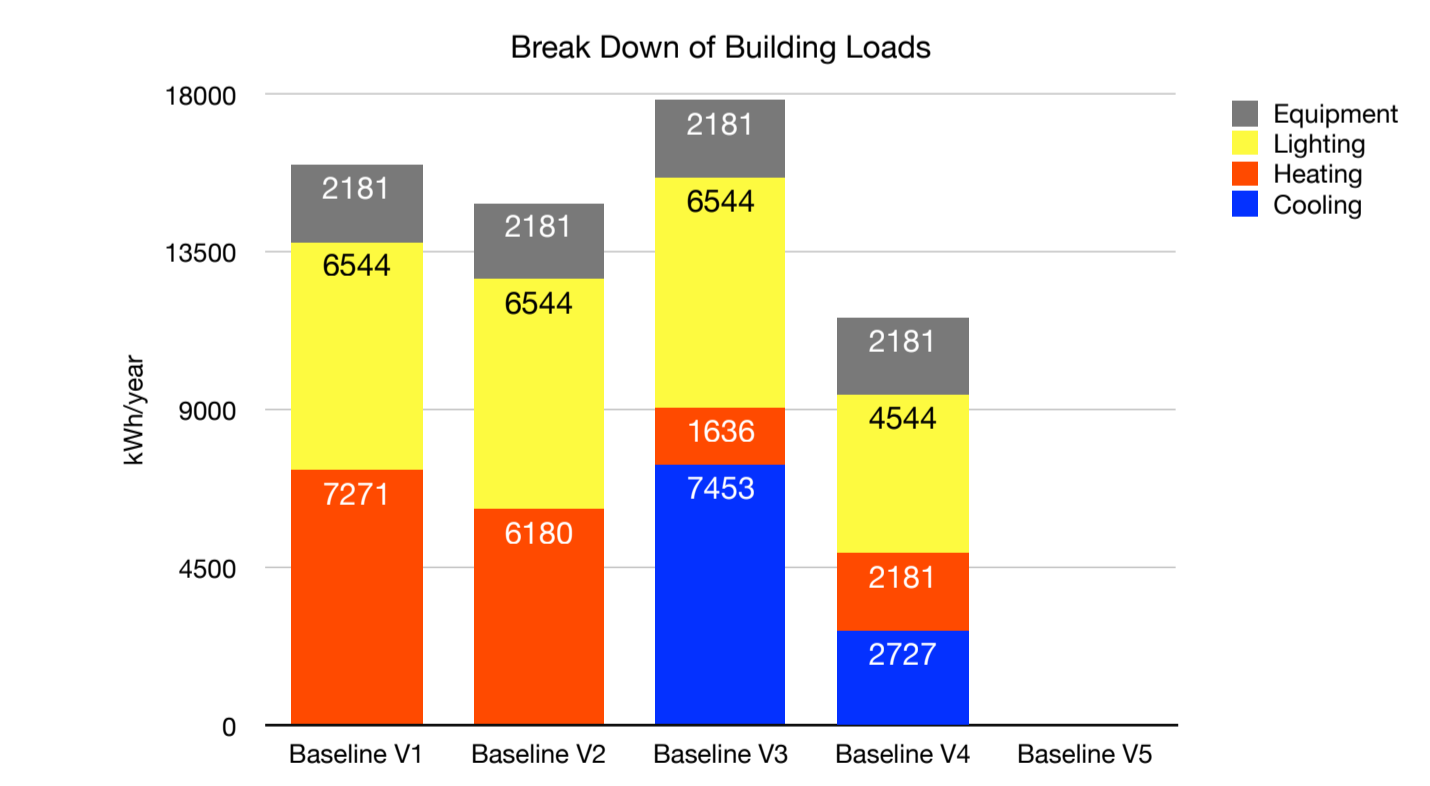
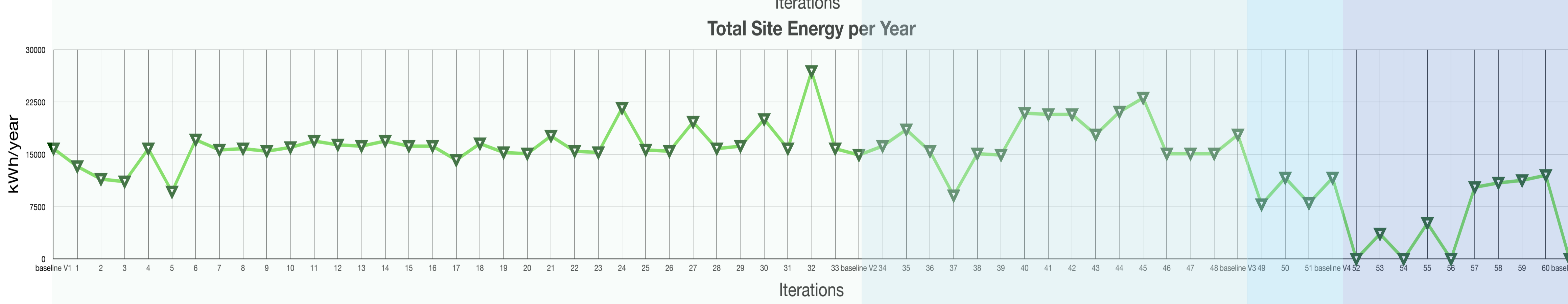
updated BASELINE V4

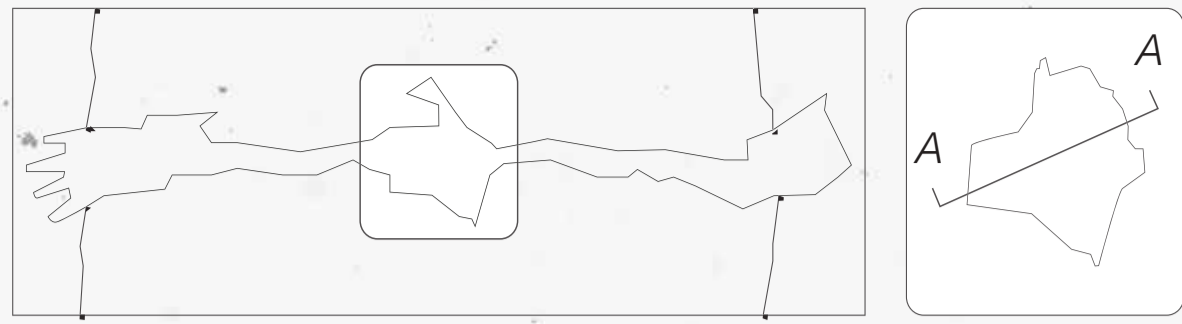
There is a compromise made where the heating load increases in the GSHP by 33% but the cooling load is reduced by 63%. The 9092kWh/year CIBSE benchmark is still not reached (having only achieved 11 633 kWh/year), but the discomfort hours are reduced to 9% which is good for a space with a programme that varies so much (from very occupied auction house to private sale).



updated and final BASELINE V5

The renewables implemented, mainly solar power panels and dams, reduce the energy usage to net zero carbon. It is the desired goal, and even though the discomfort hours are still 8%, it is good enough for an auction house where people do not live.





- 1- GSHP
- 2- electrical grid
- 3- water sewage
- 4- toilets
- 5- underfloor heating
- 6- solar panels
- 7- VAV
- 8- CO2 sensors
- 9- lights
- 10- switches
- 11- electrical sockets

1:50 on A1

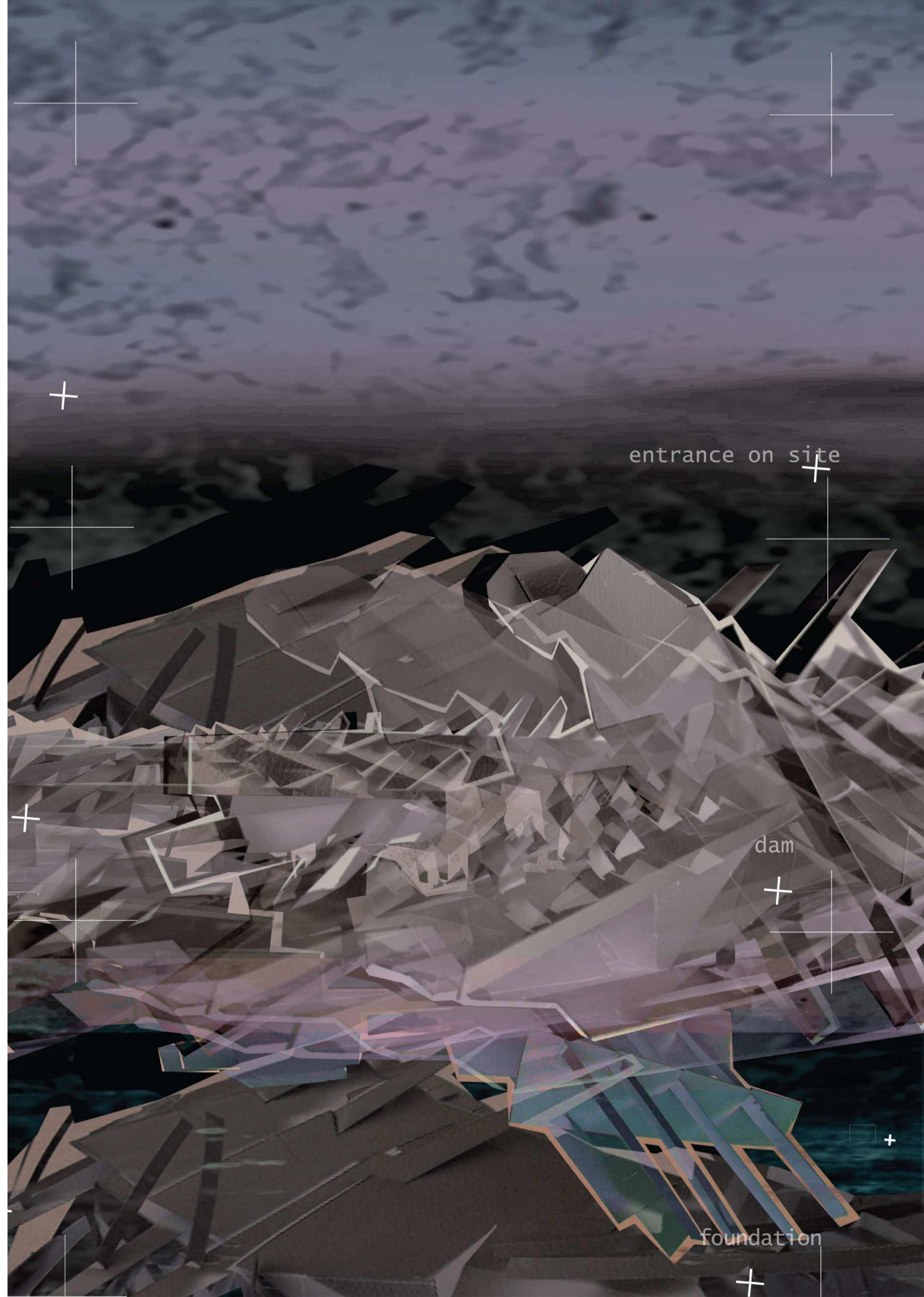
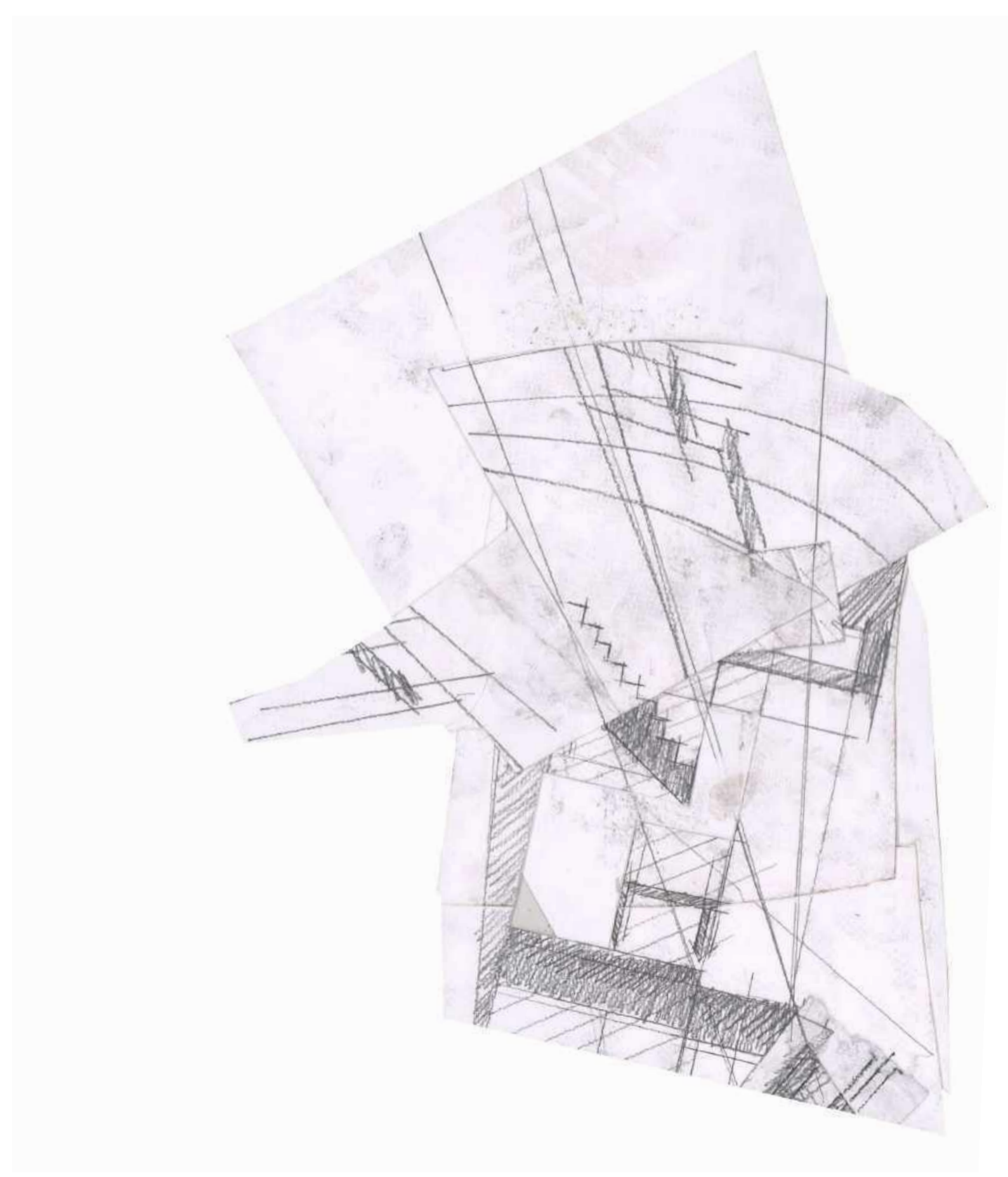


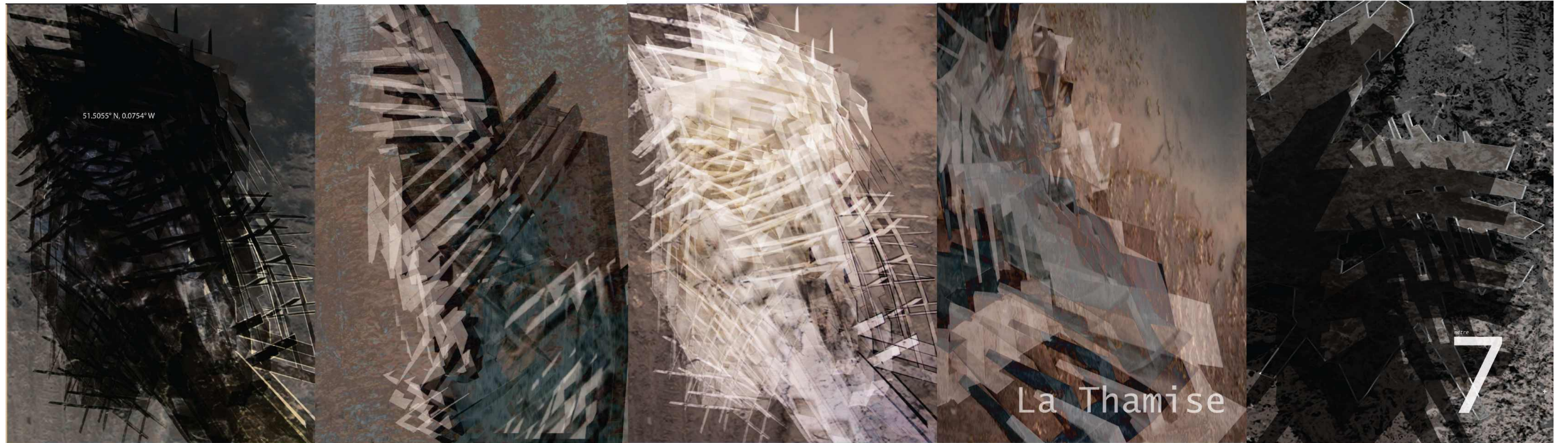


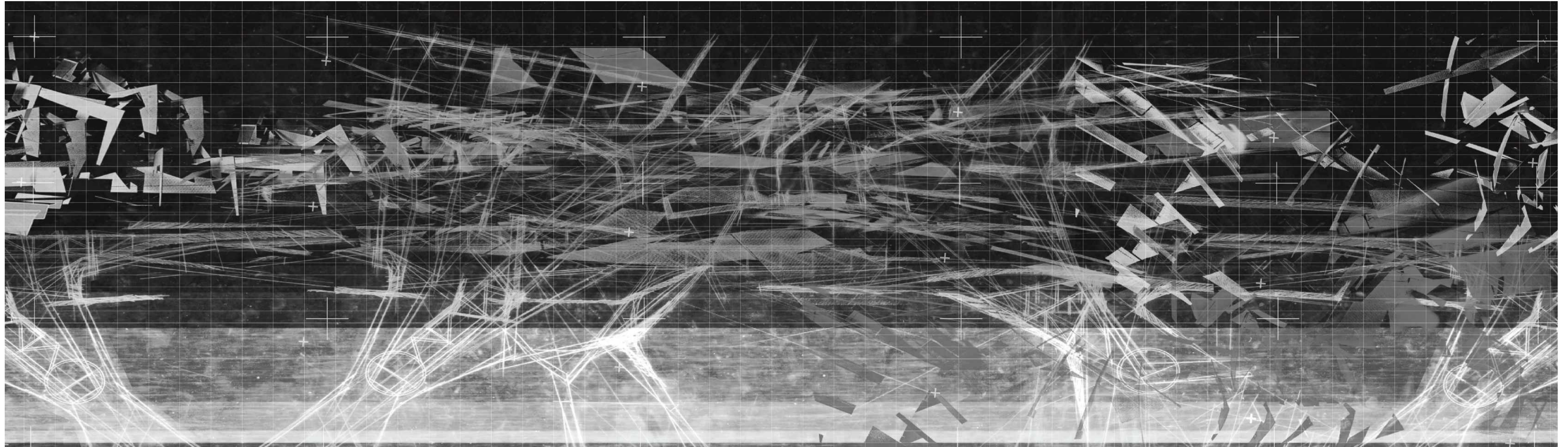


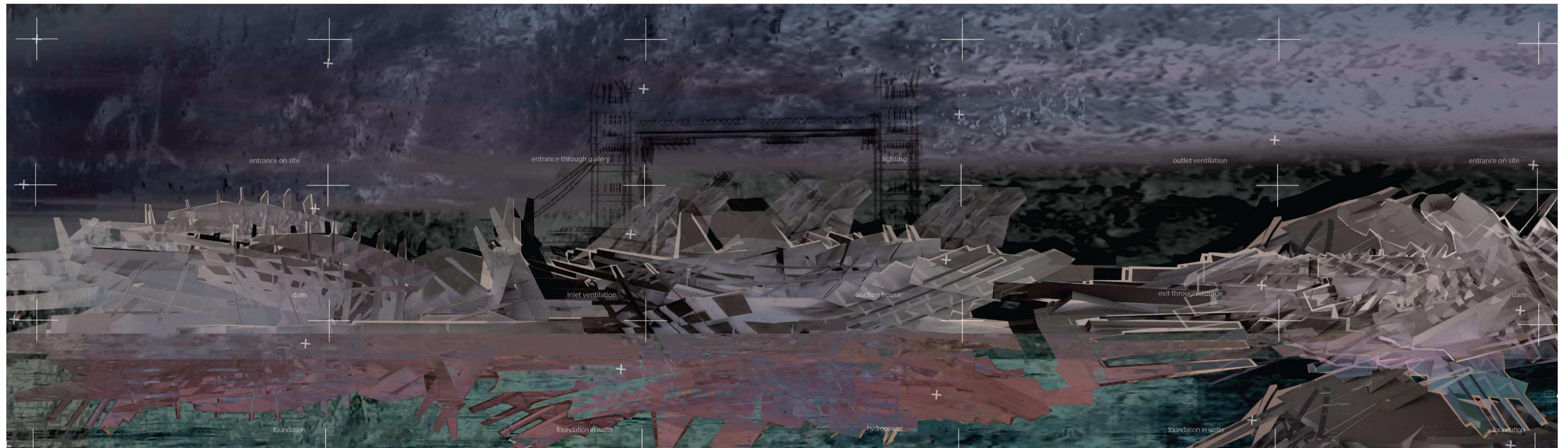
Section 15. Final Drawings: Venice

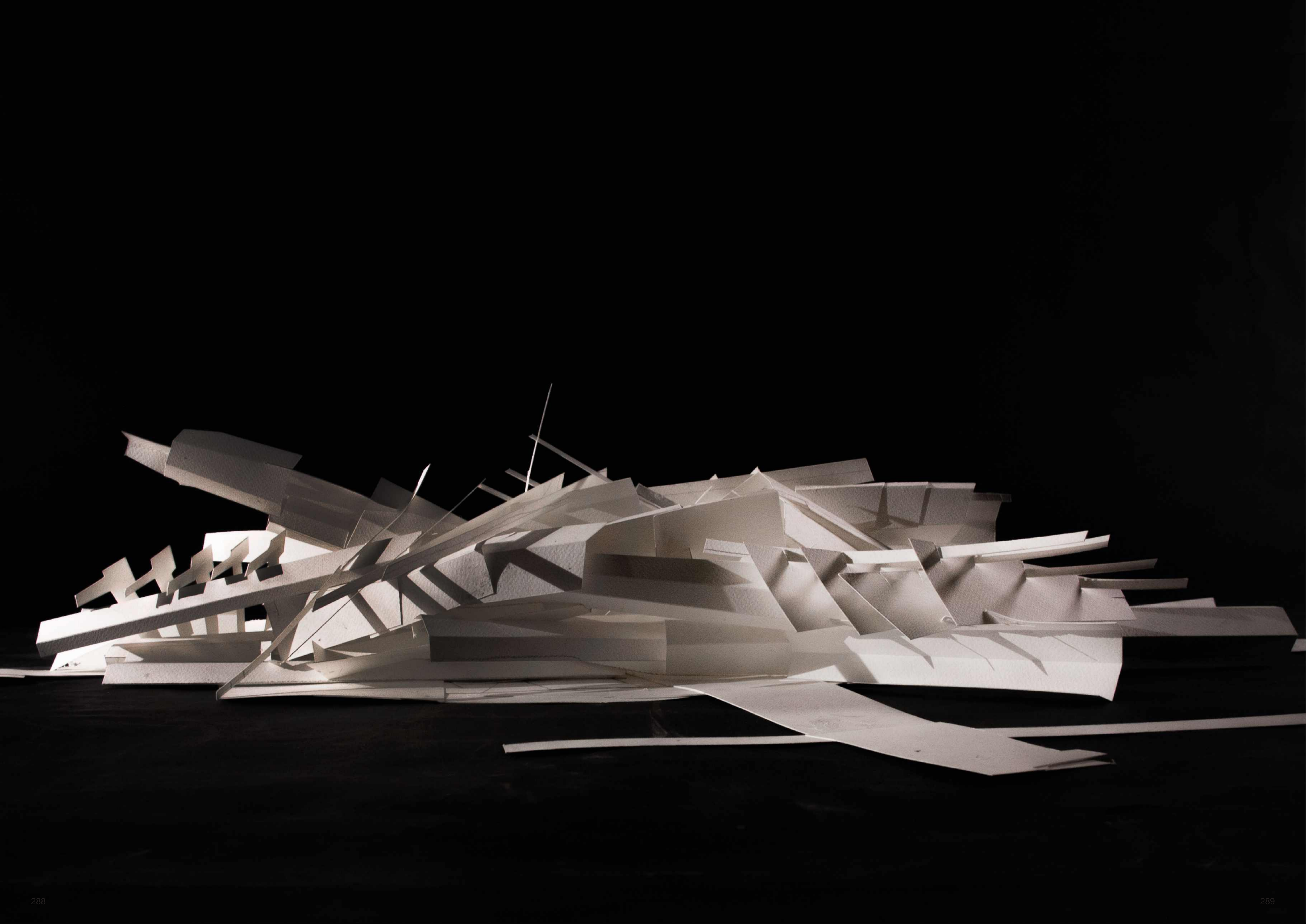
The outcome of the project, is like clay, malleable. Malleable in the sense that it is not completely finished, yet accomplished in its own right. The drawings vary from being concrete to abstract, and inbetween the collages-that sit somewhere inbetween art, illustrations and architecture, is my vision of my future practice as a creator. The renders took form of ceramic glazed fragments, that lie on tables, floors and rivers just like the Thames artefacts: lost, forgotten and scavanged. The building is one that can last or be destroyed, one that exists and does not at the same time. If one agrees to see it, it will appear, like a sinking ship in St Saviour's dock, with its rusted beams, muddy stairs and algae wettened rails.













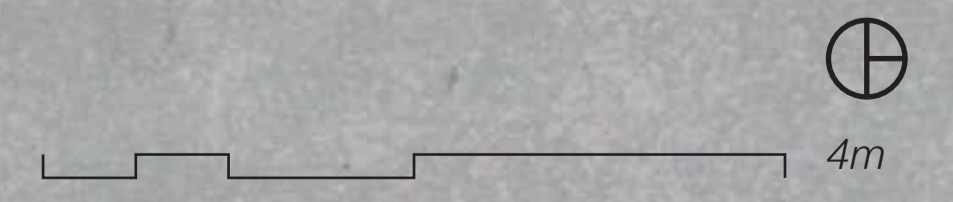


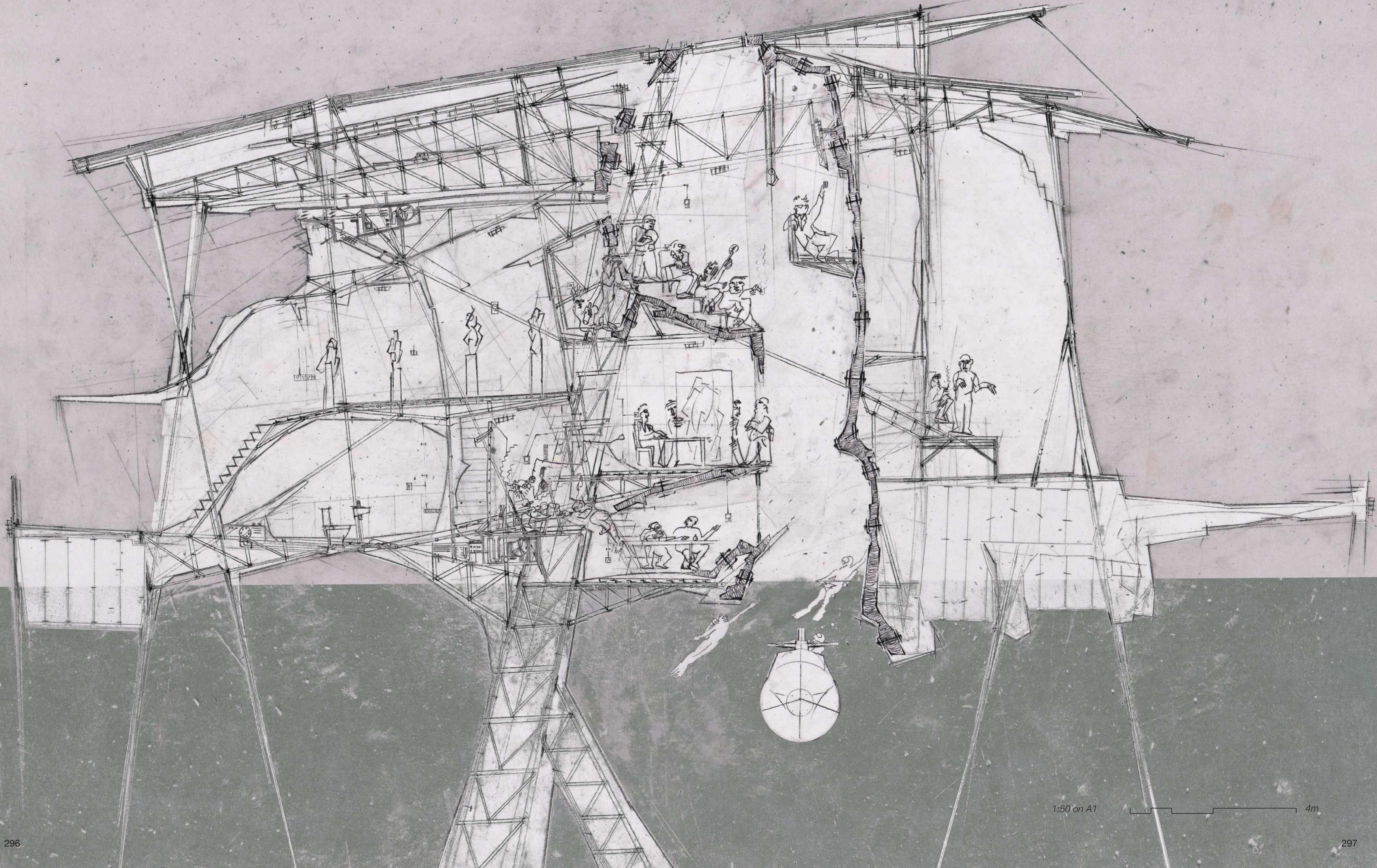
Floor Plan of center of bridge 1:50



- 1-auction space
- 2-peeping room
- 3-private sale room
- 4-gallery
- 5-main entrance
- 6-disabled entrance
- 7-toilets
- 8-back door entrance
- 9-cargo entrance
- 10-artists entrance
- 11-auctioneer's stand
- 12-trap door

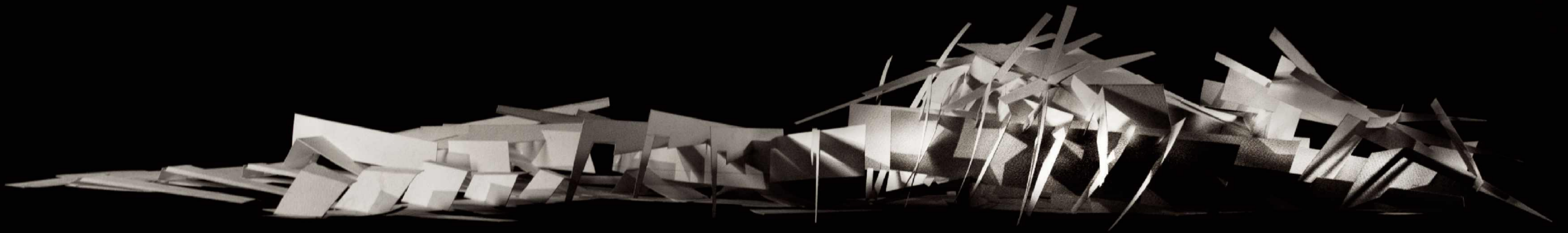
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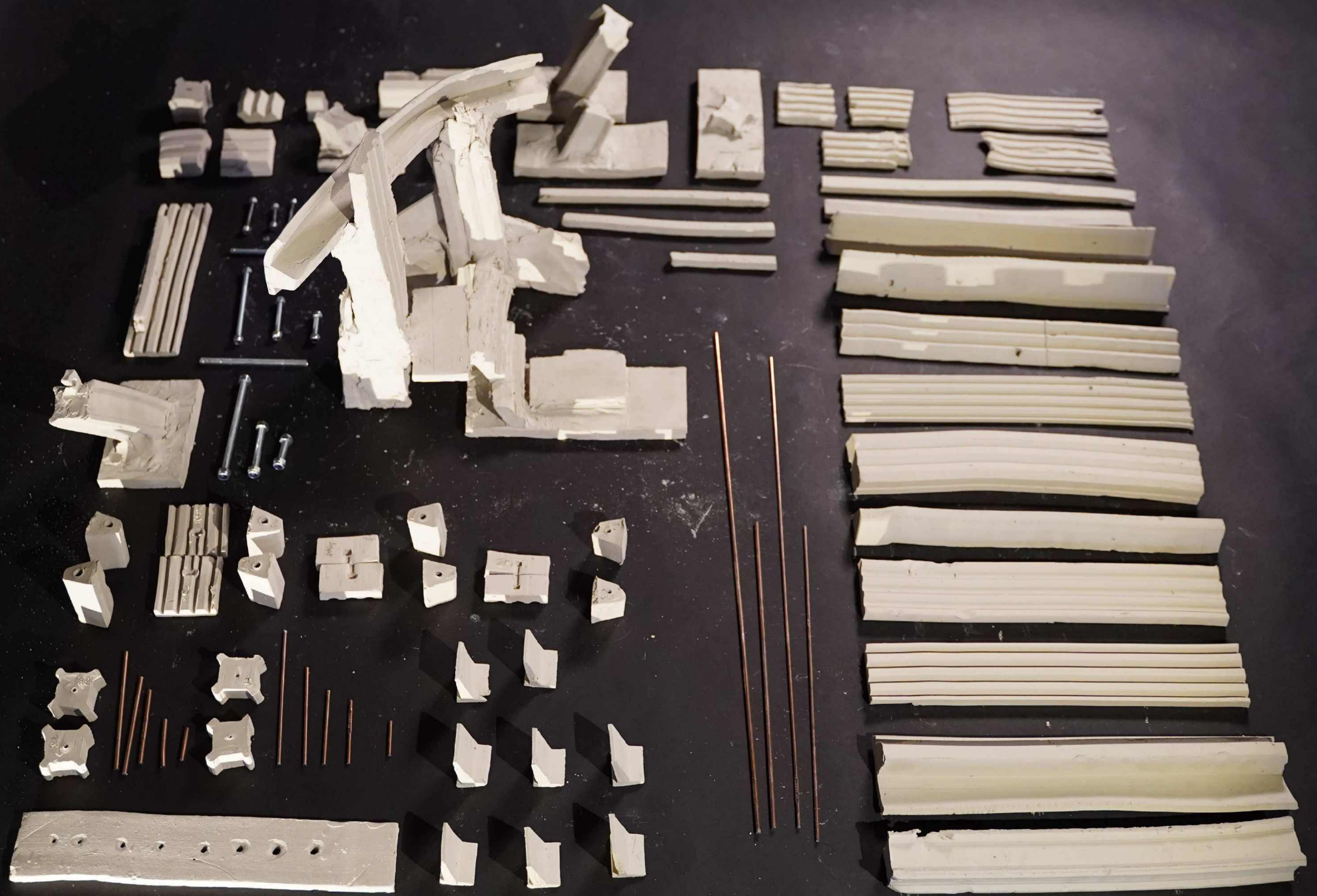




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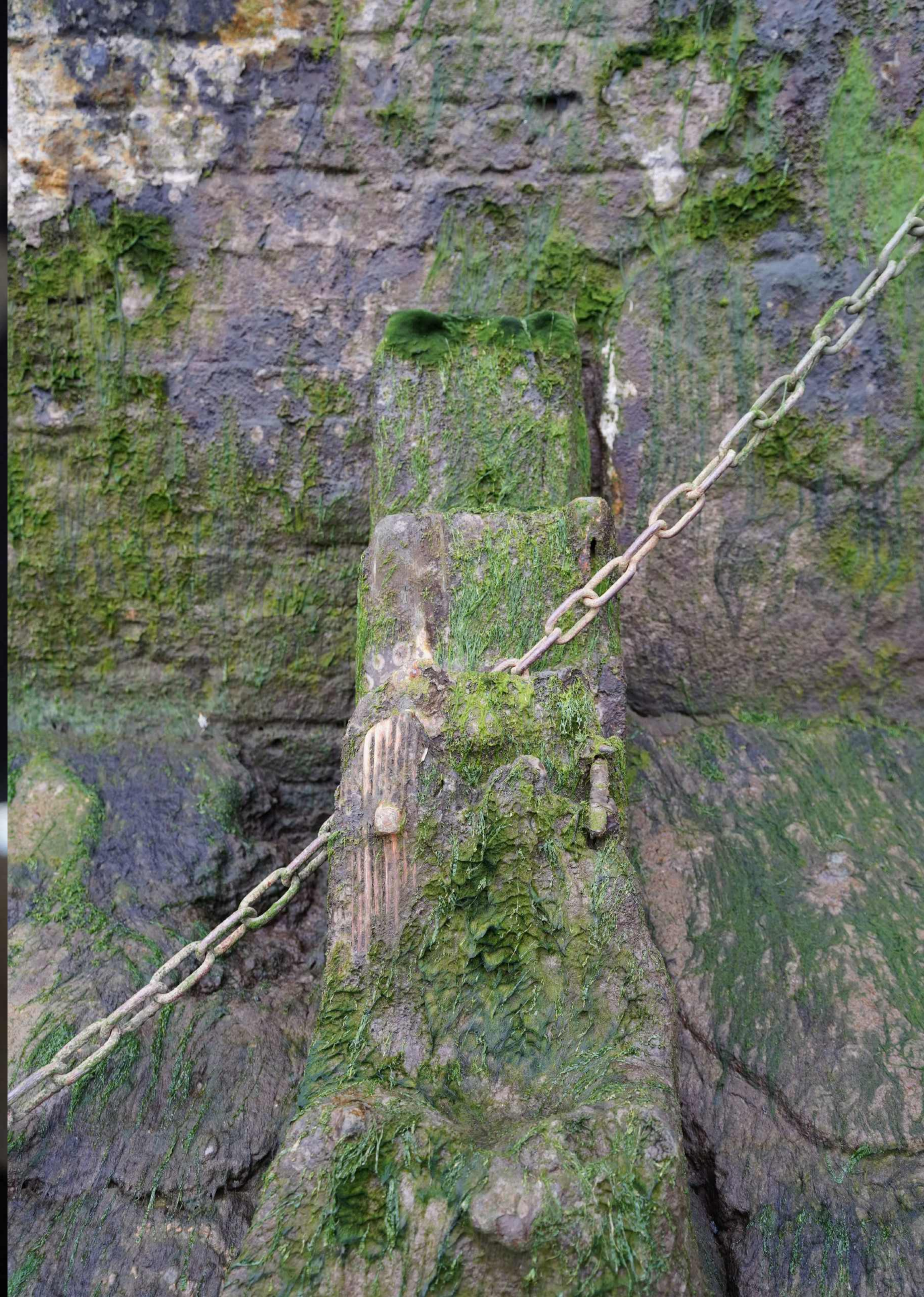
4m



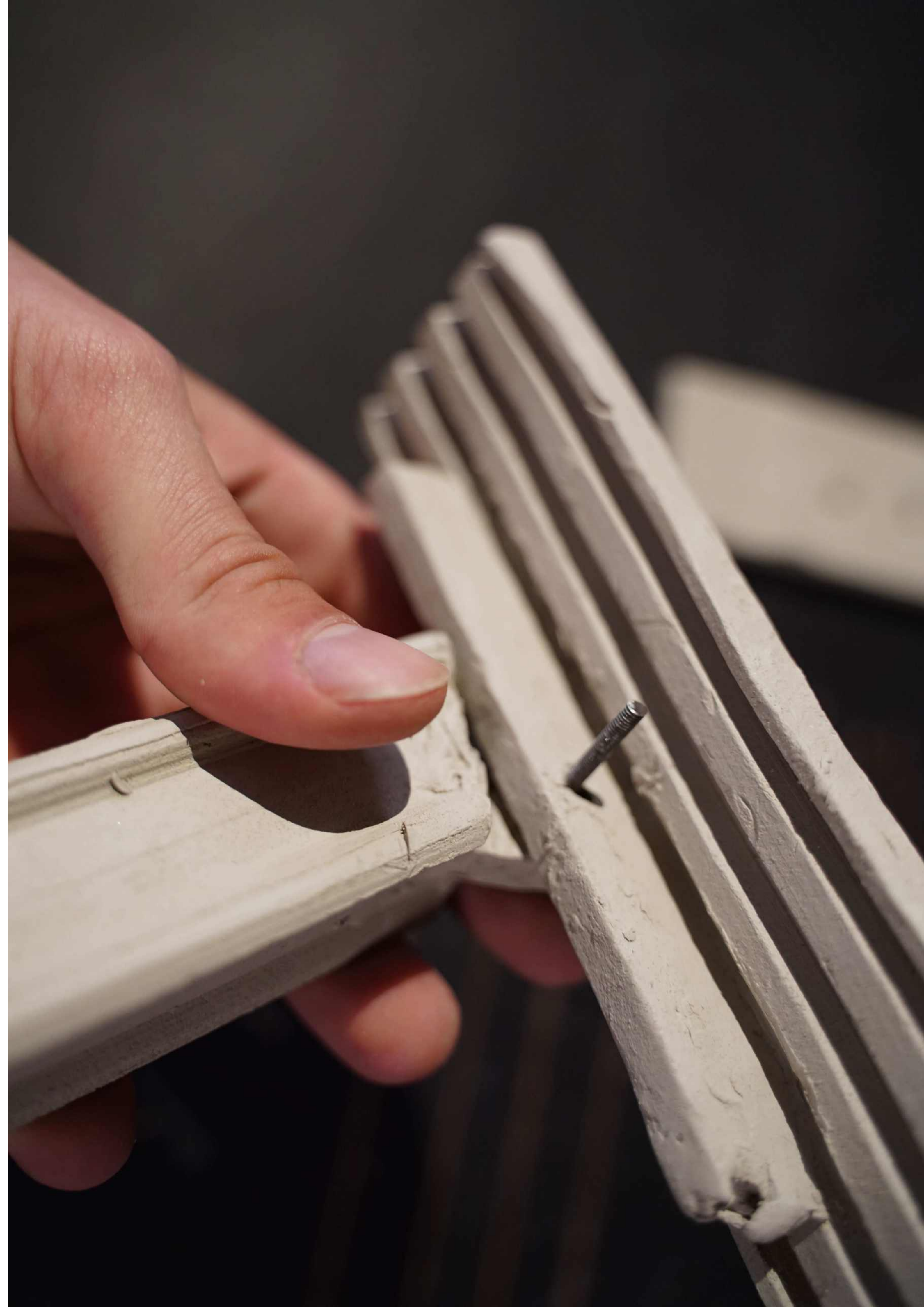




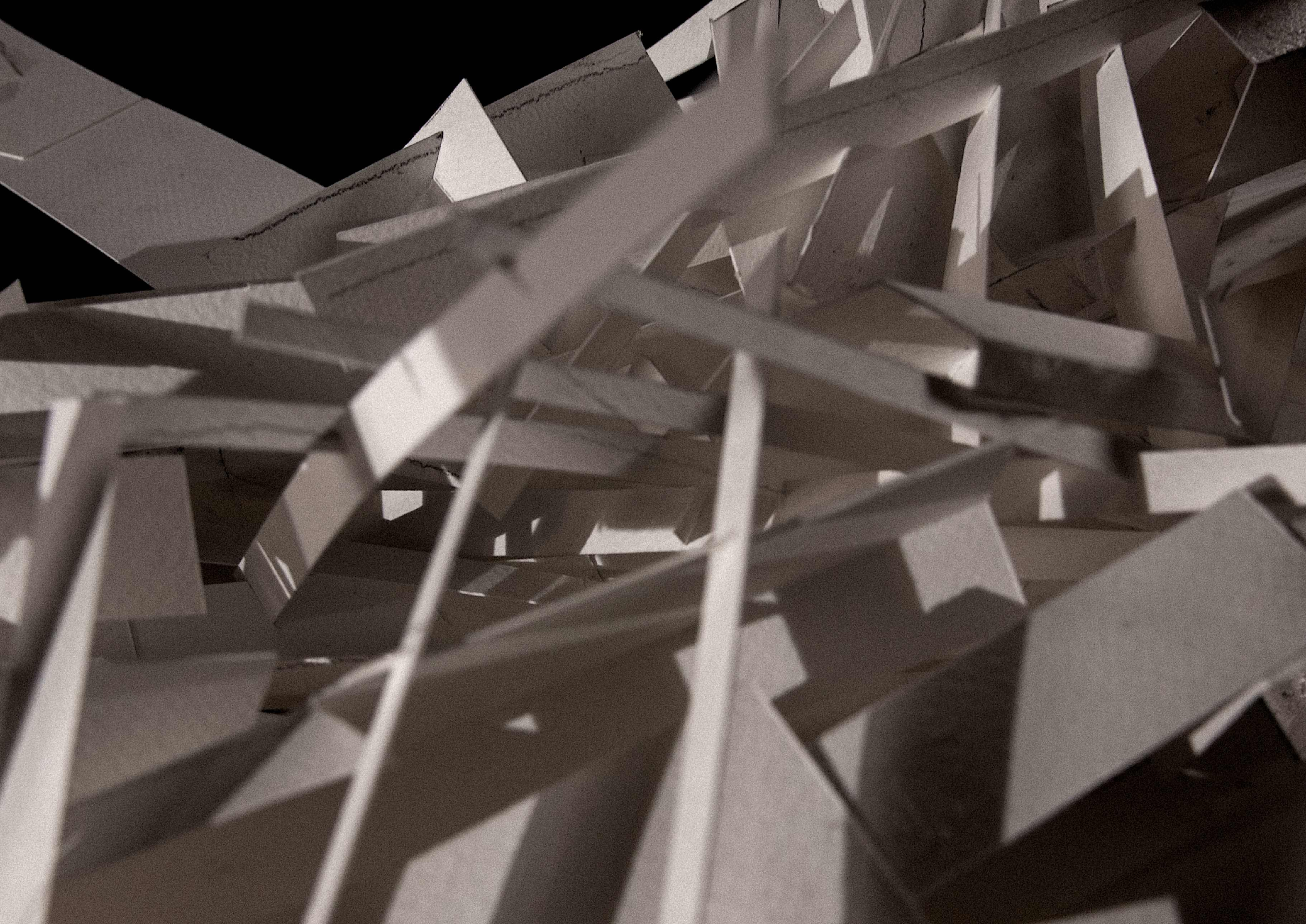












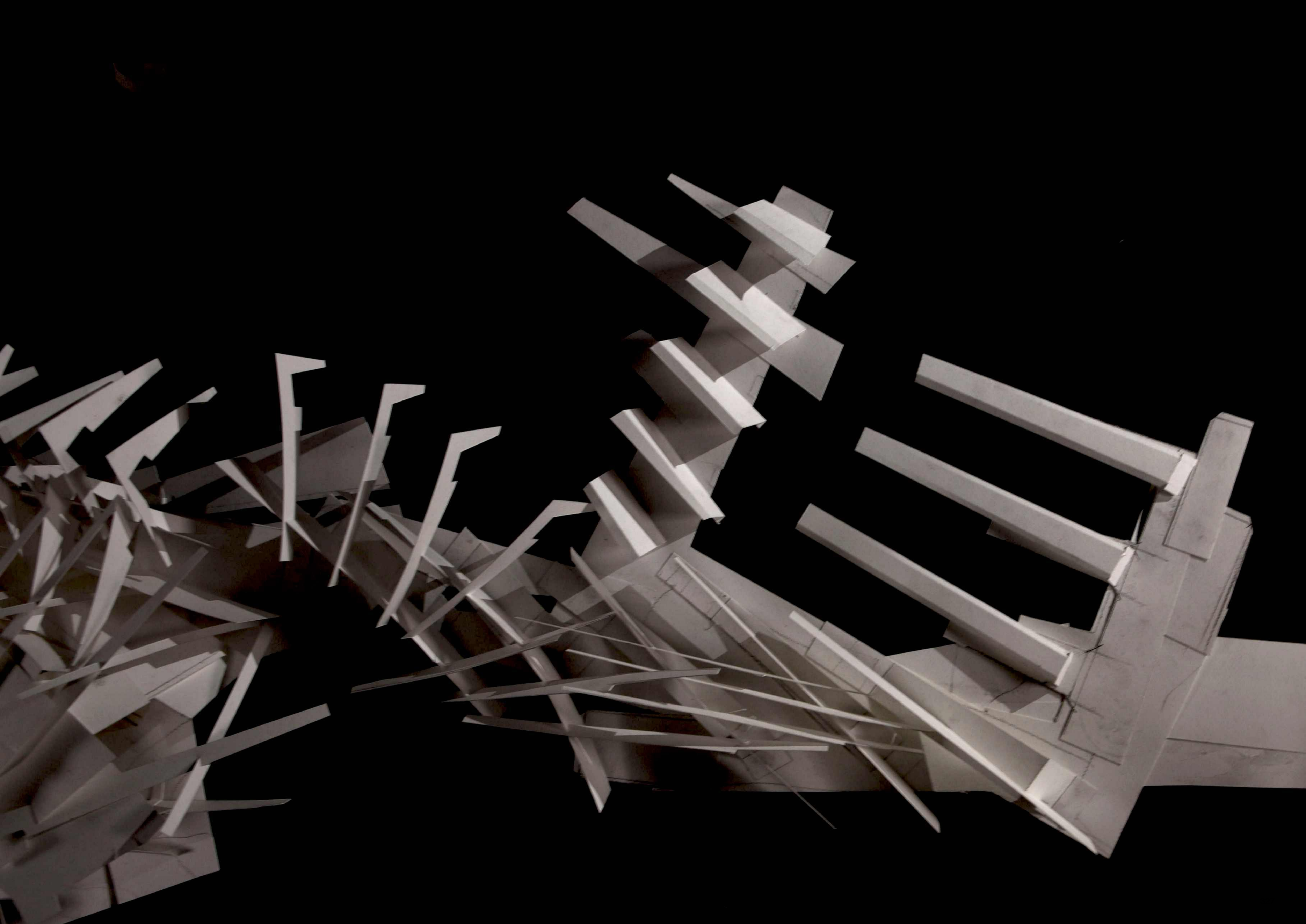






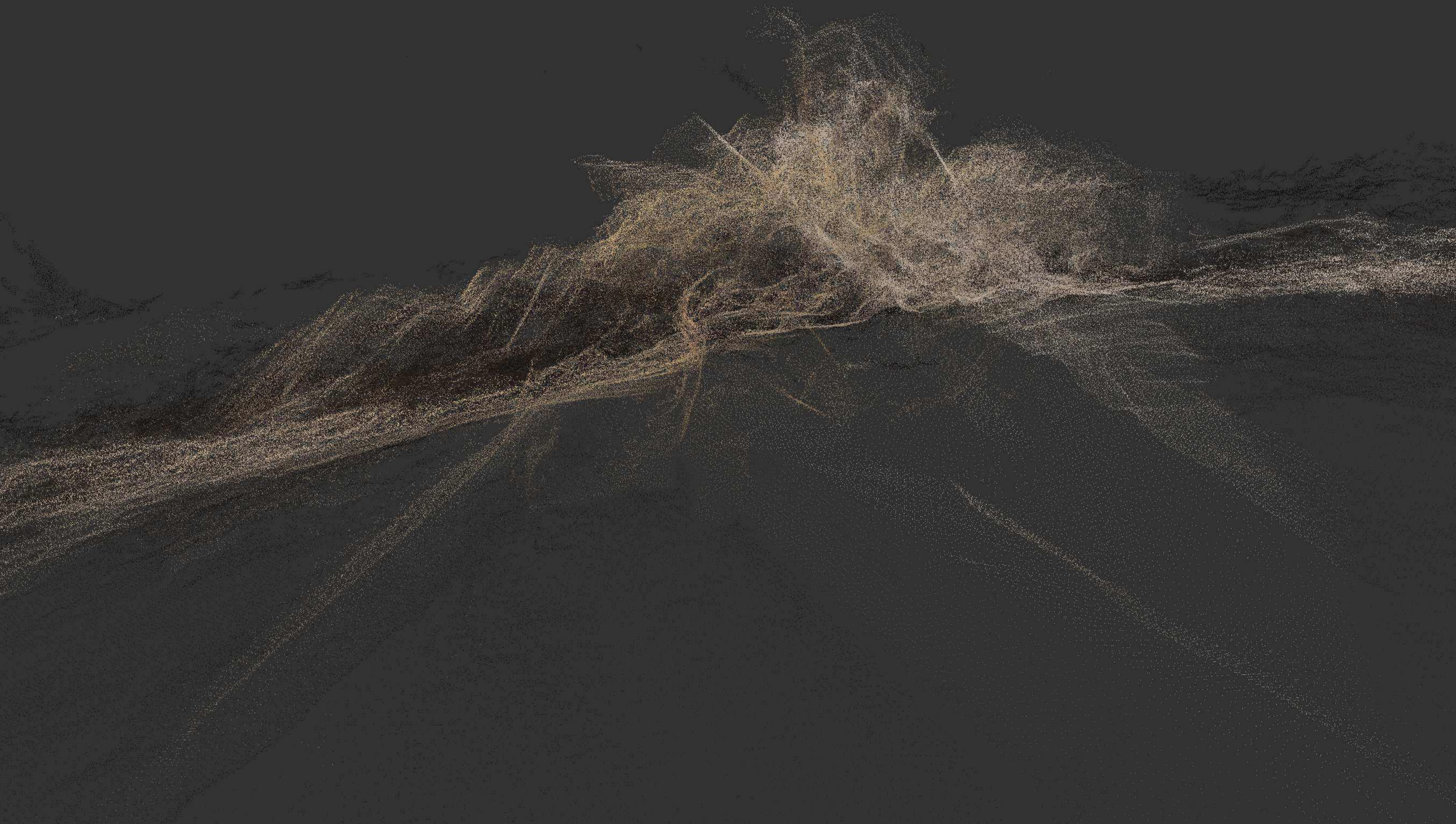


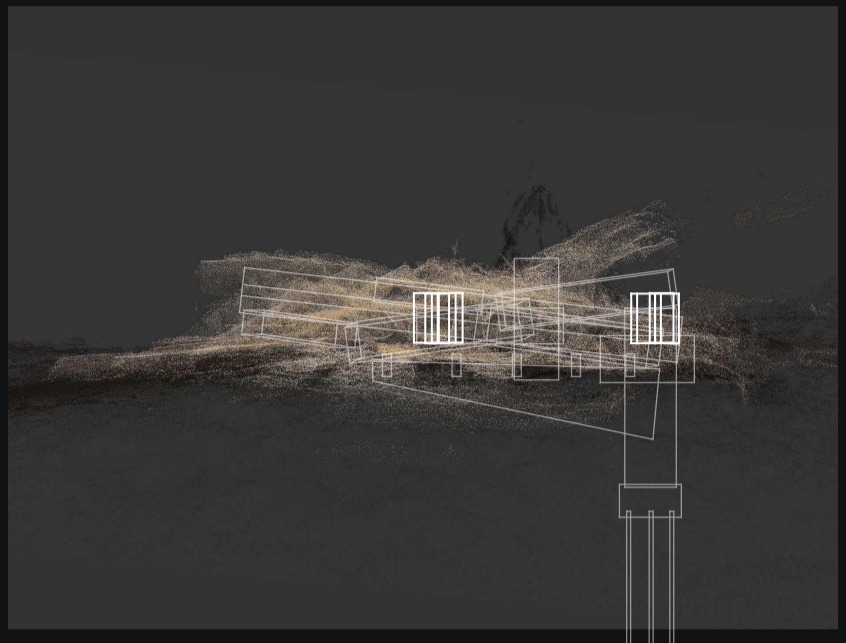
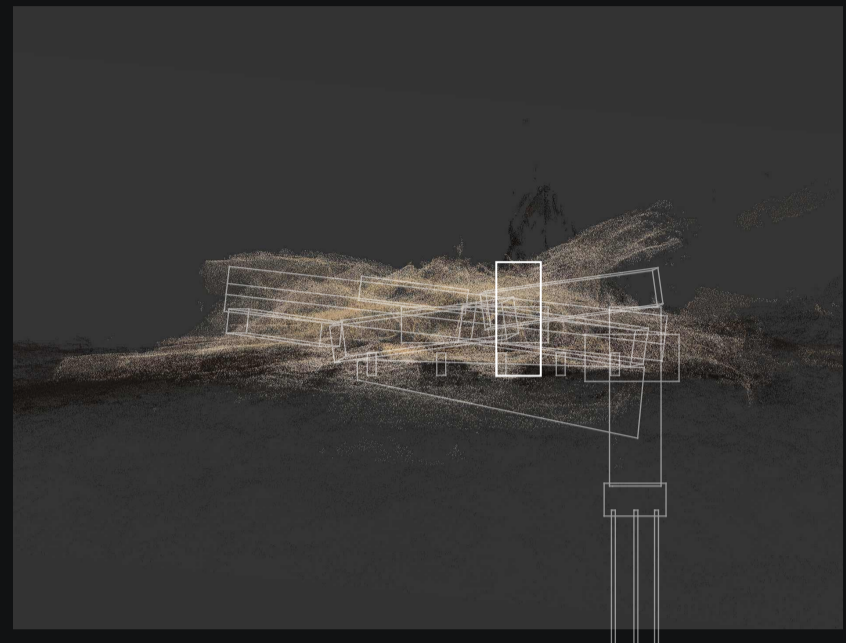
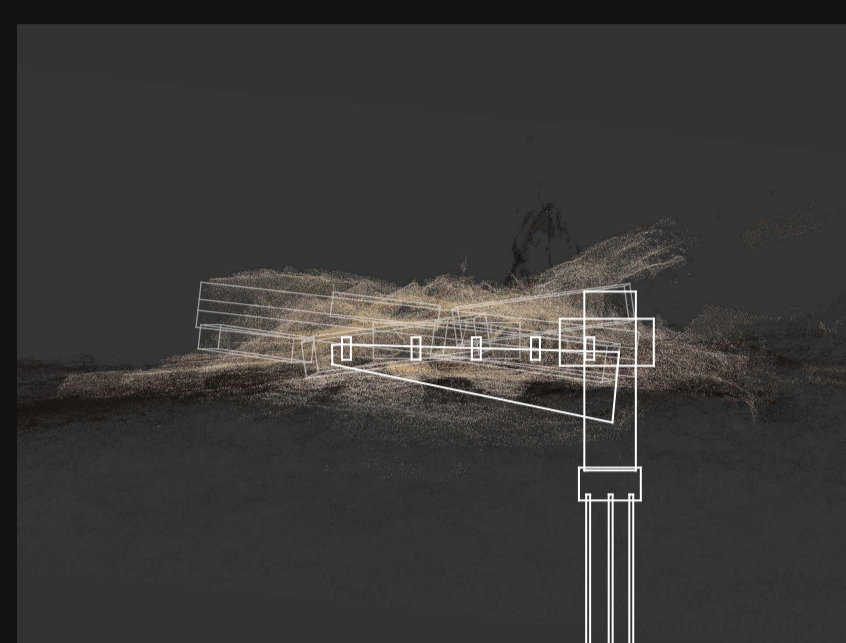
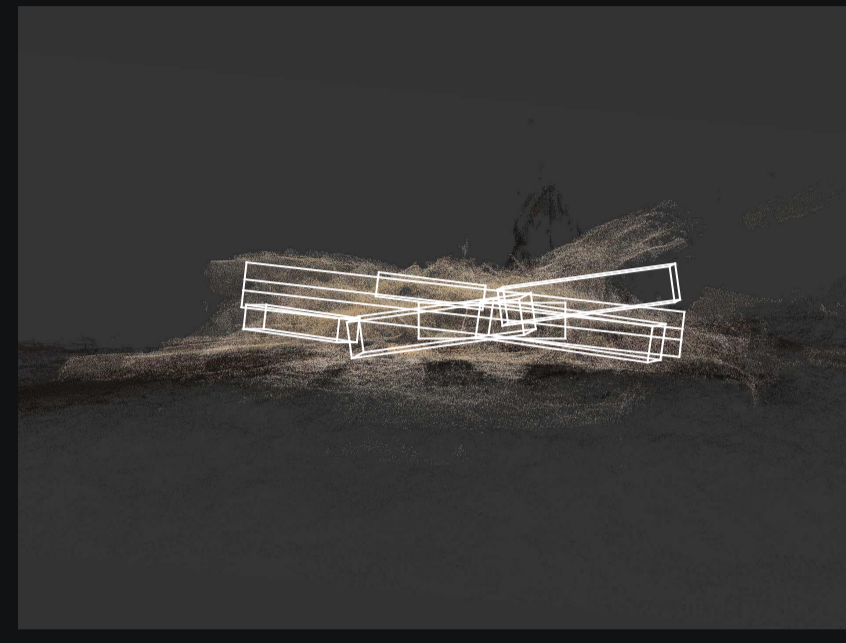
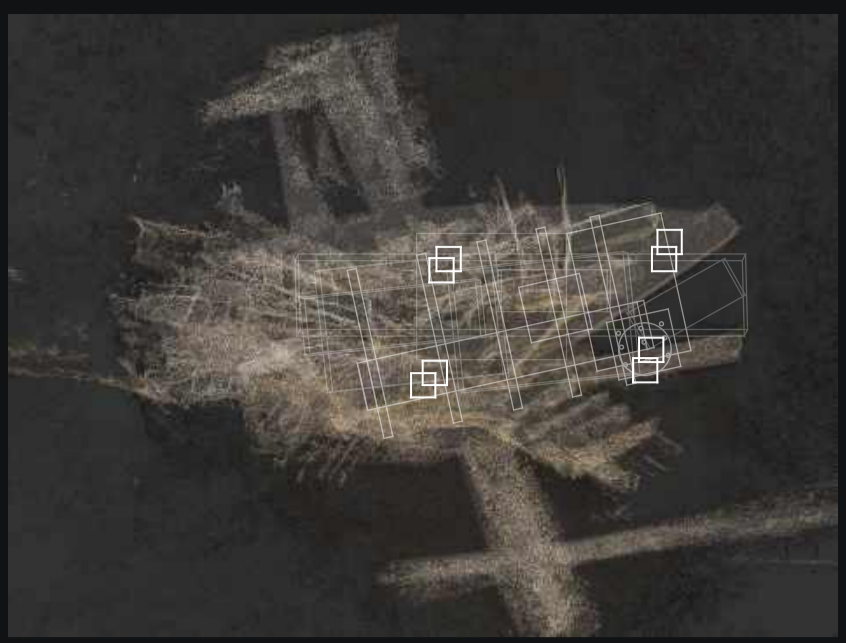
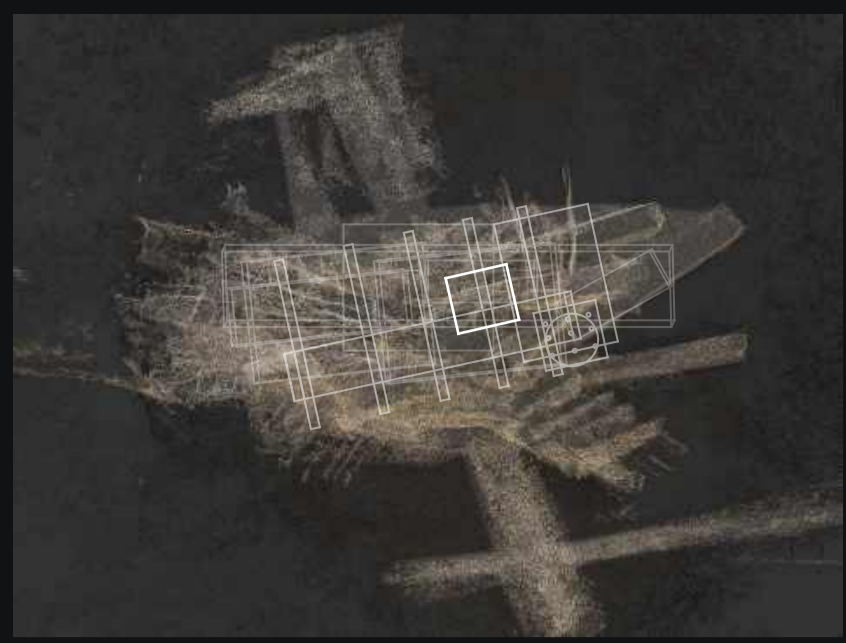
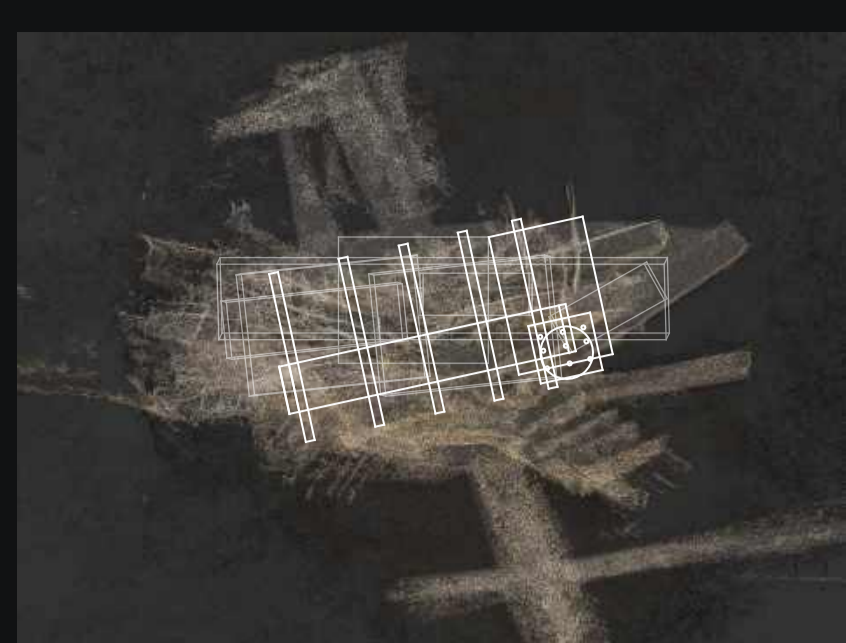
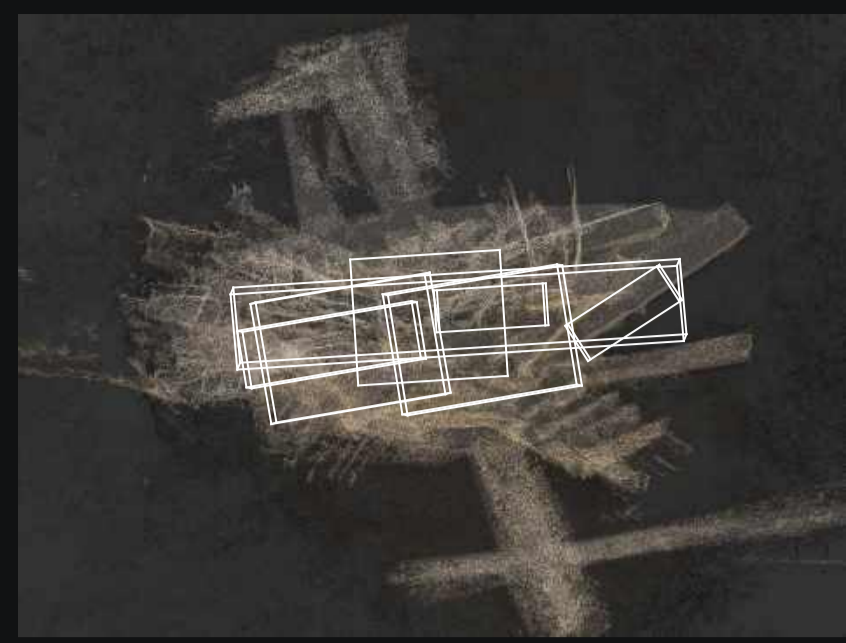






Appendix



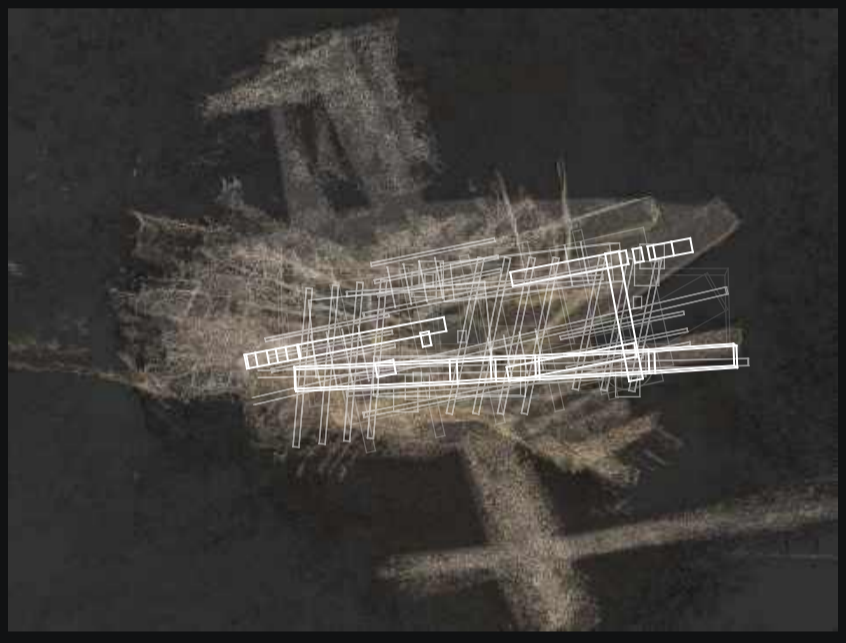
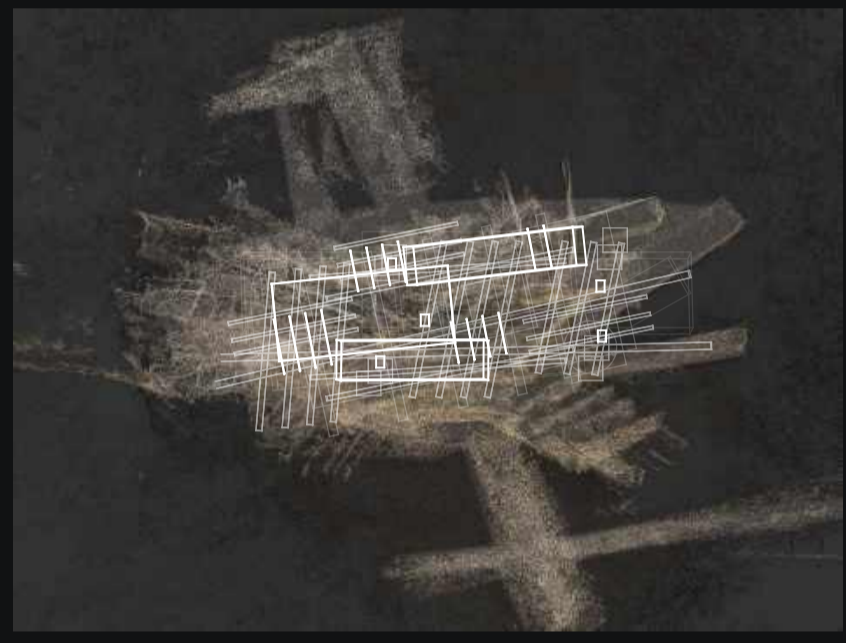
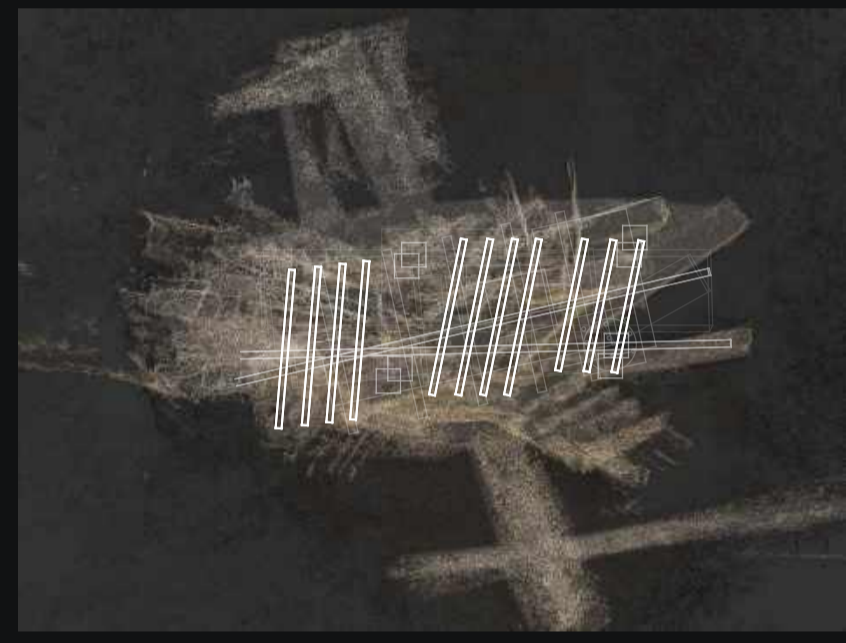
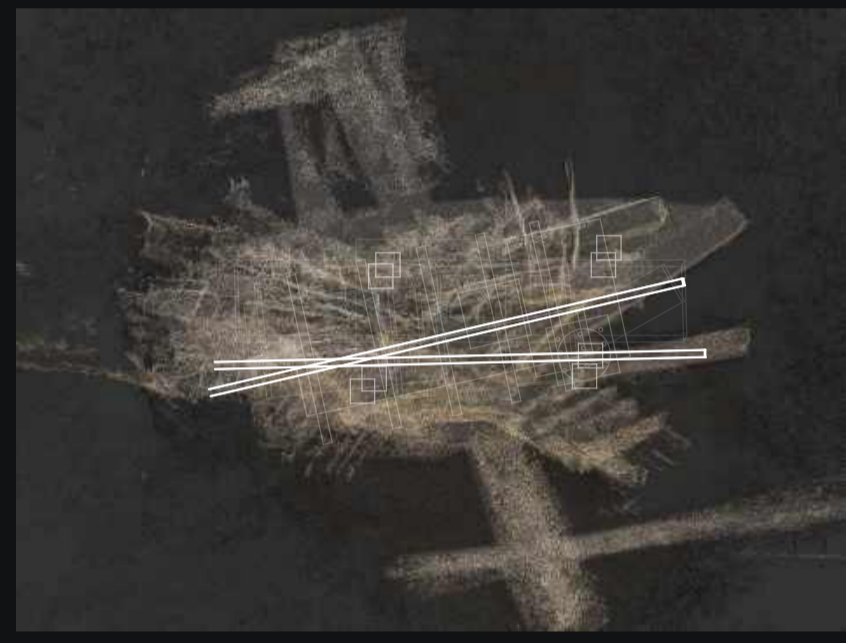


massing

foundations

services

primary columns



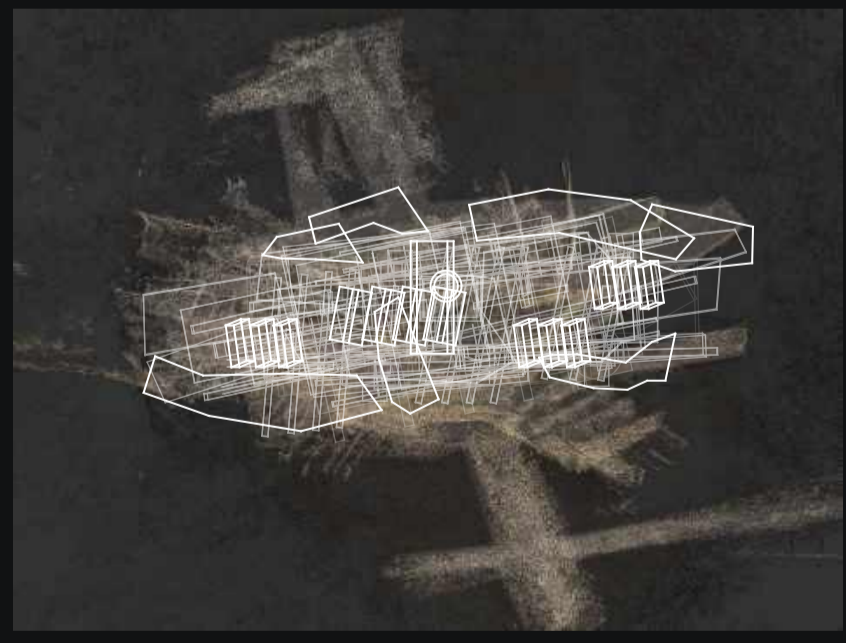
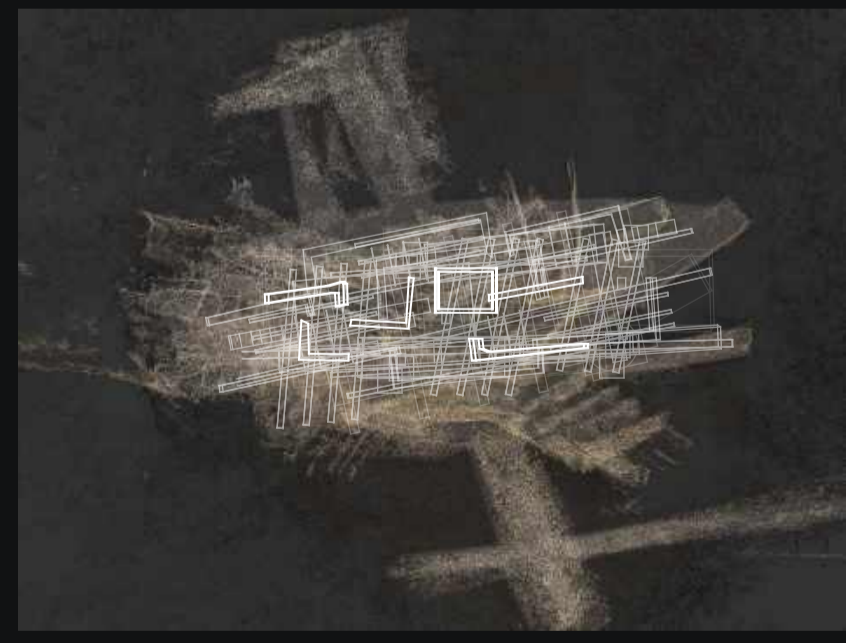
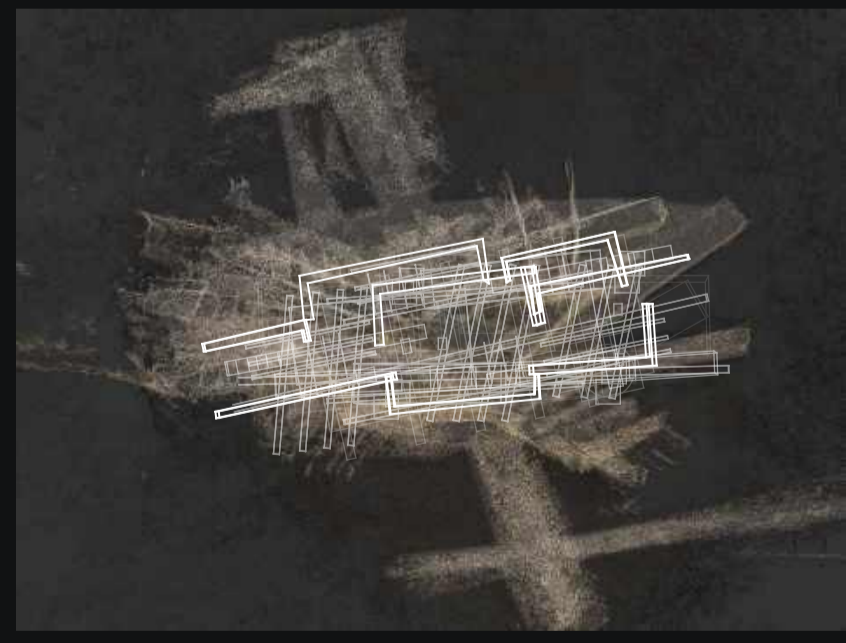
primary beams

secondary beams

third degree beams and bracing

joints, cables and ramps

natural ventilation and HVAC



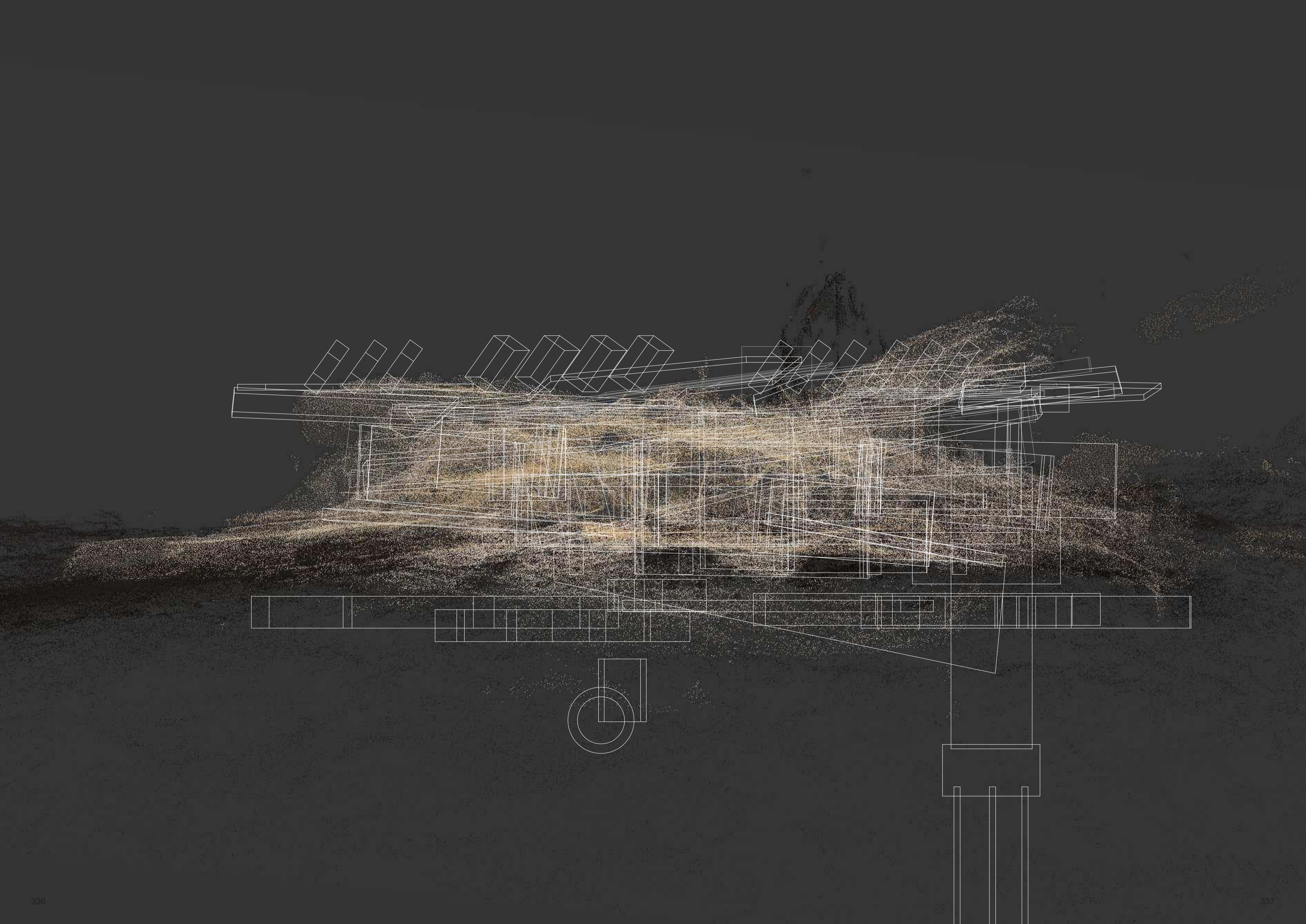
load supporting walls

internal partitions

roof and floors

louvres, floaters and hydropower

1:200 on A1



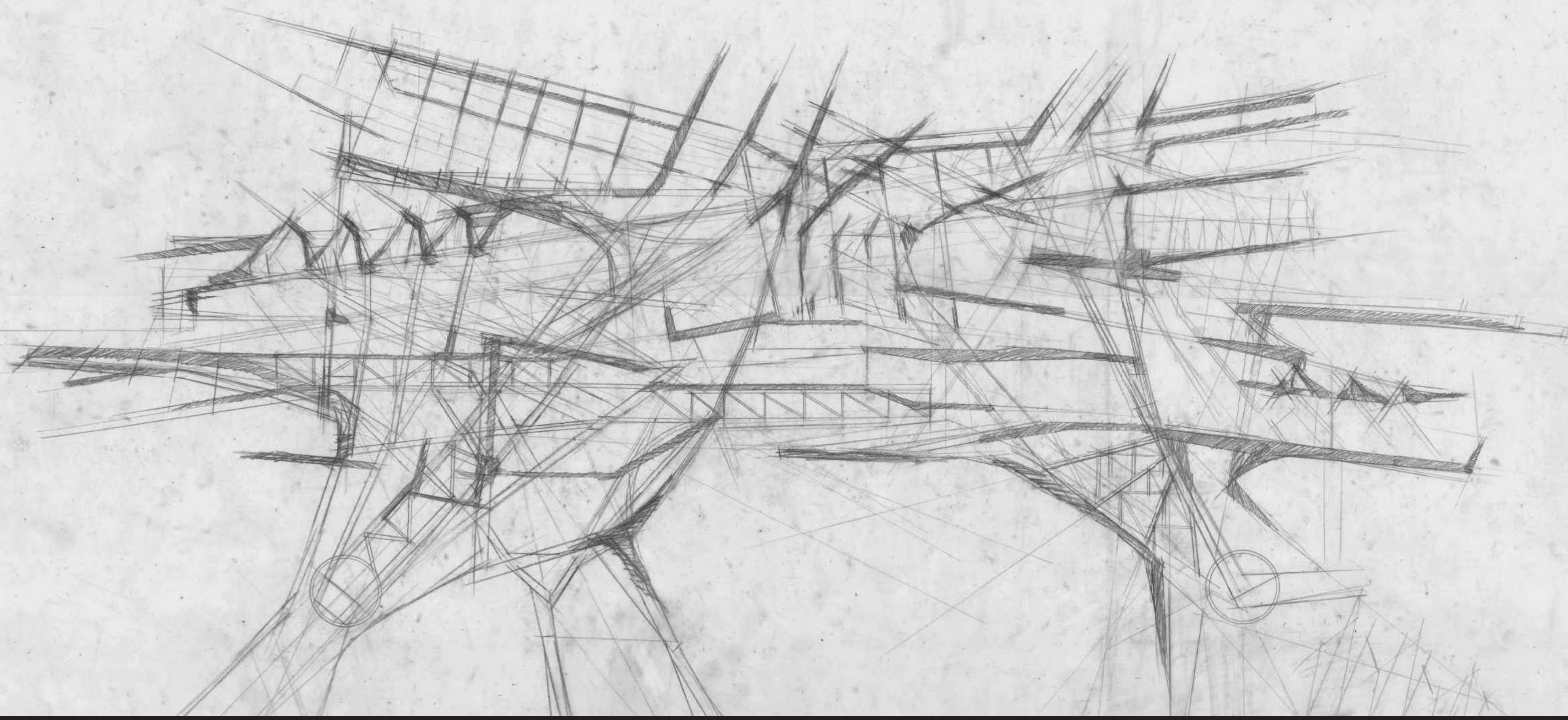


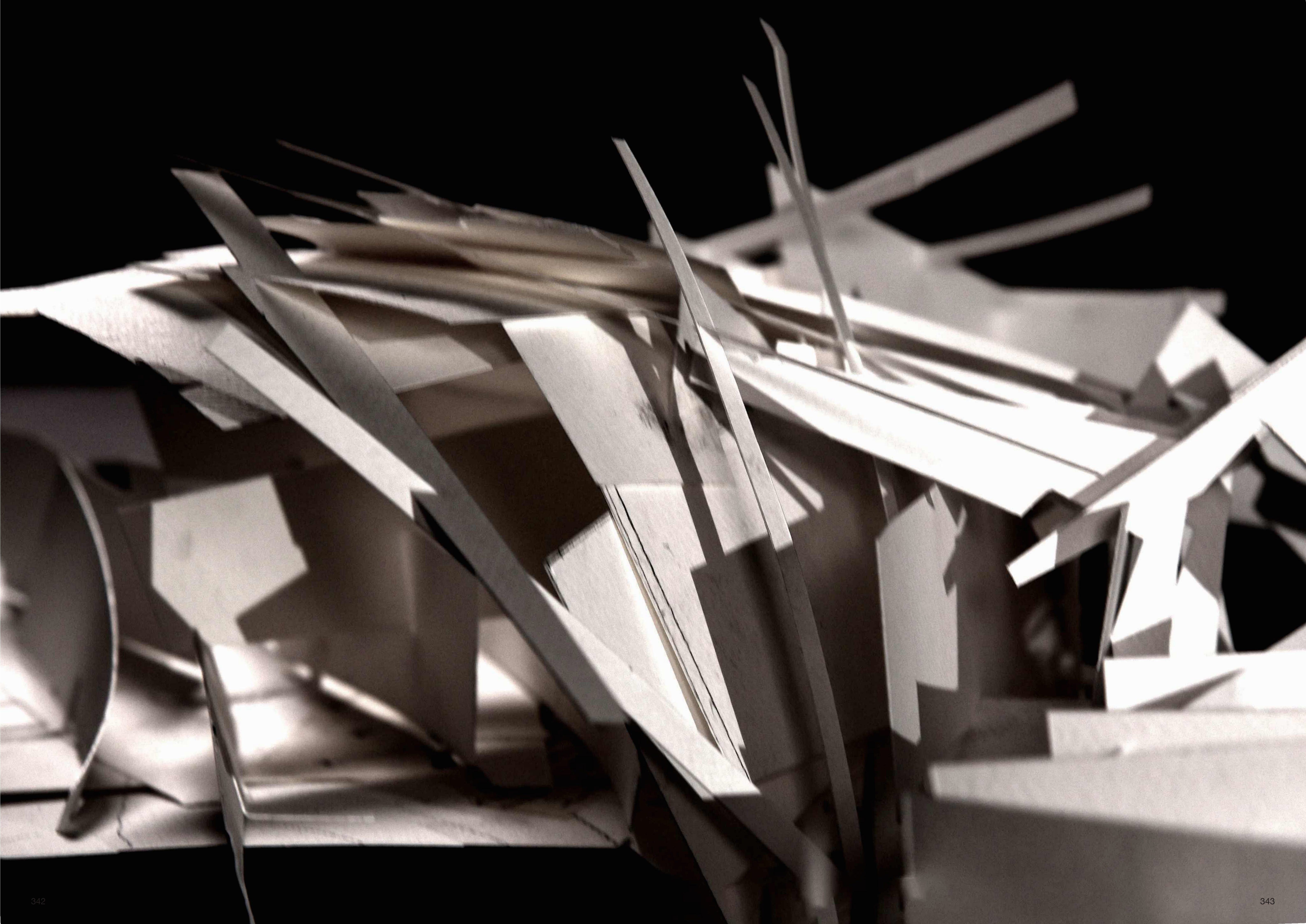
section

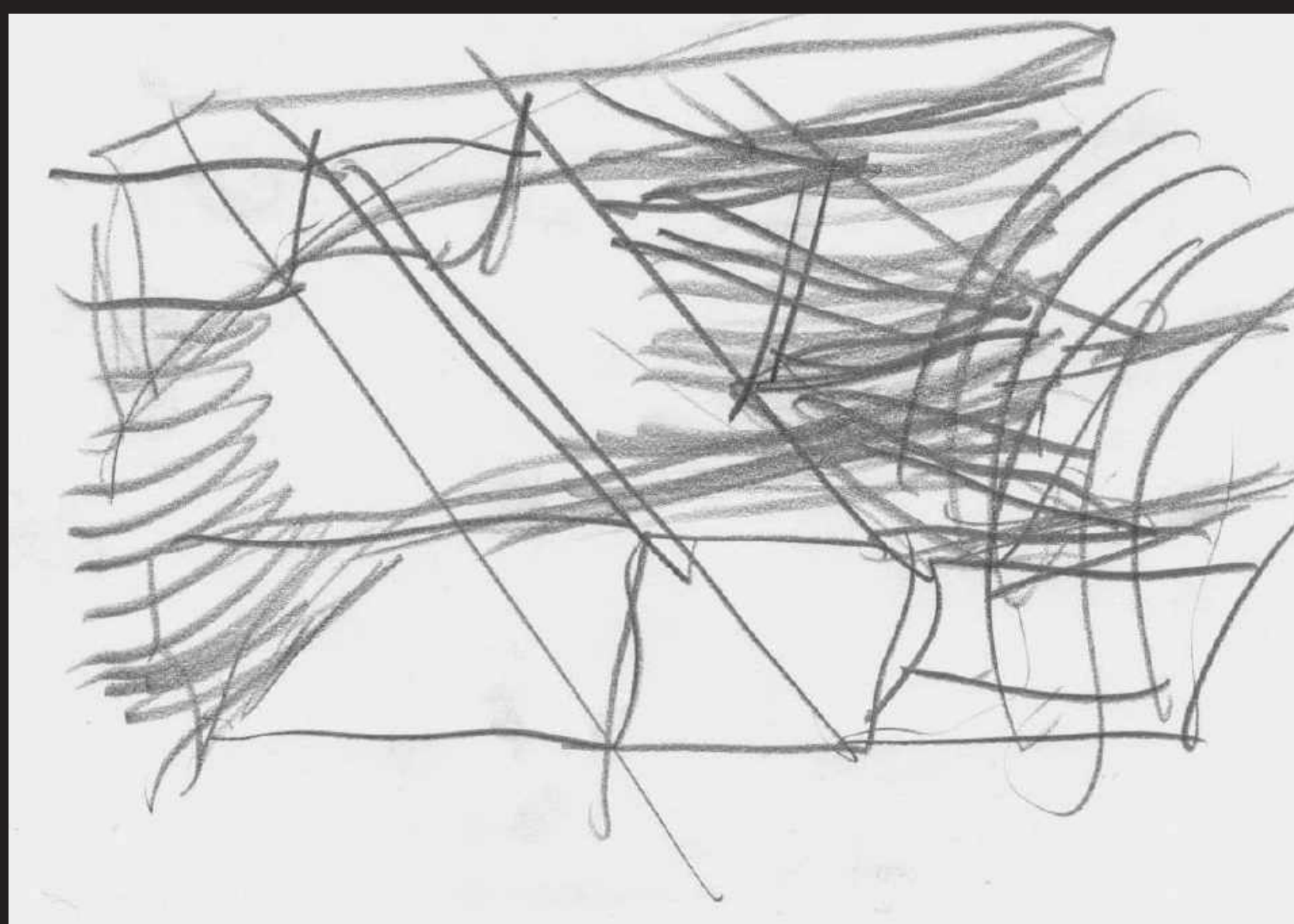
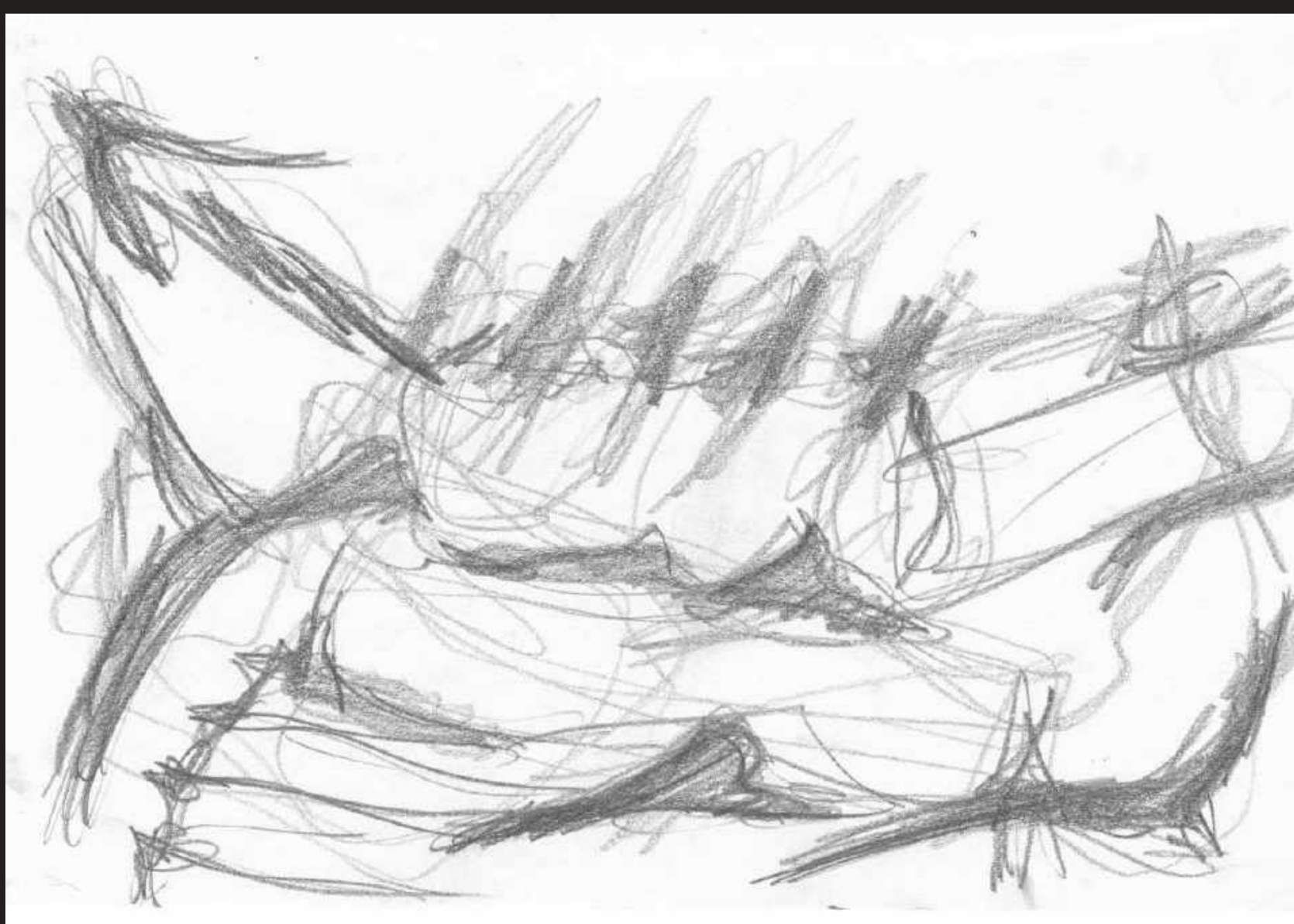
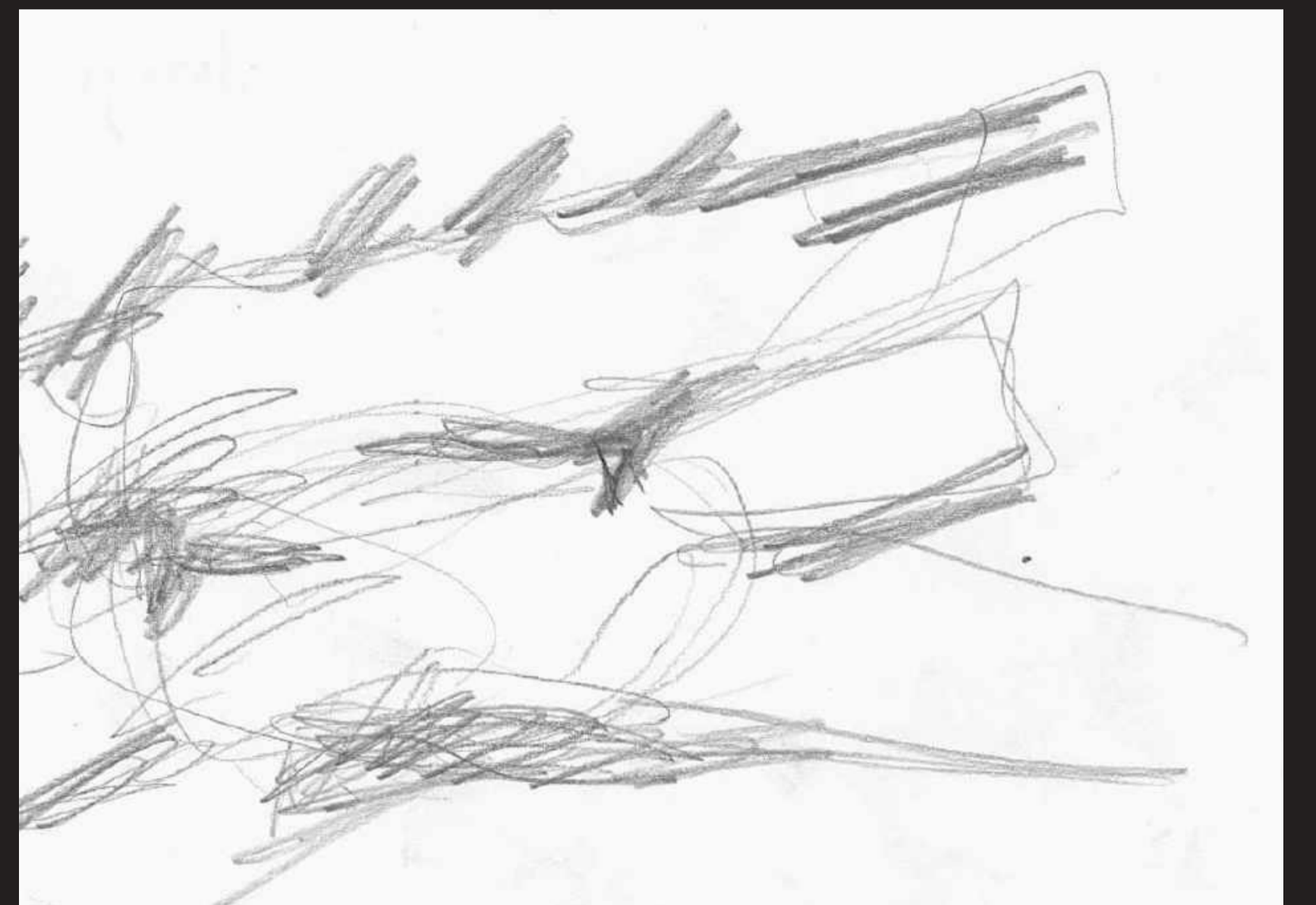
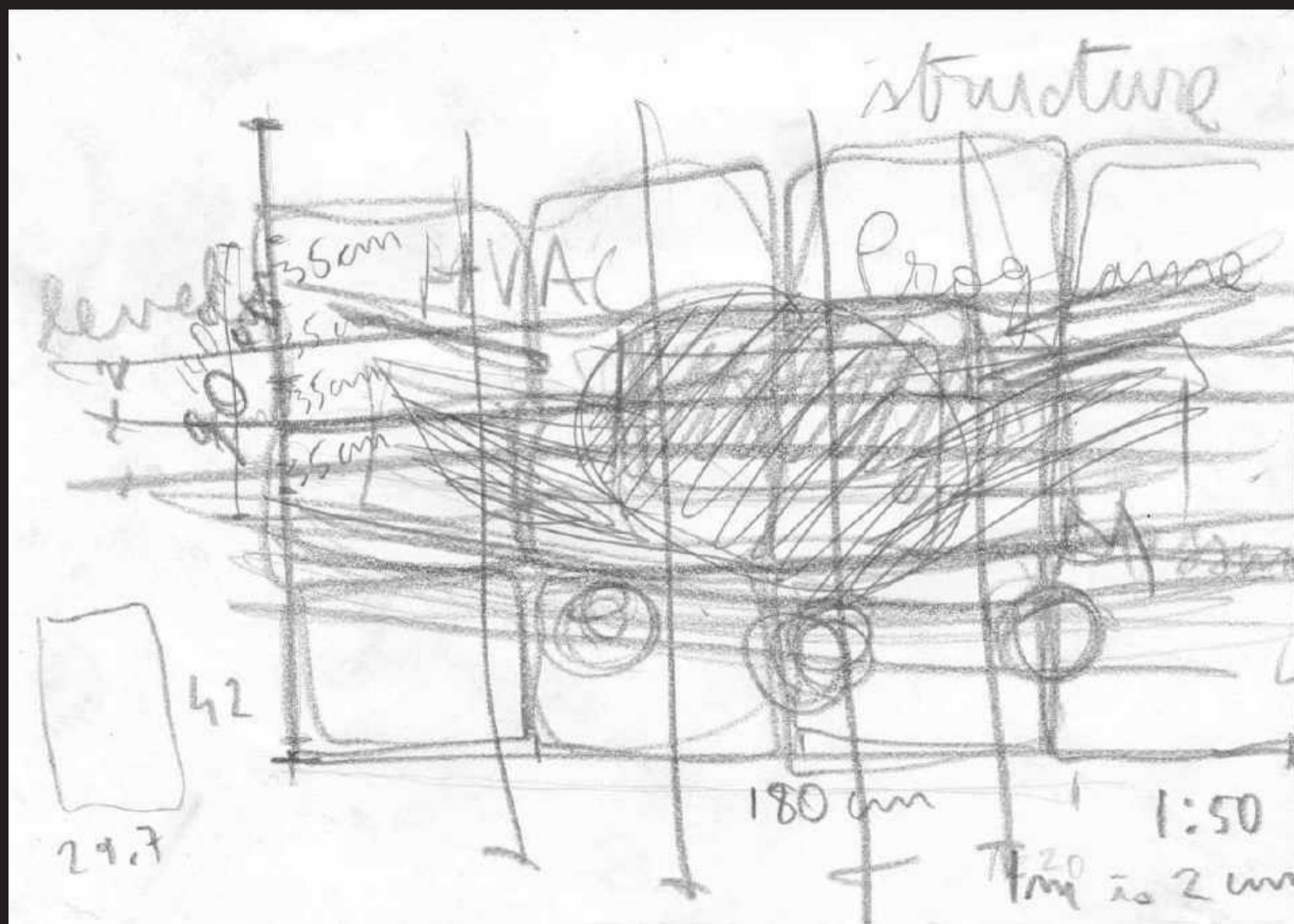
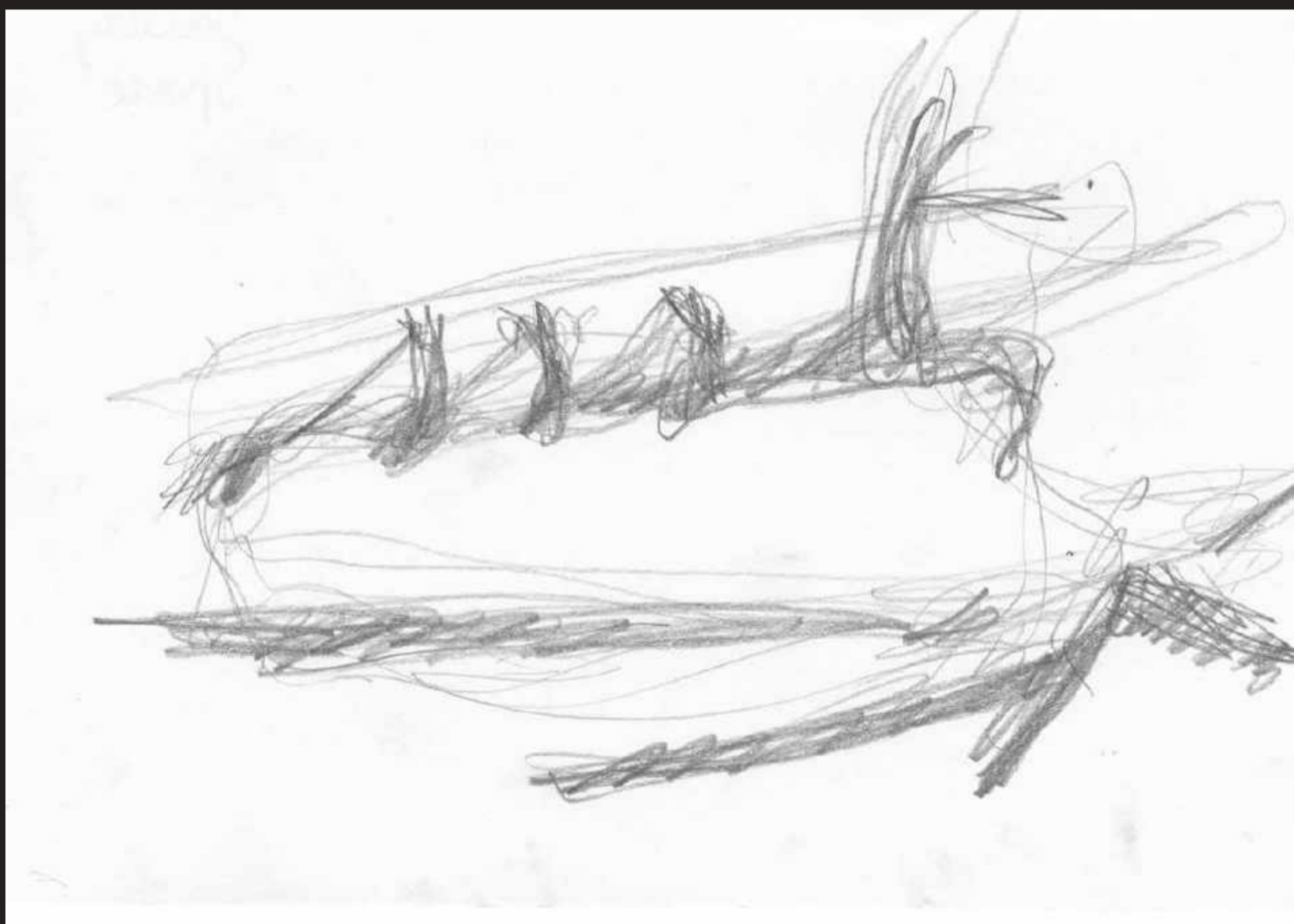
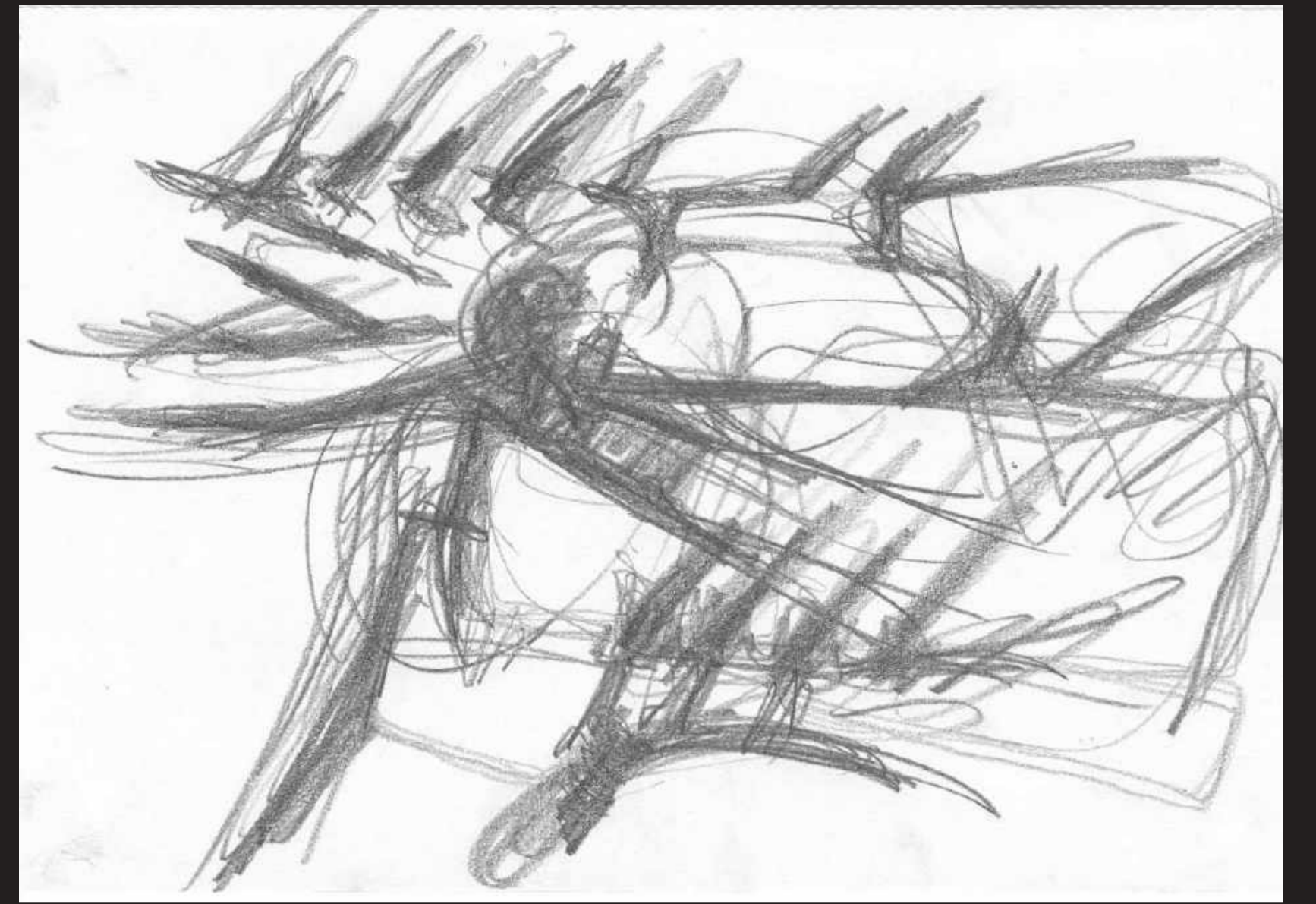
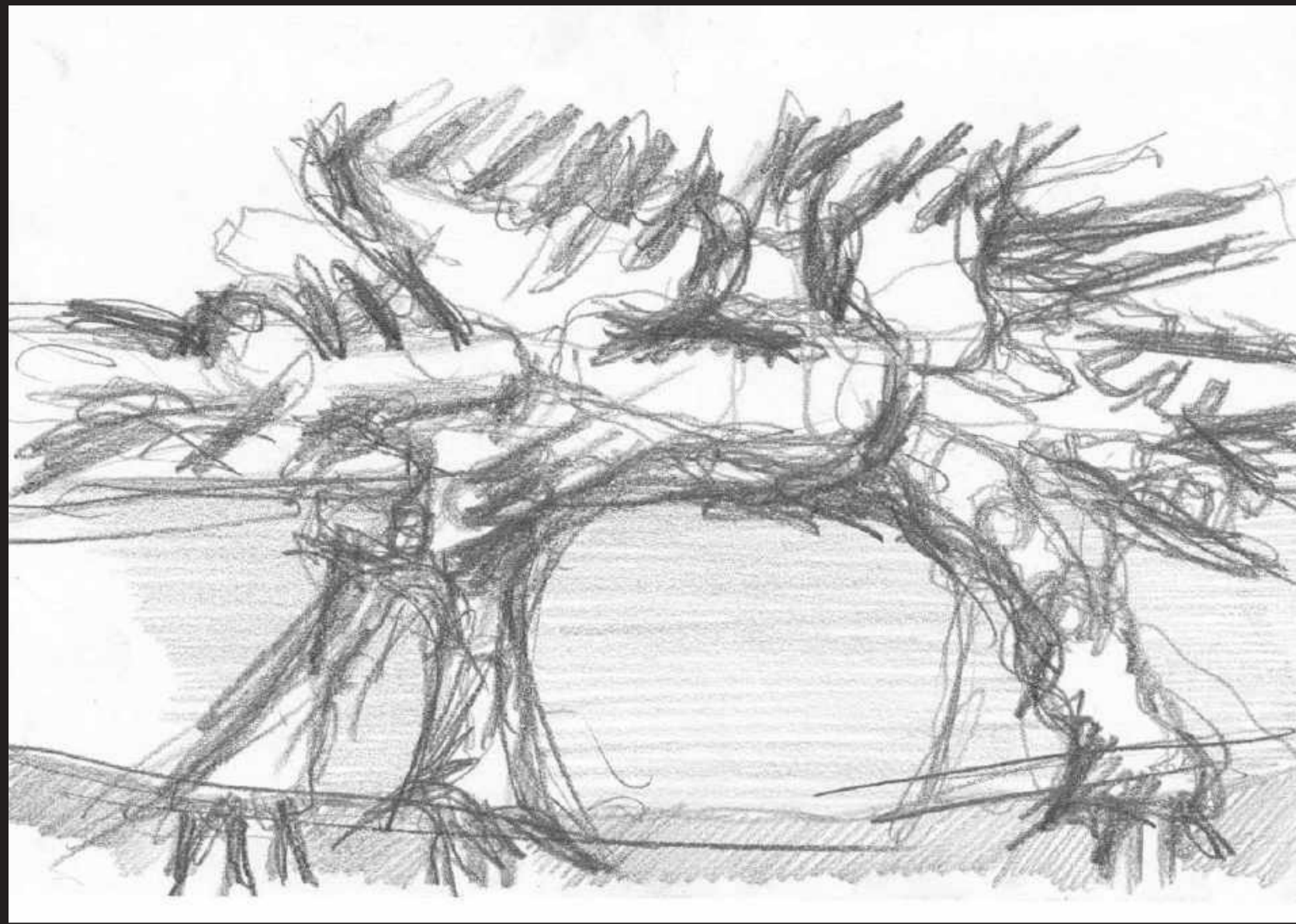
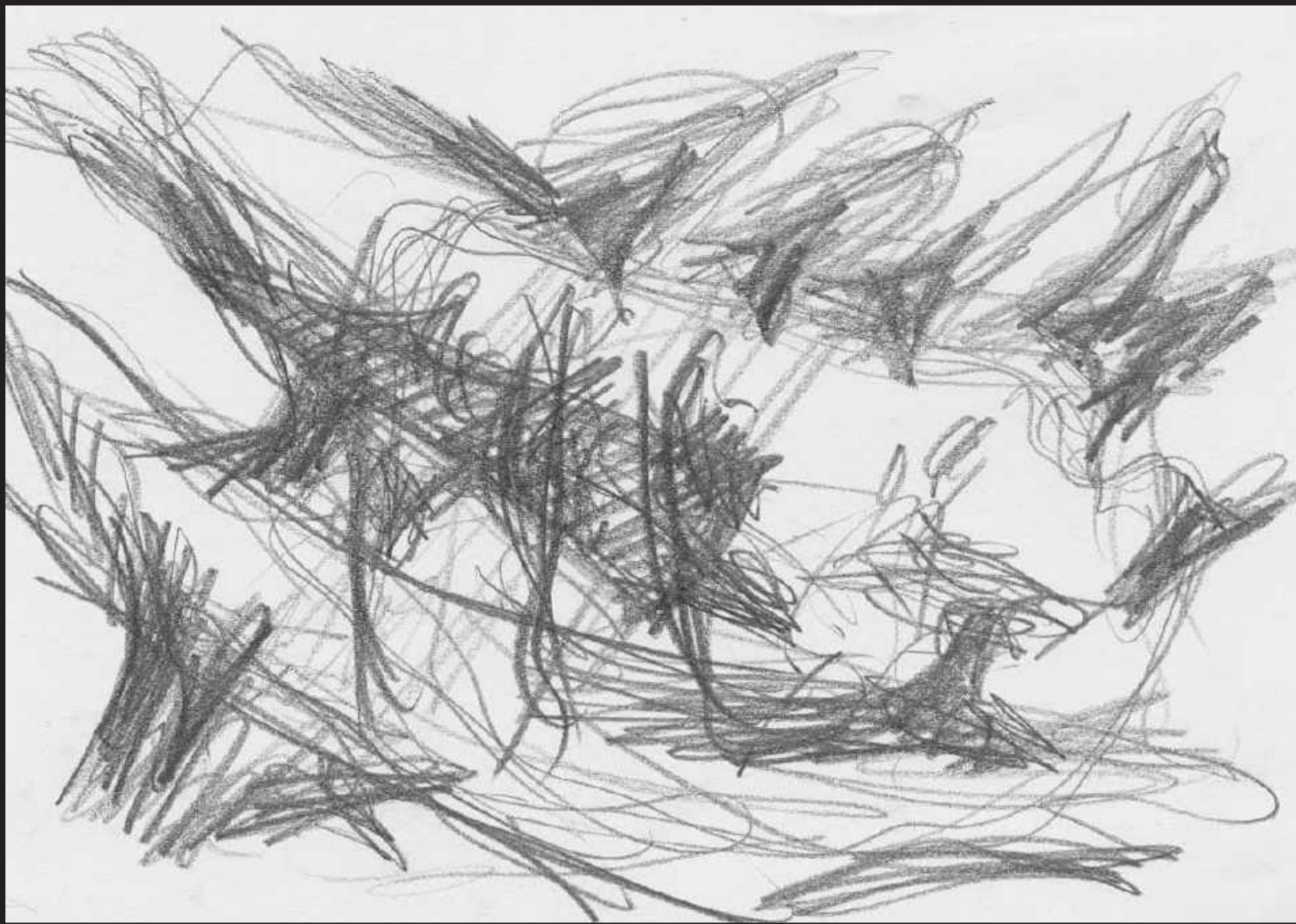


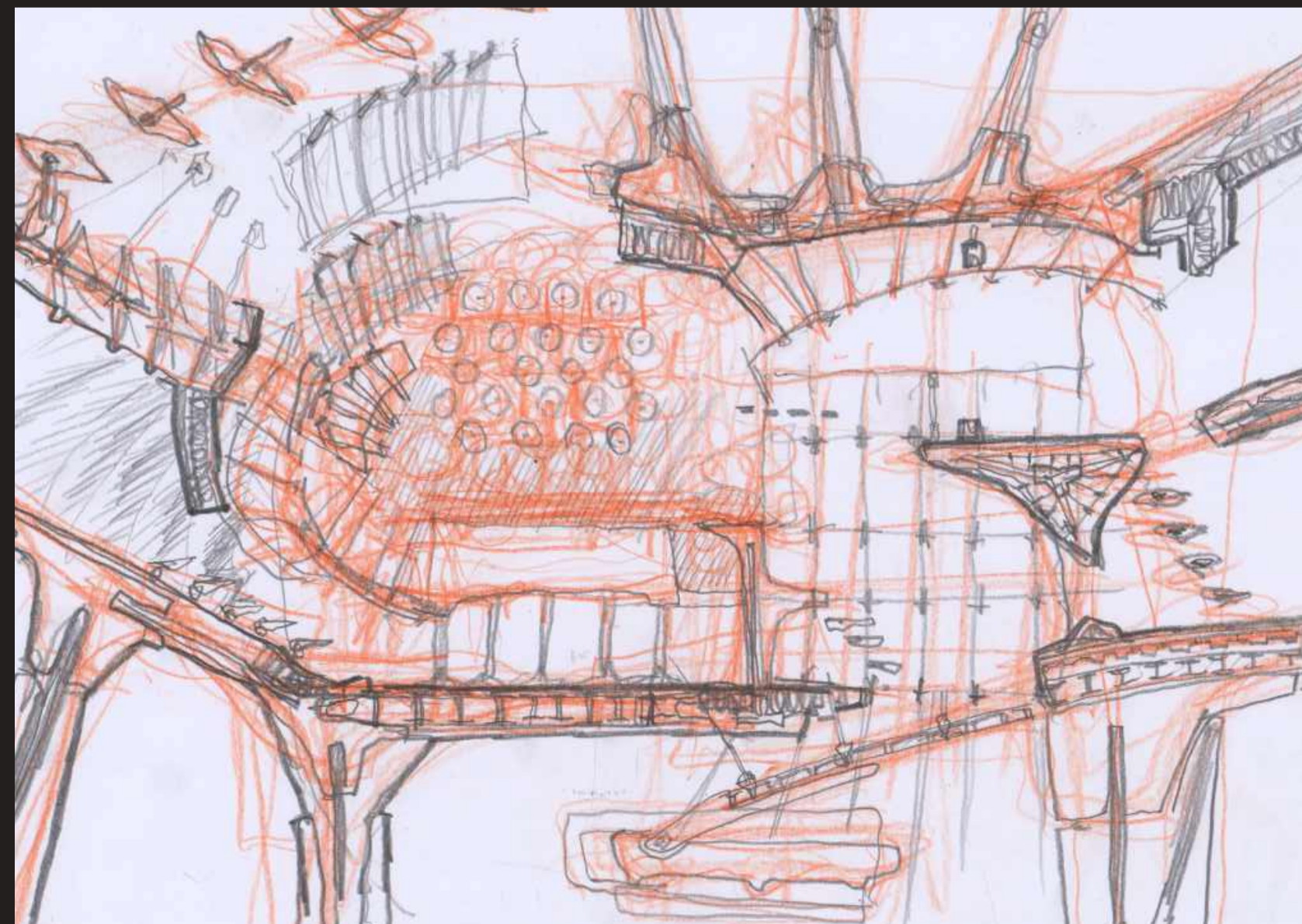
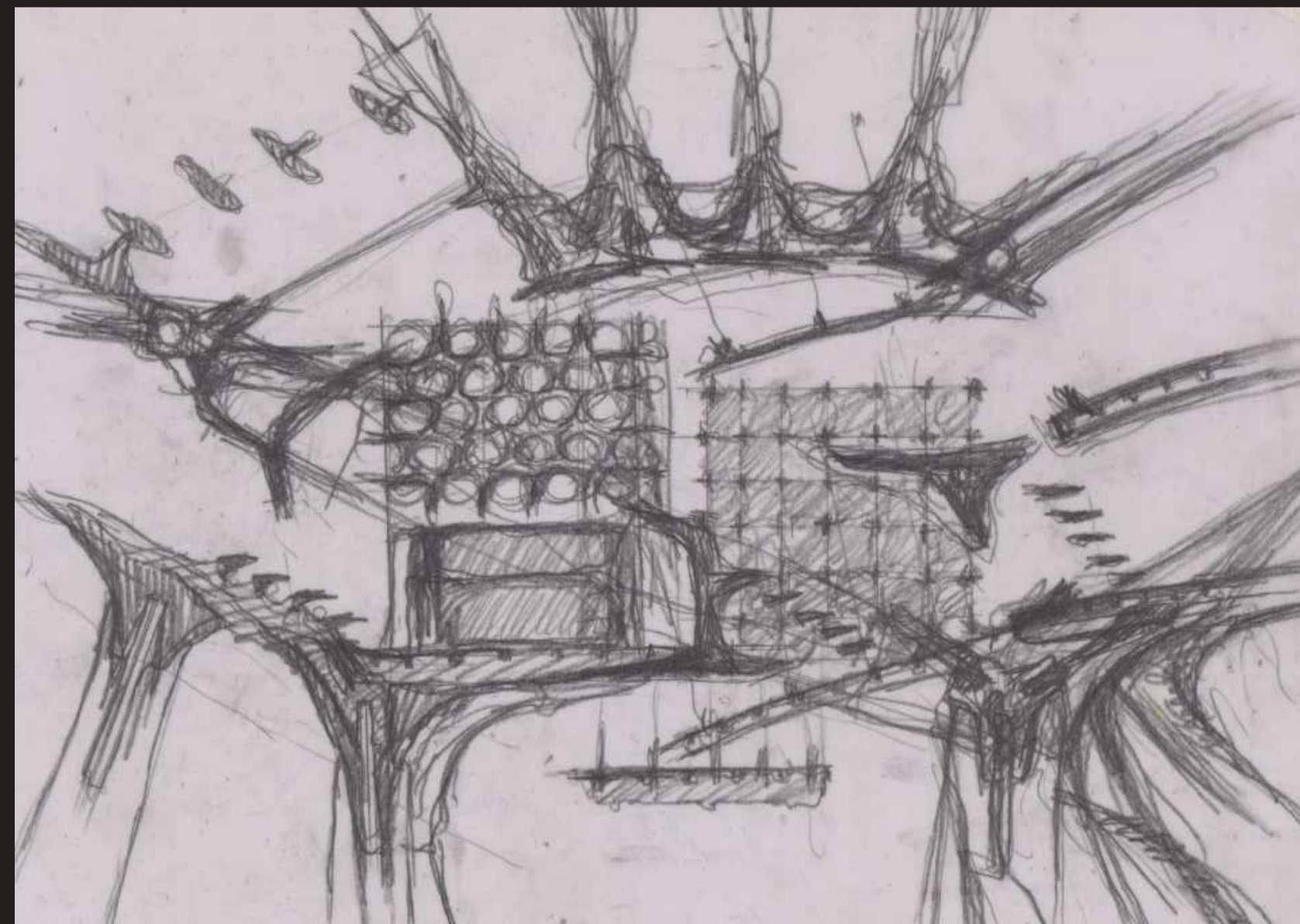
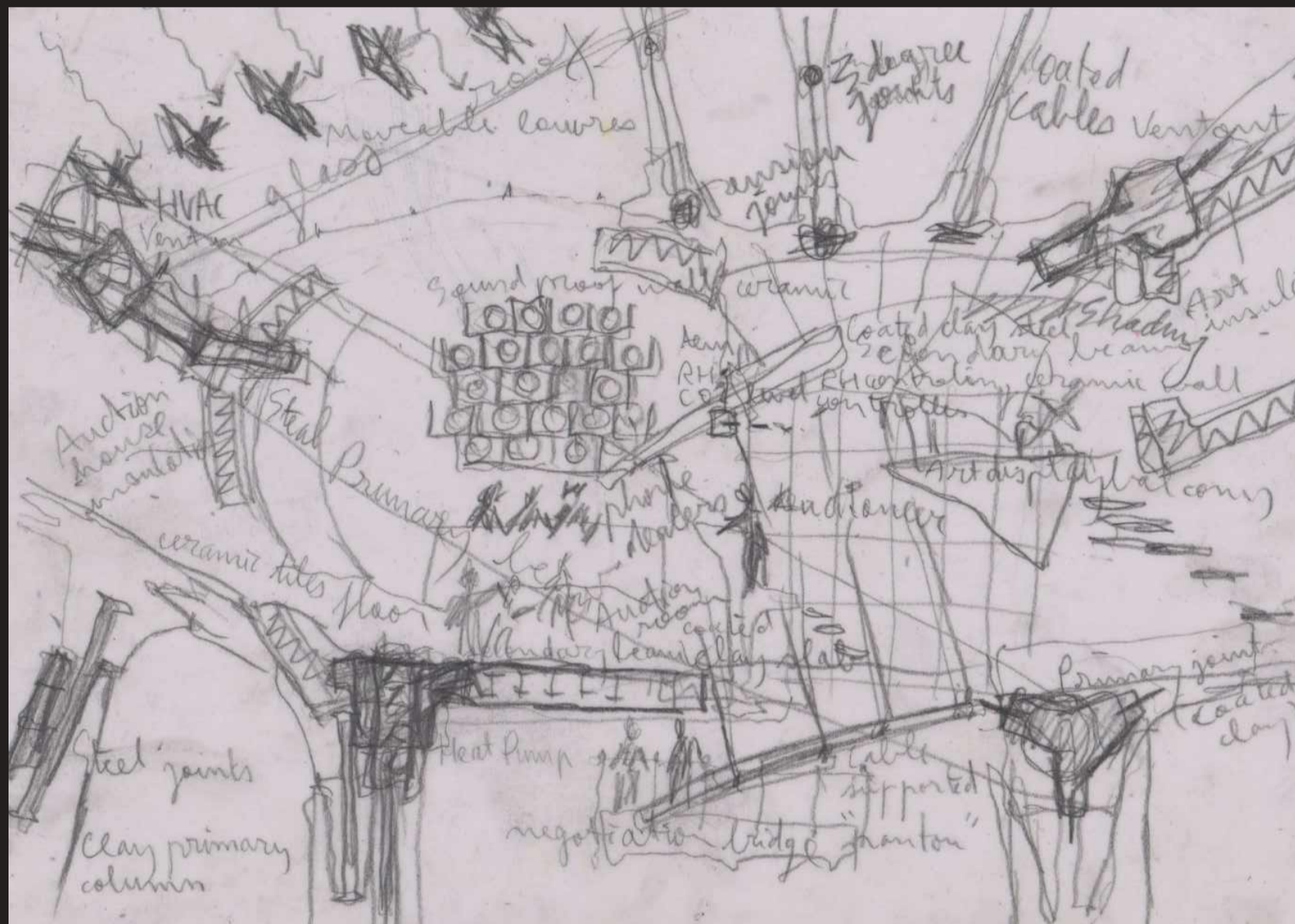
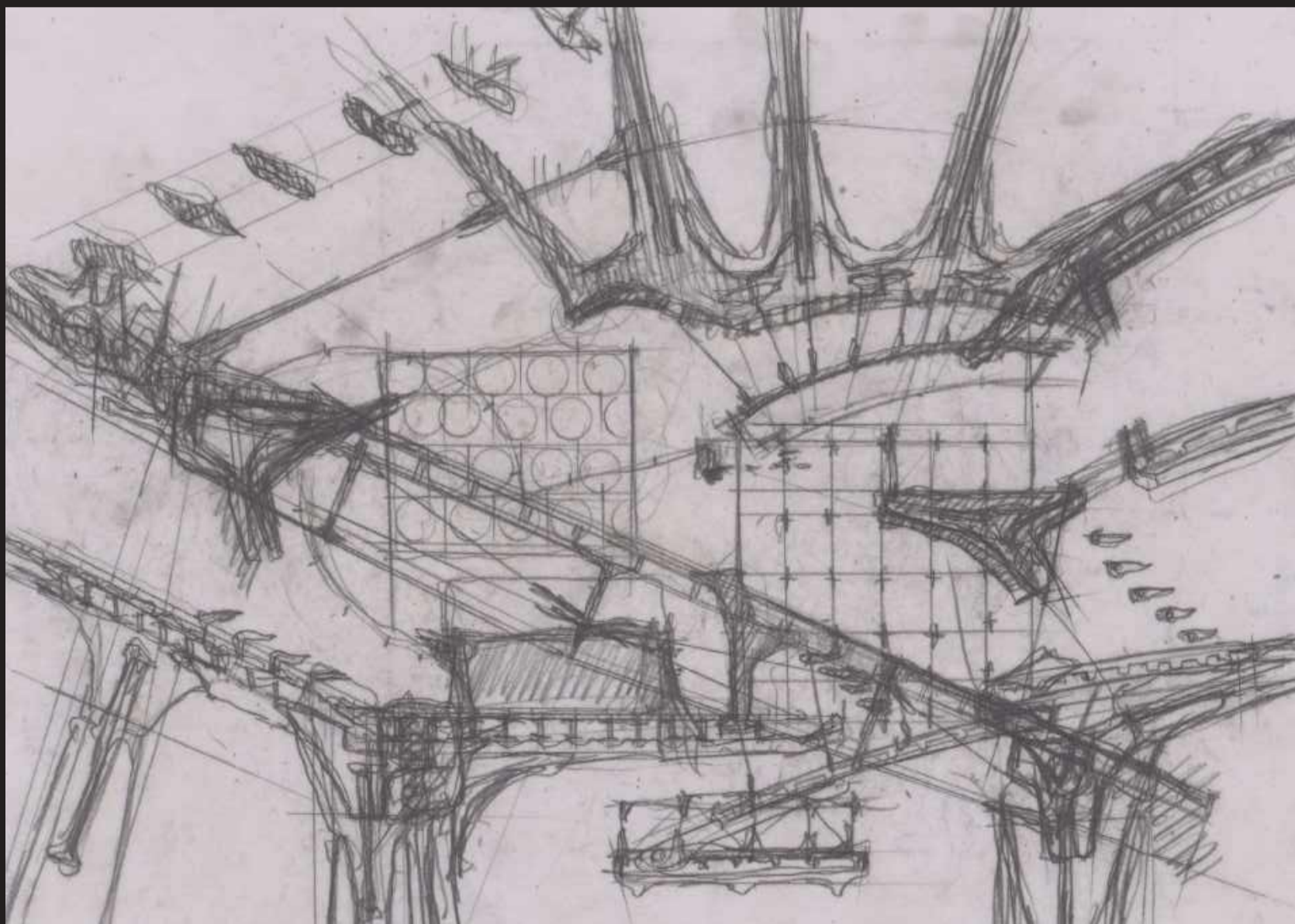
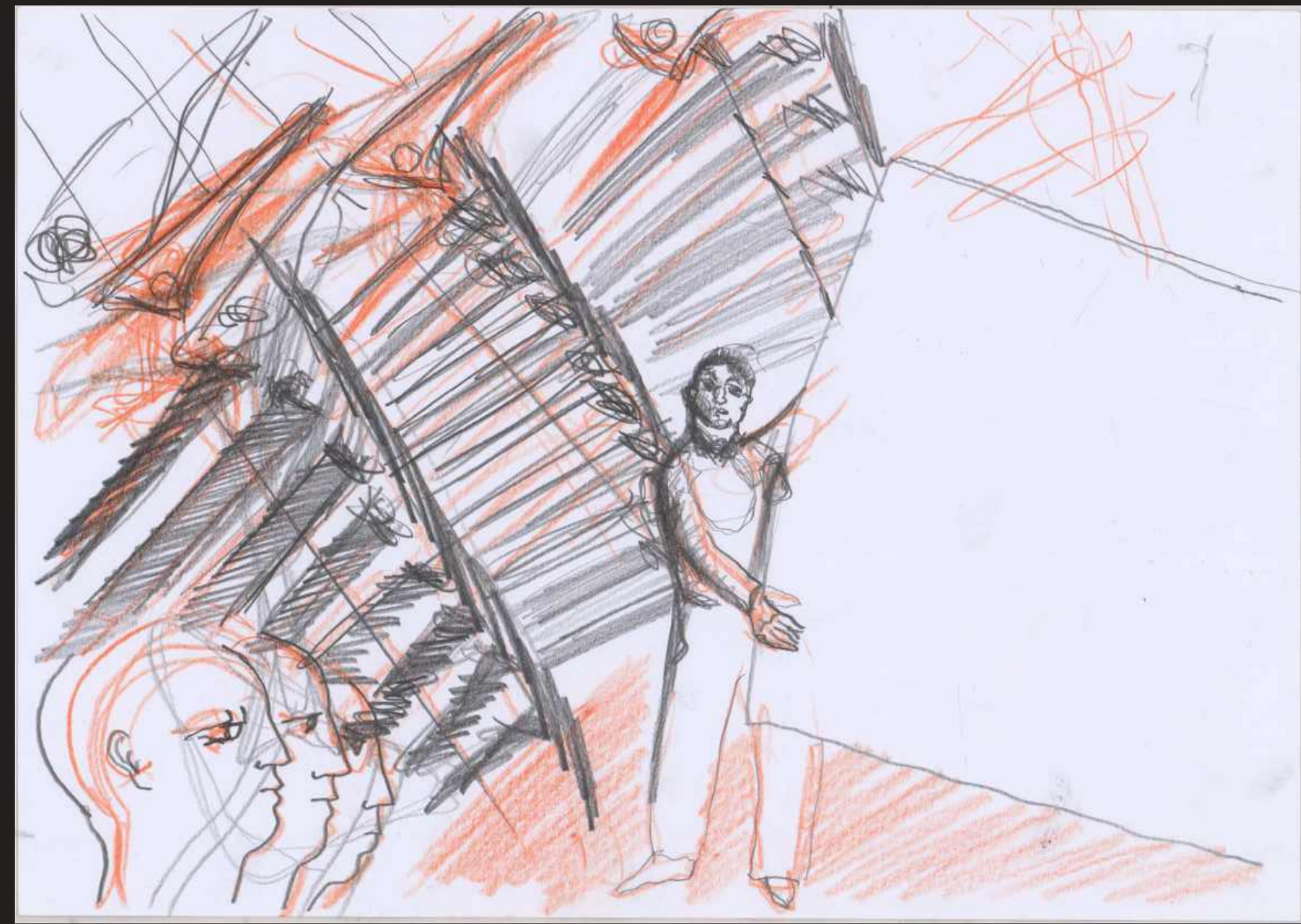
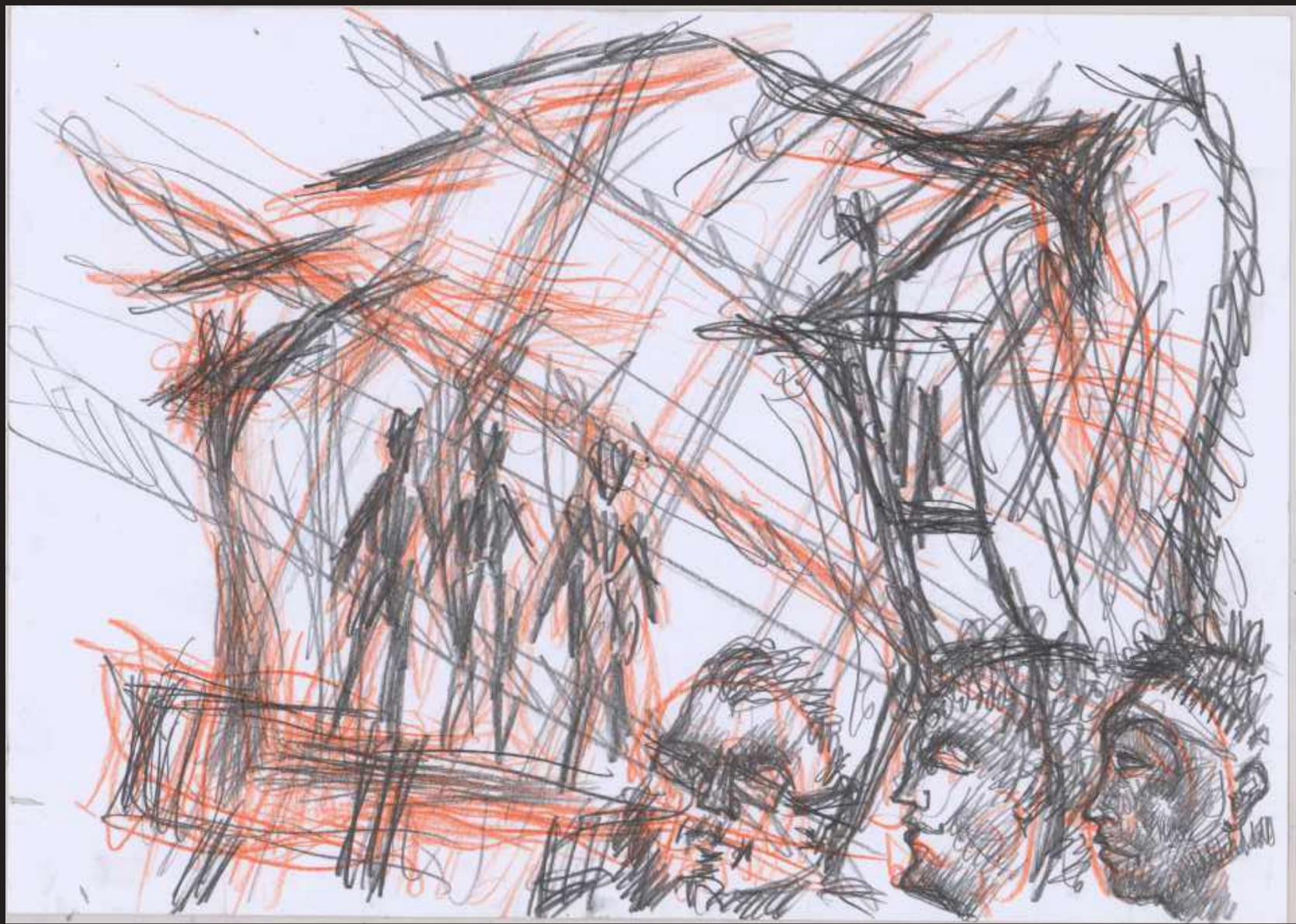
plan

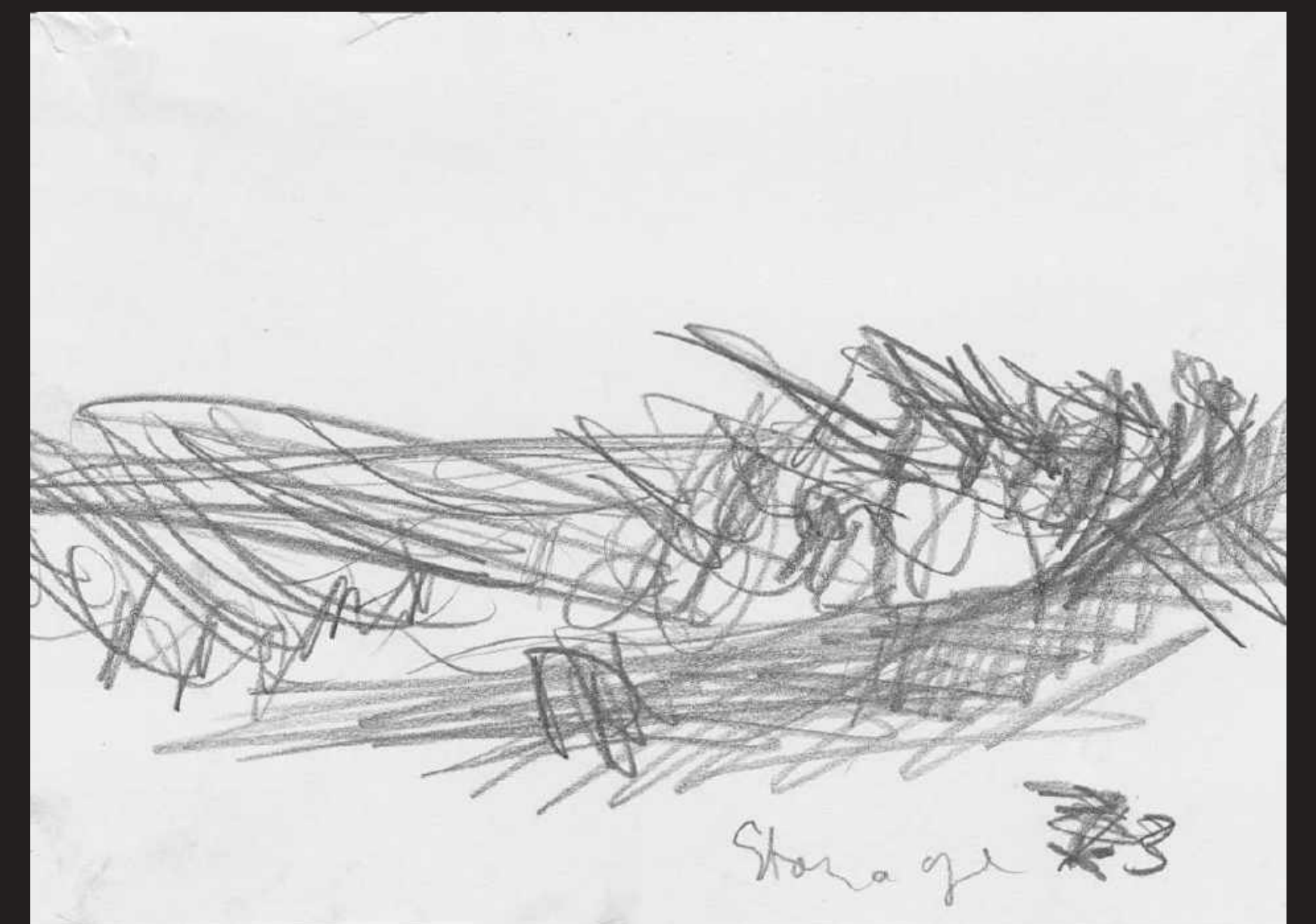
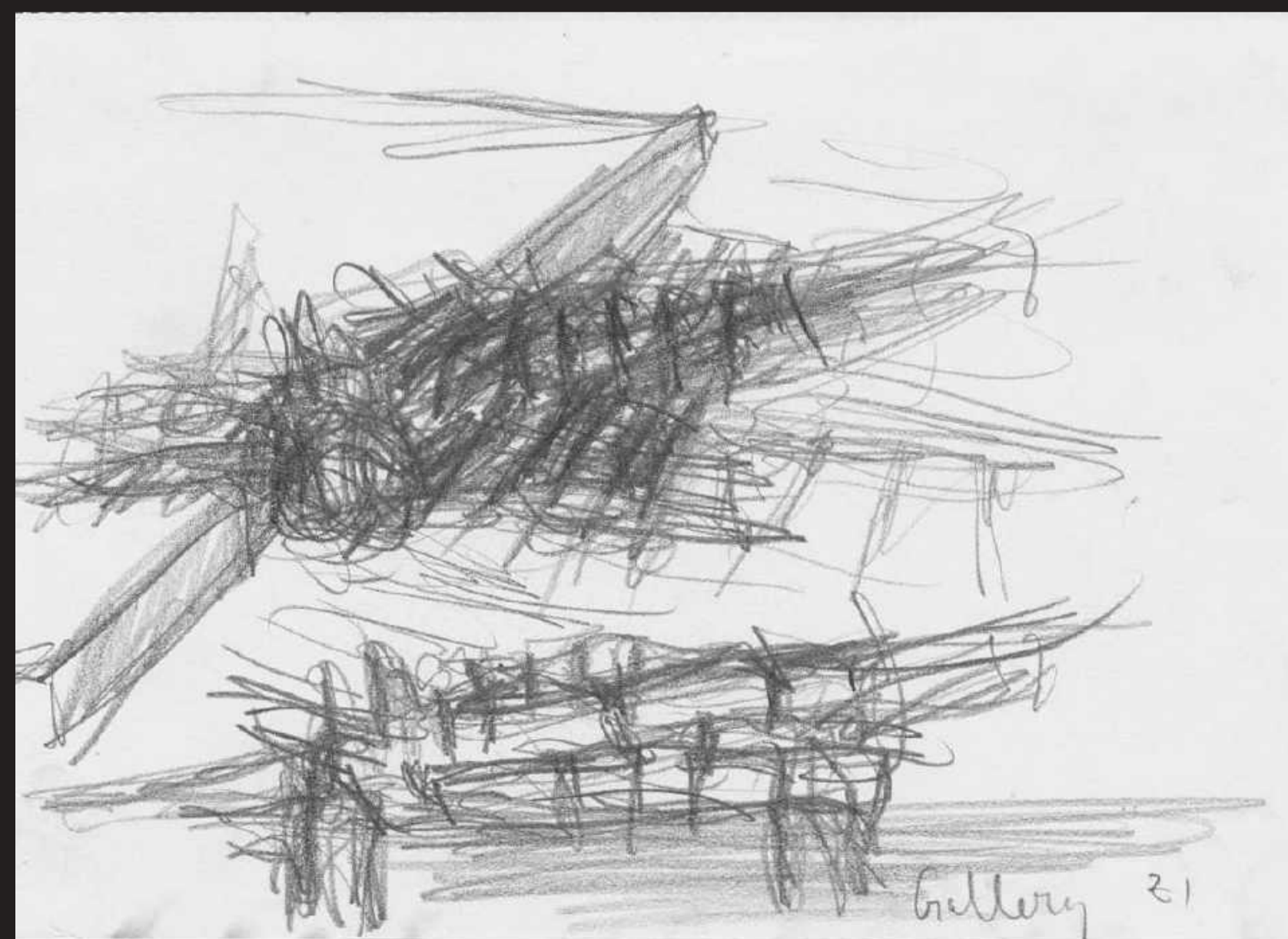
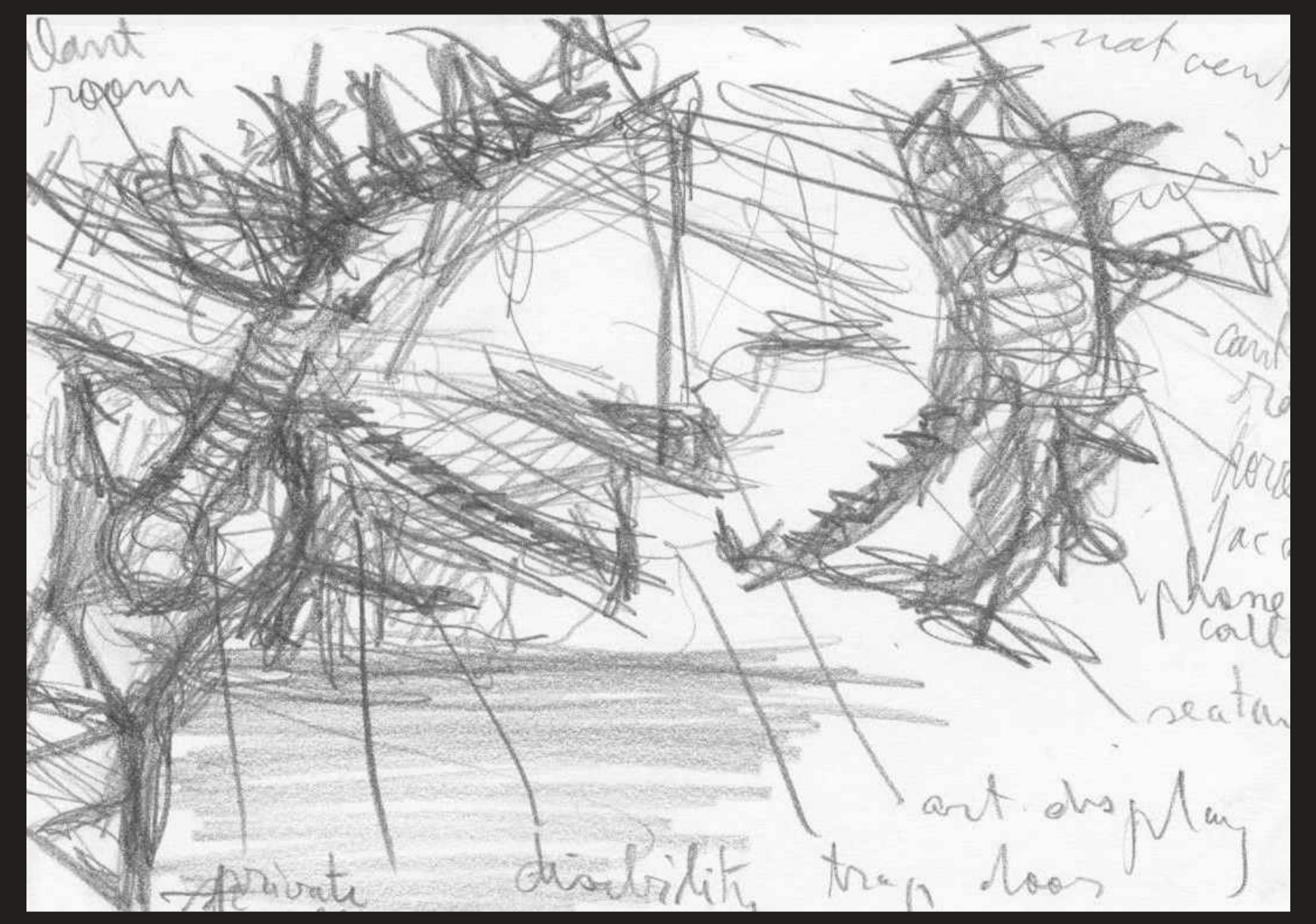
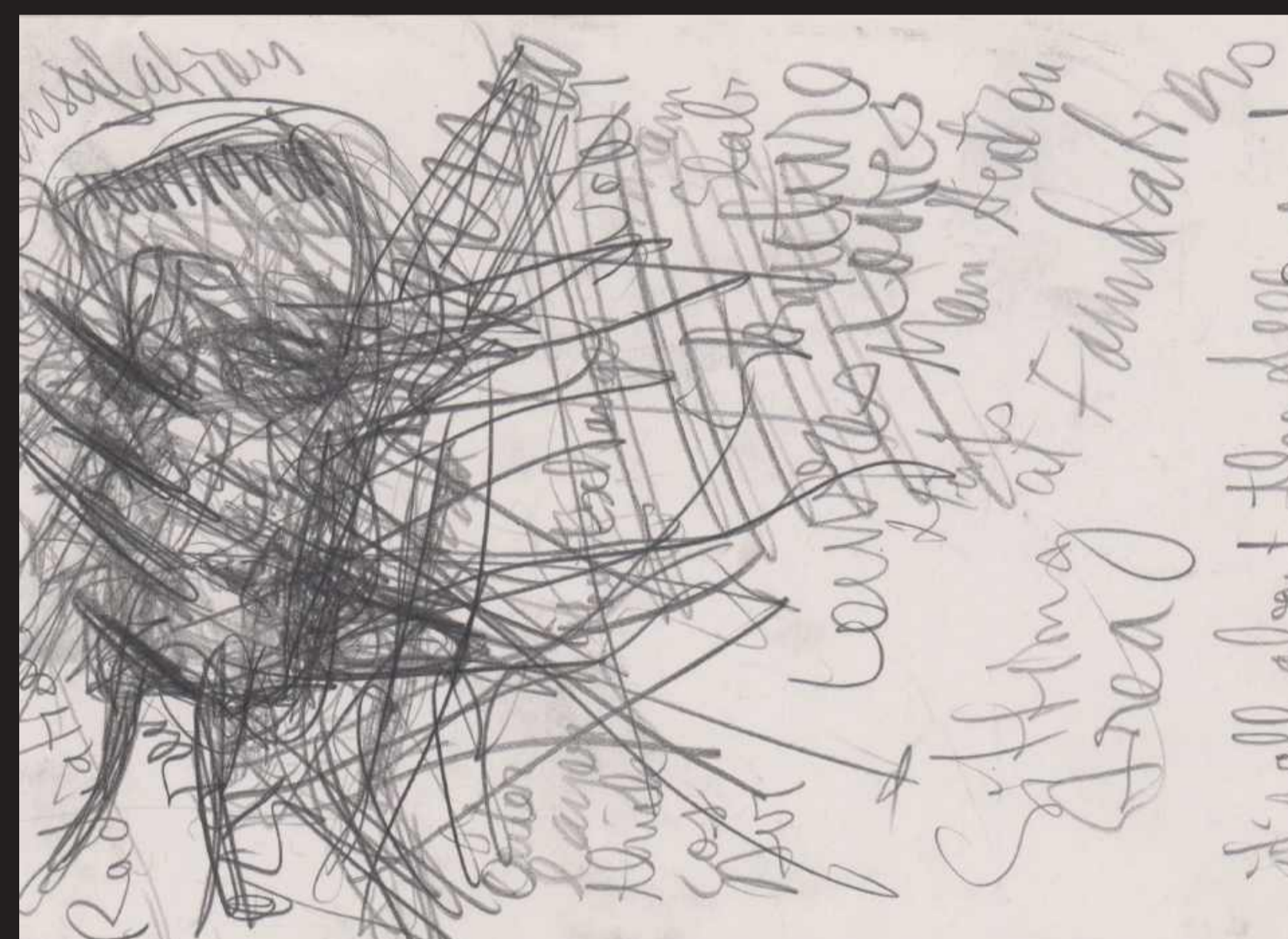
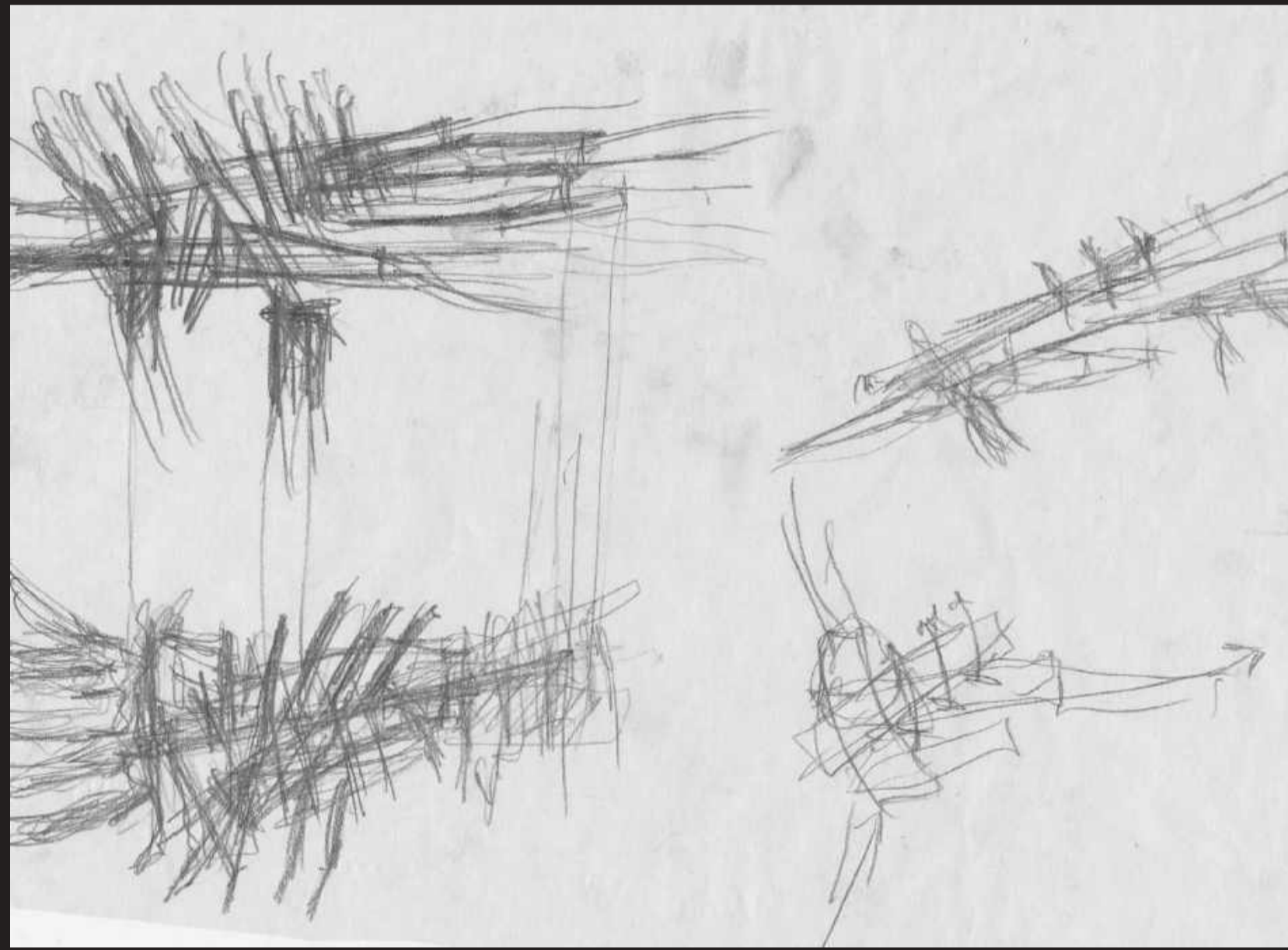
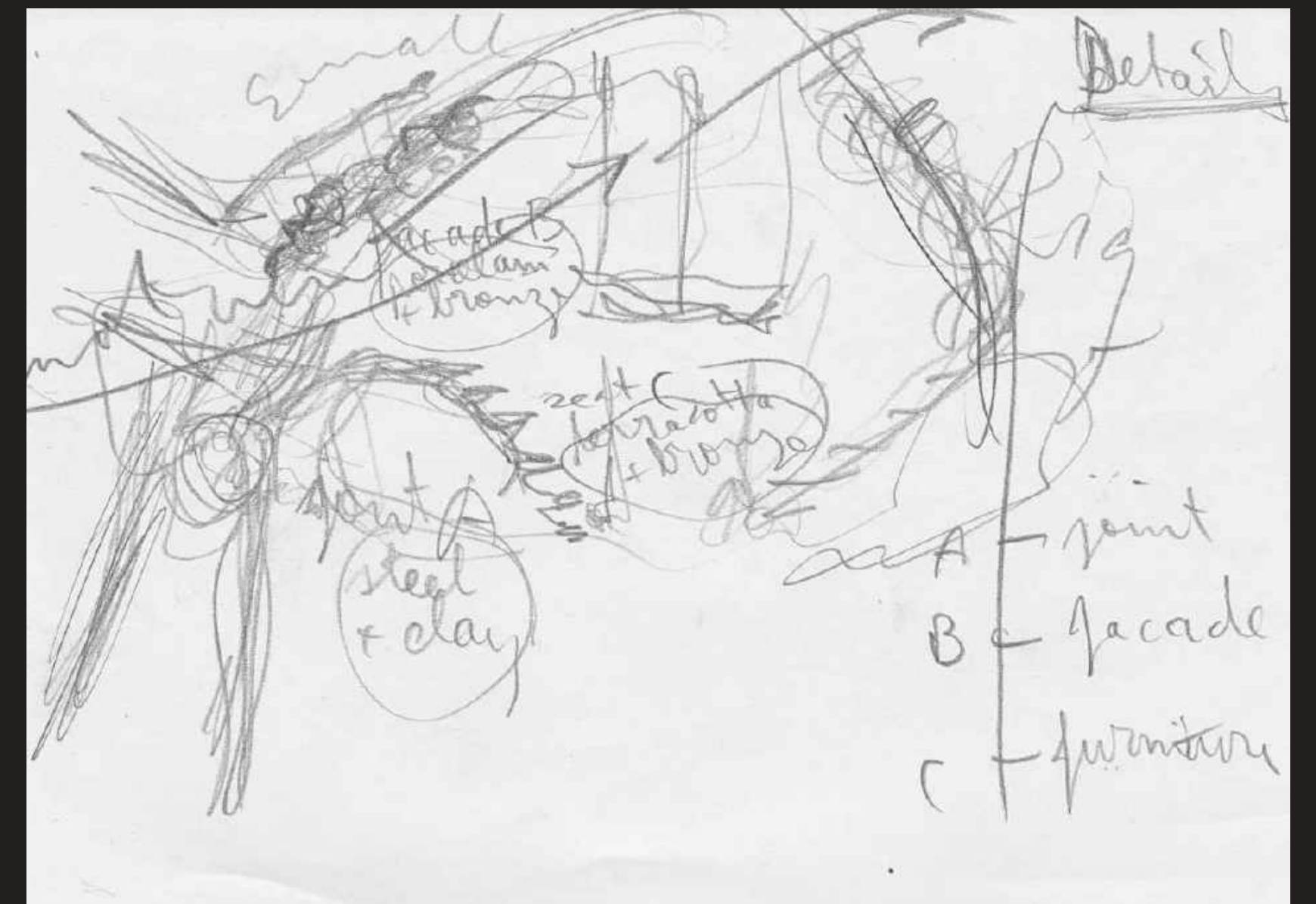
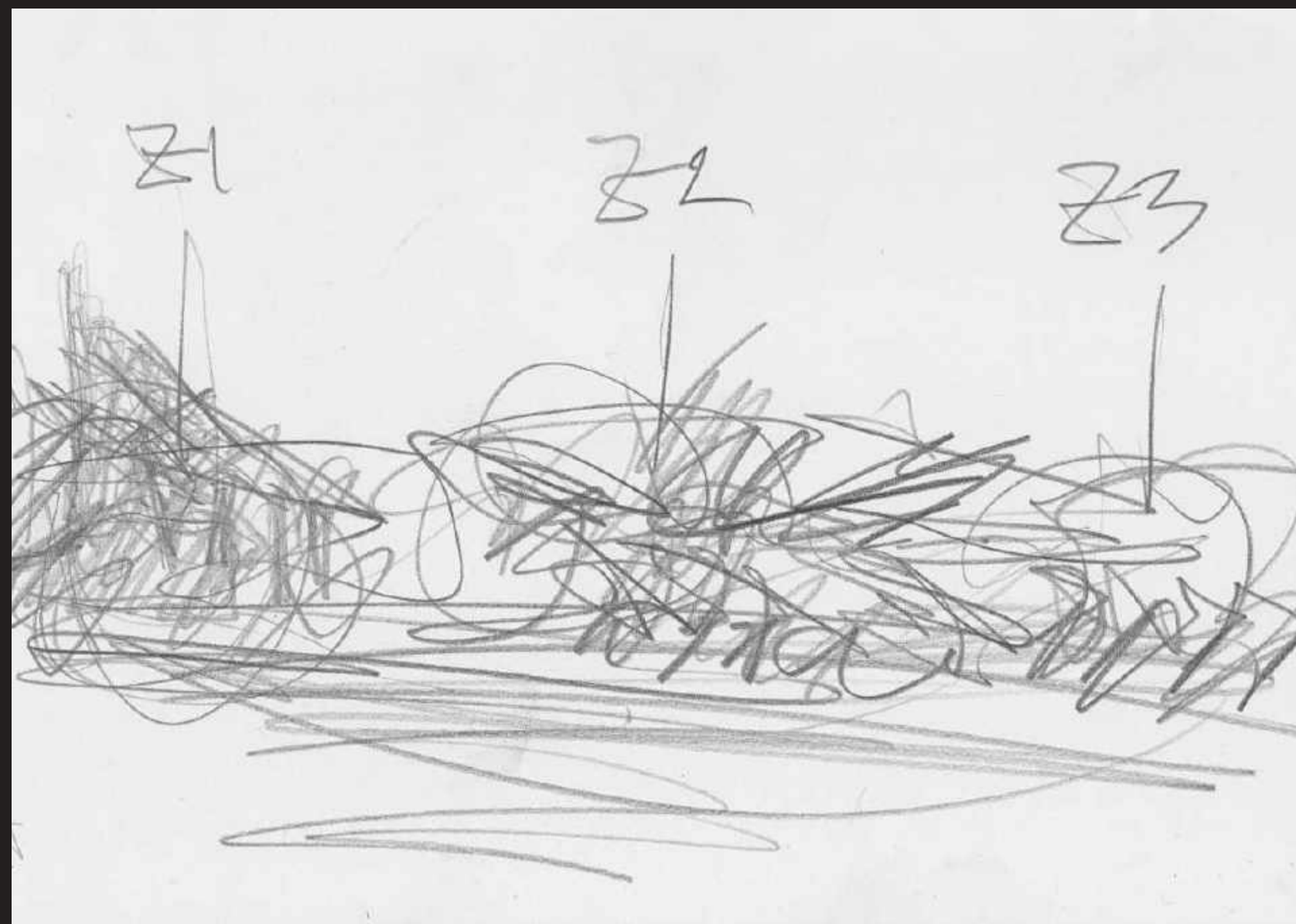
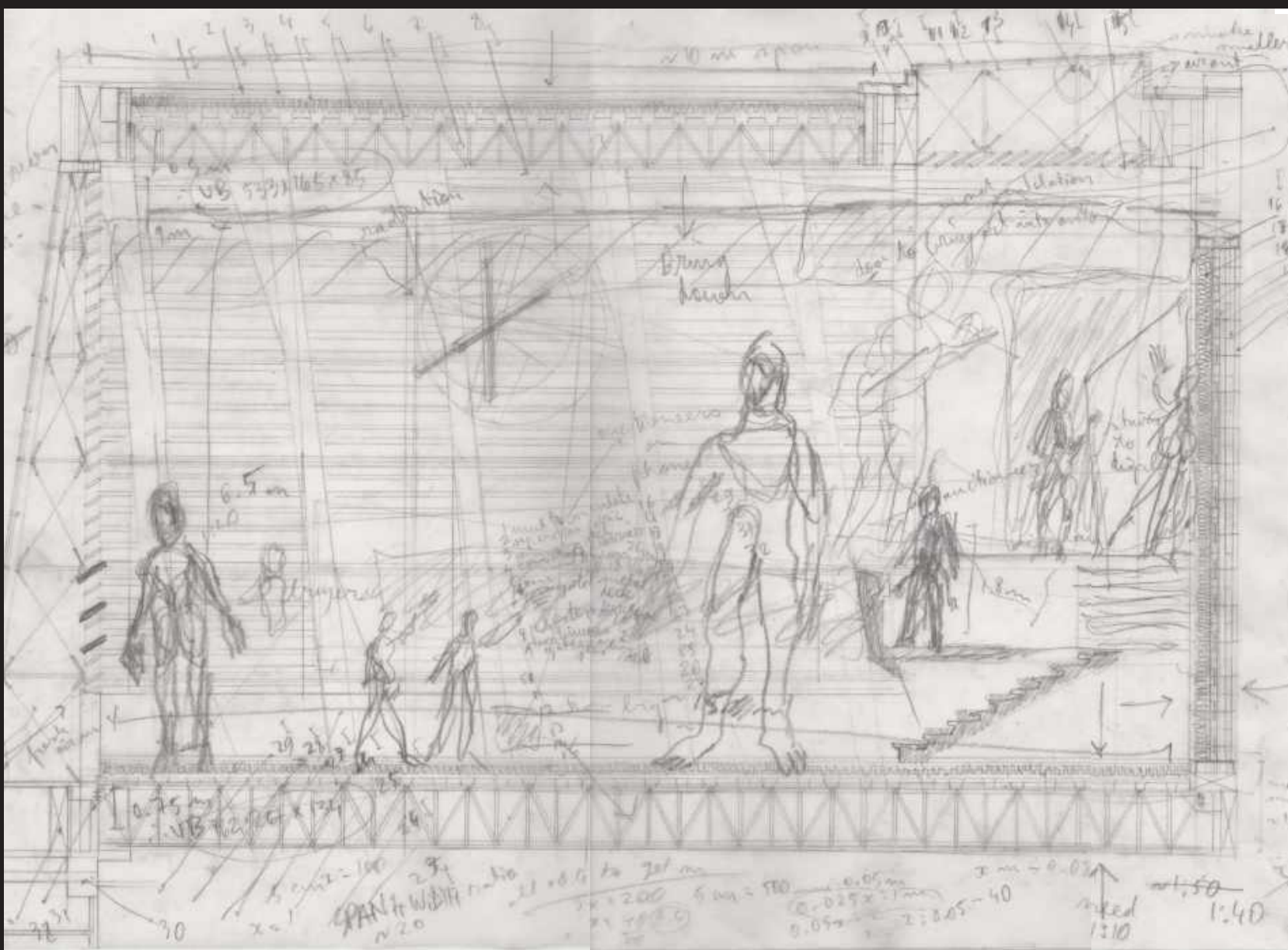
PAPER MODEL SCAN

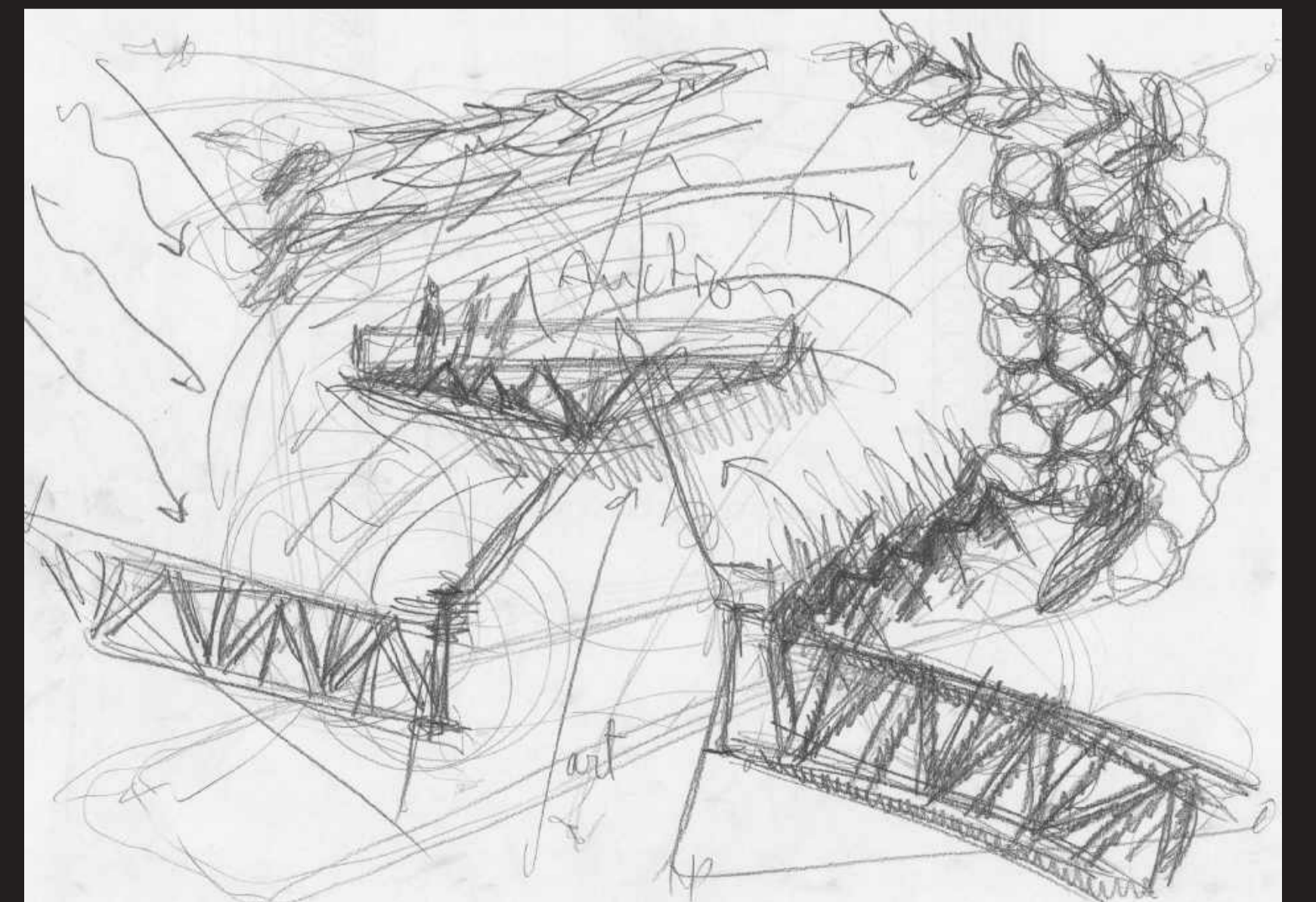
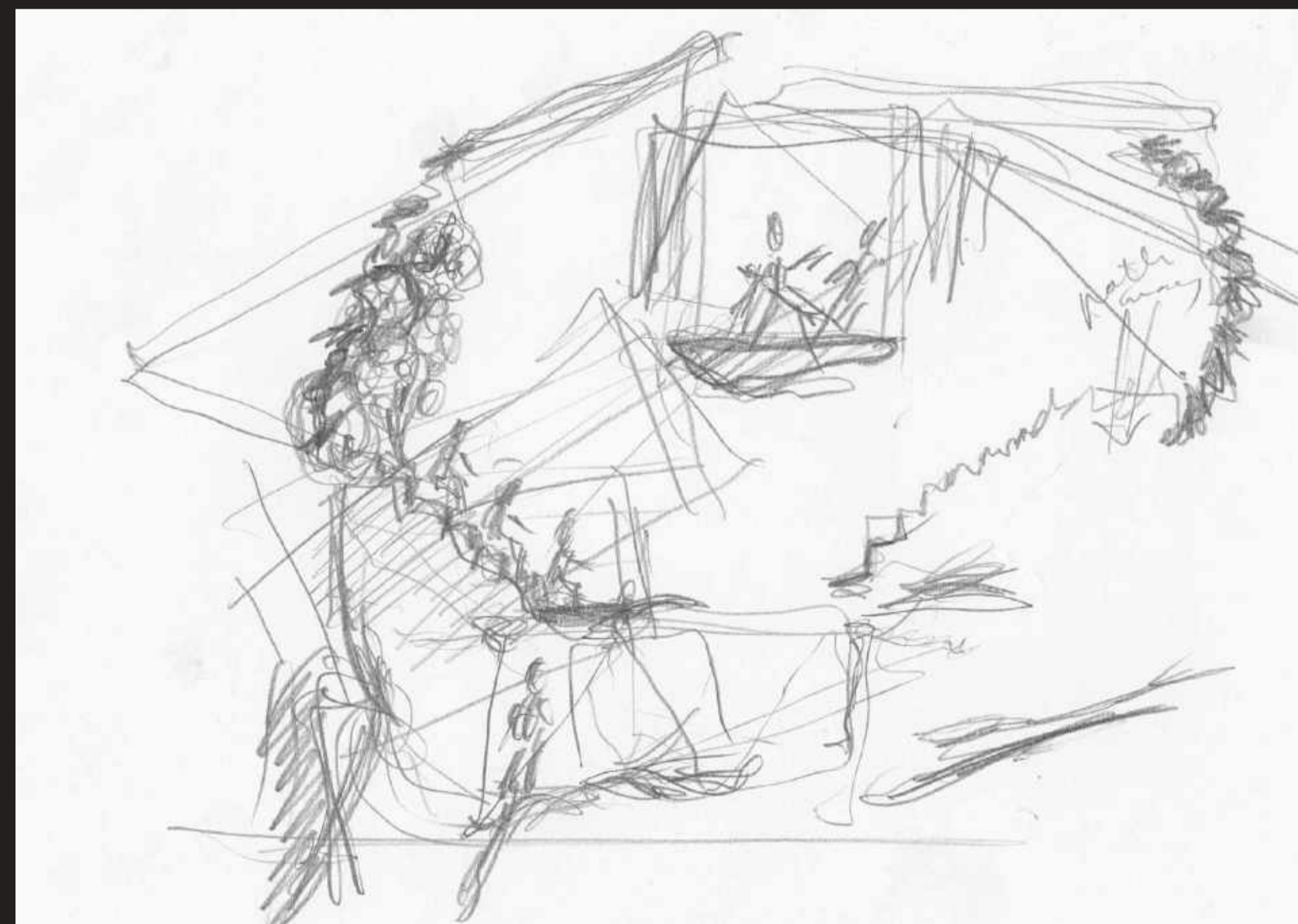
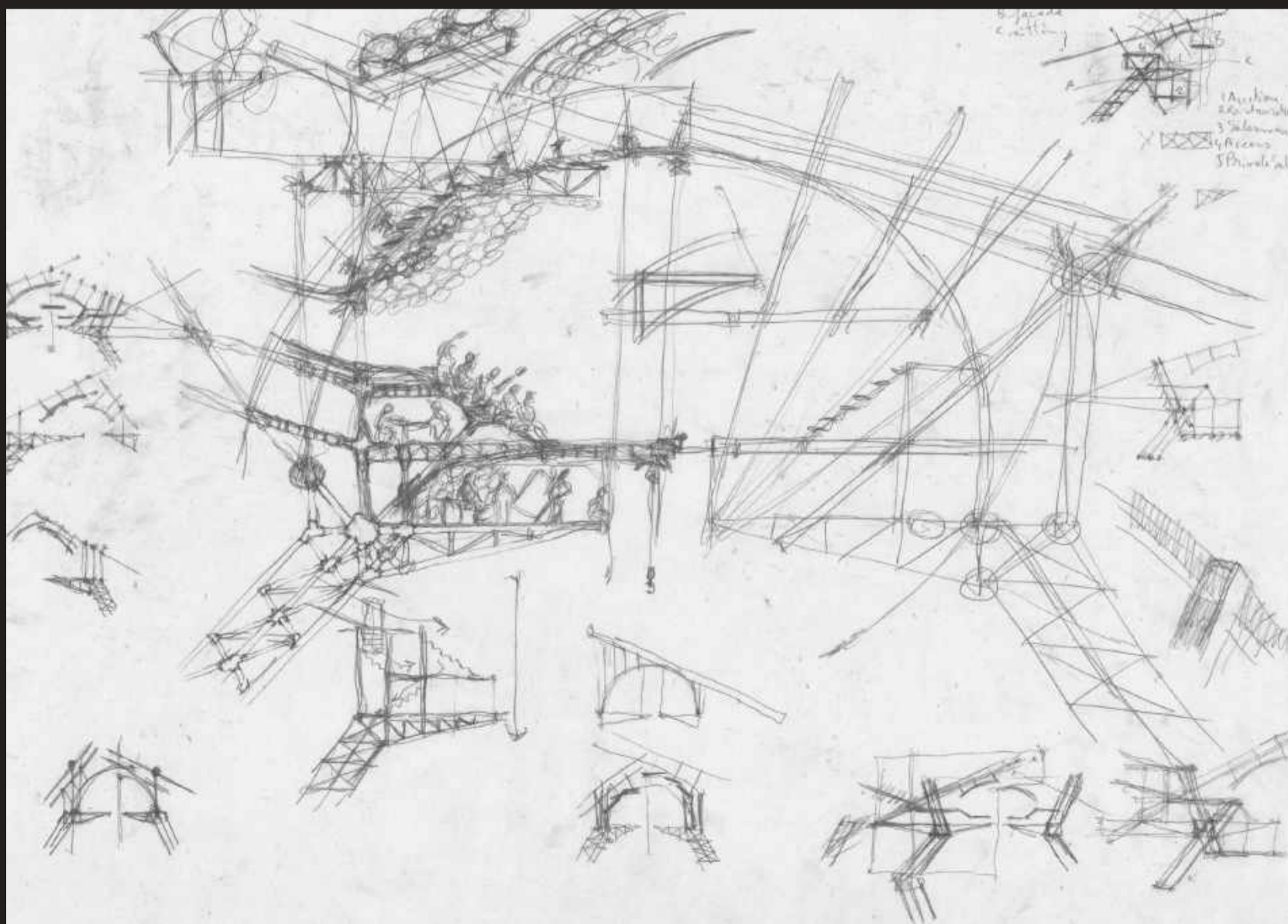
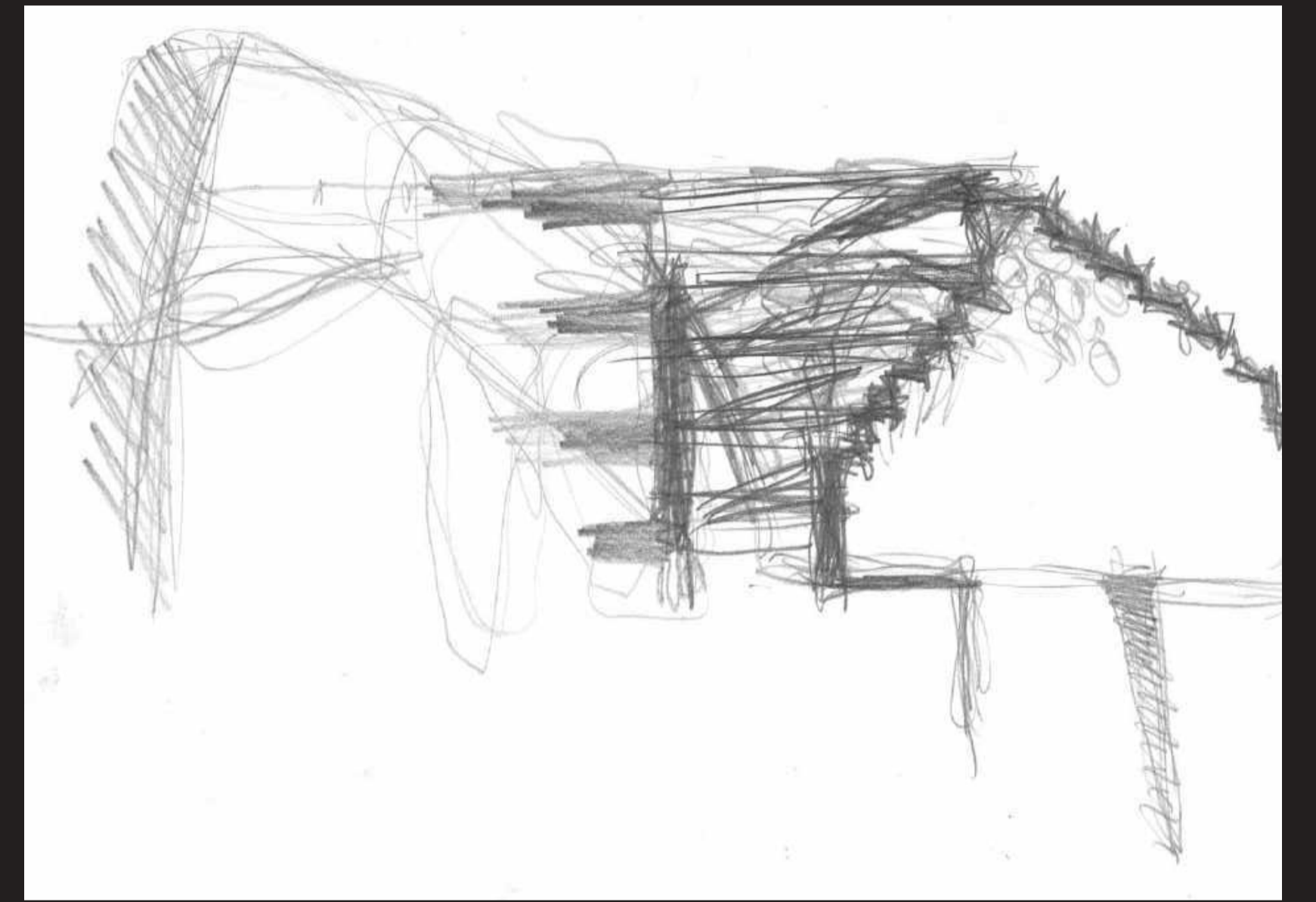
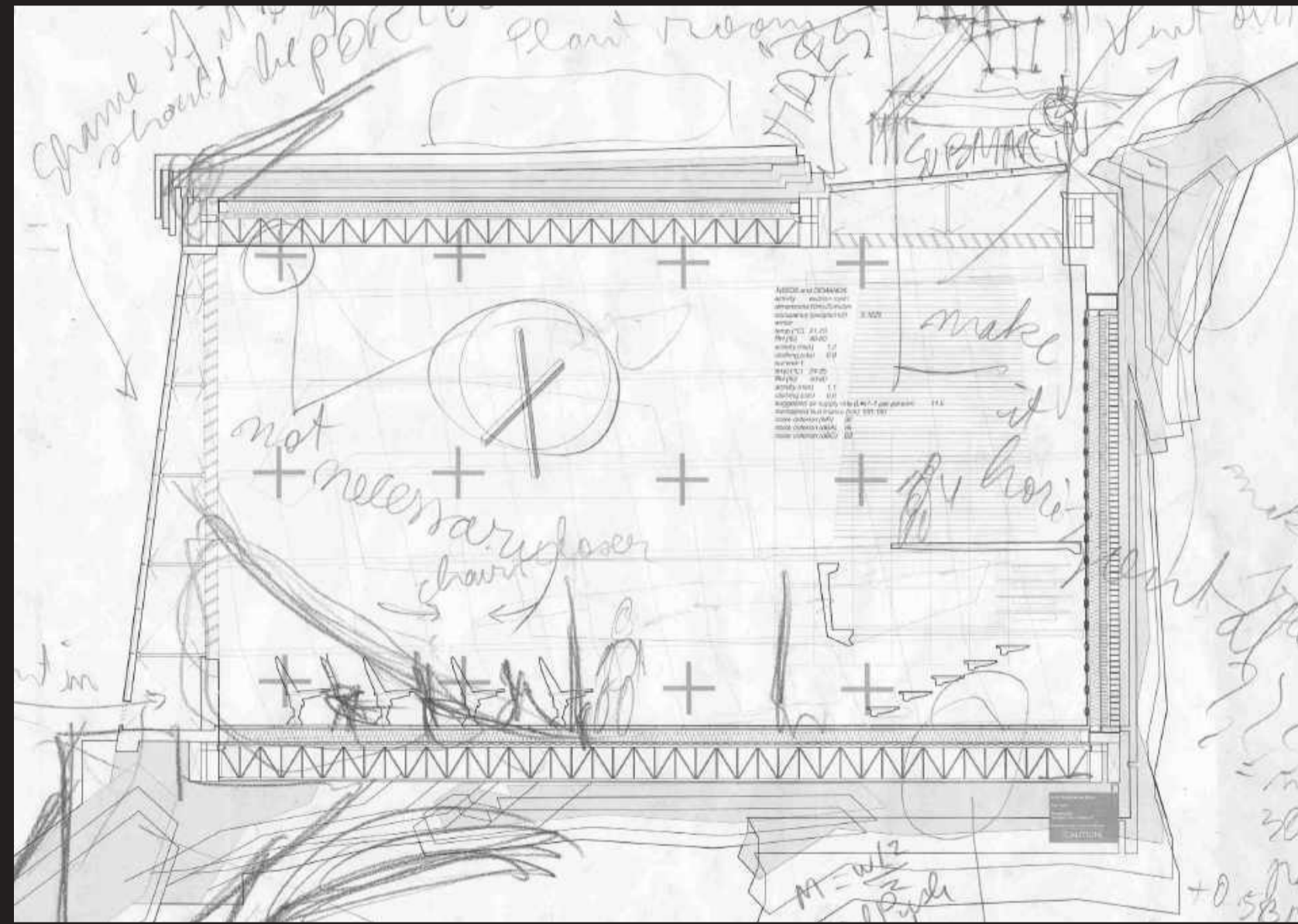
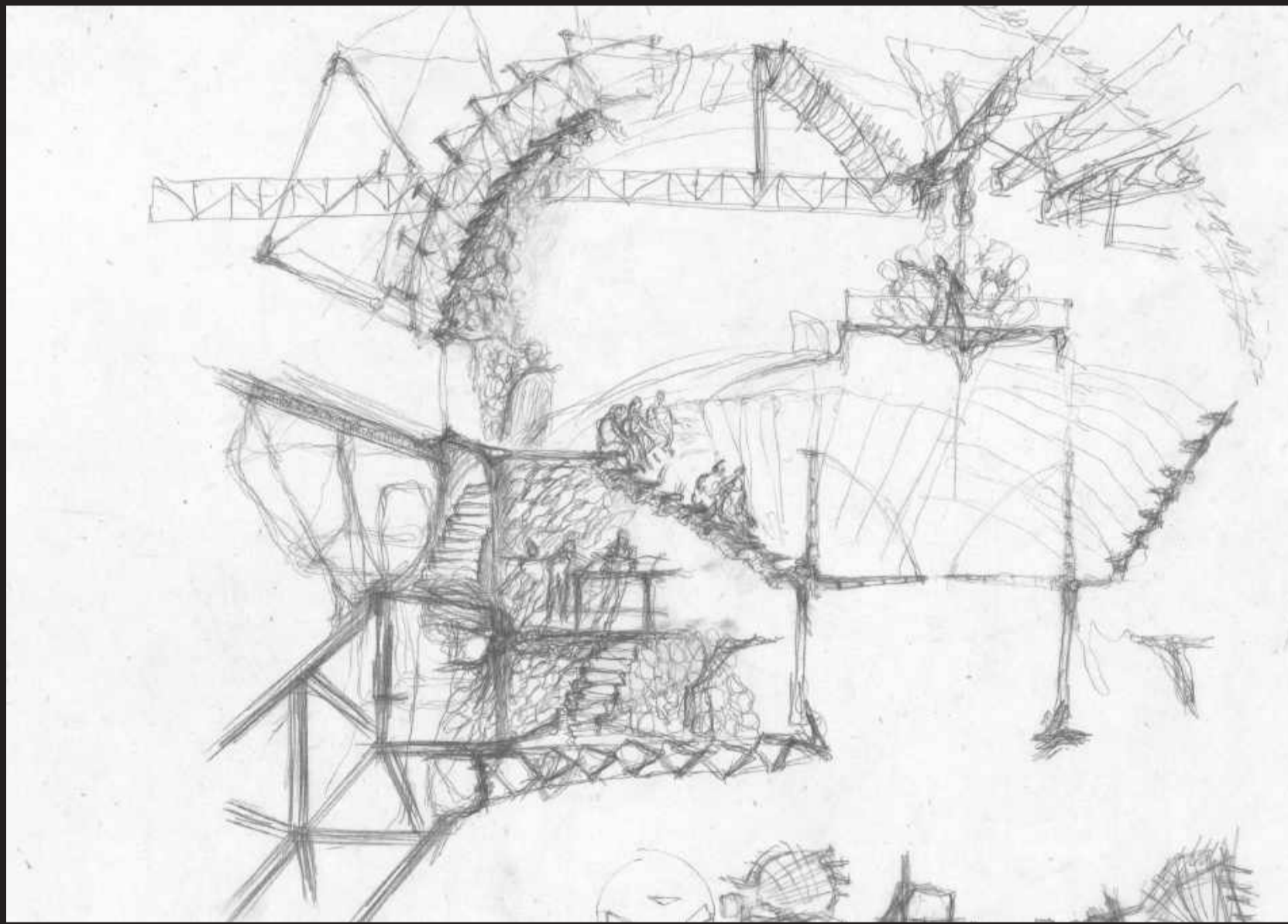
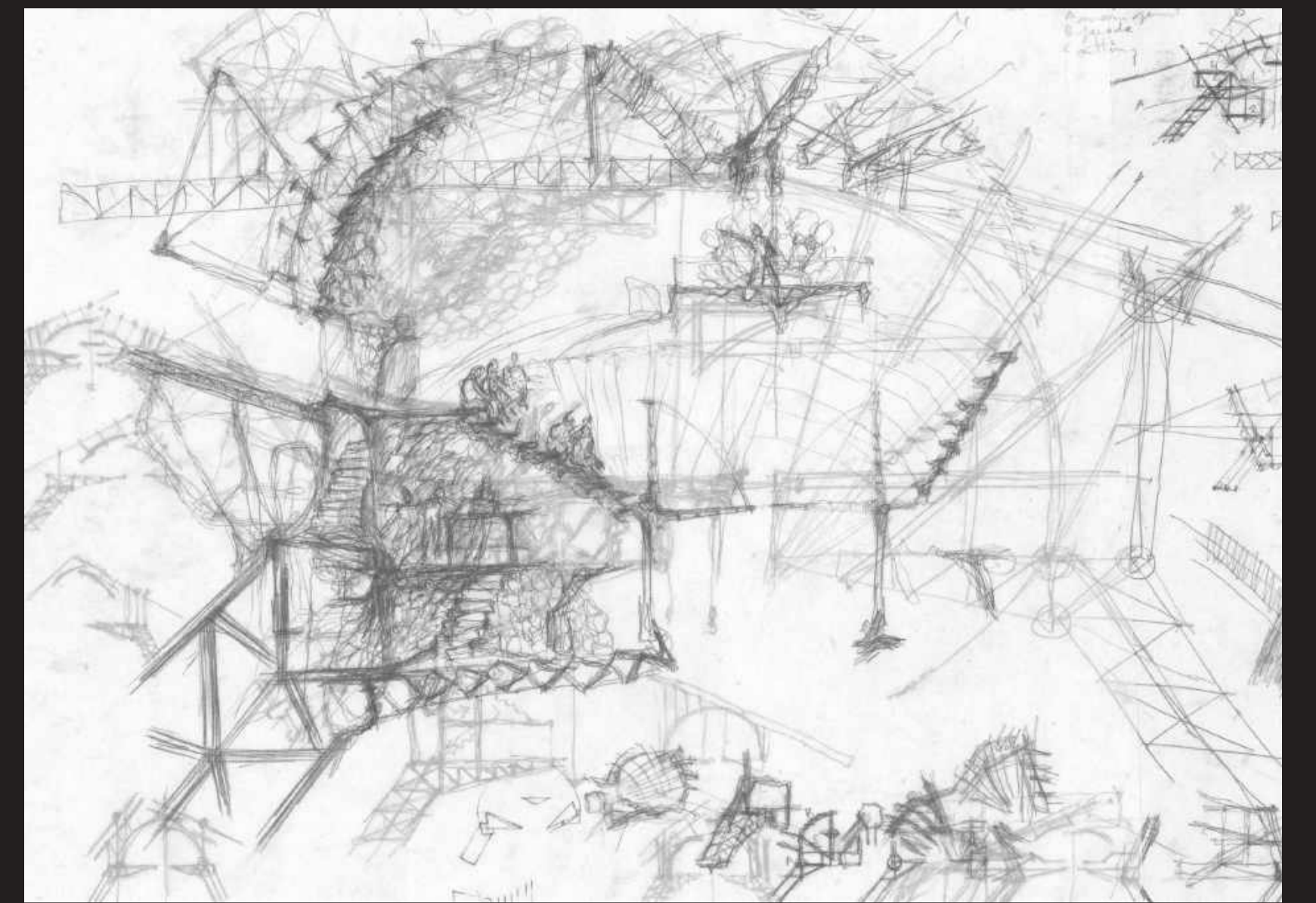
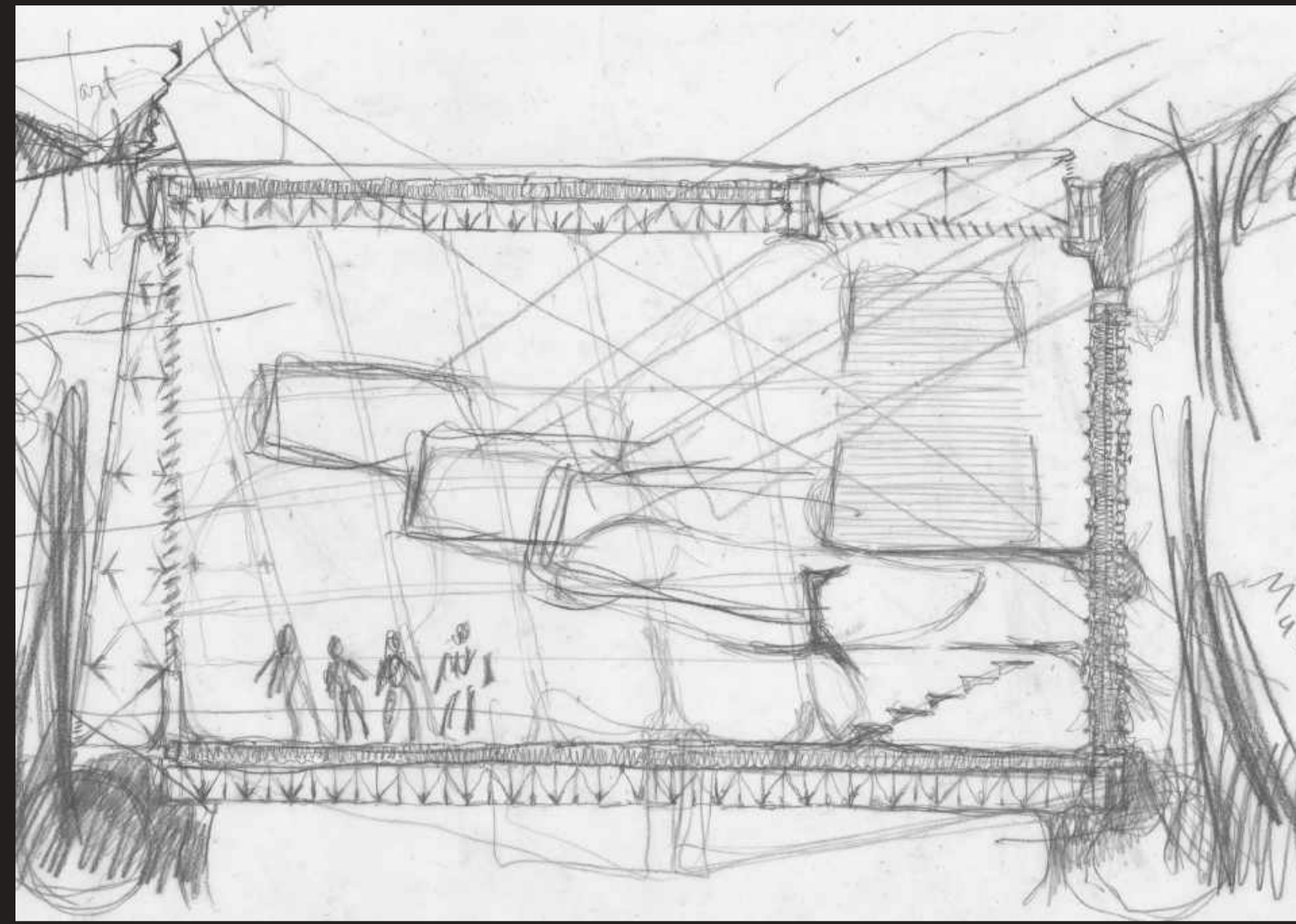
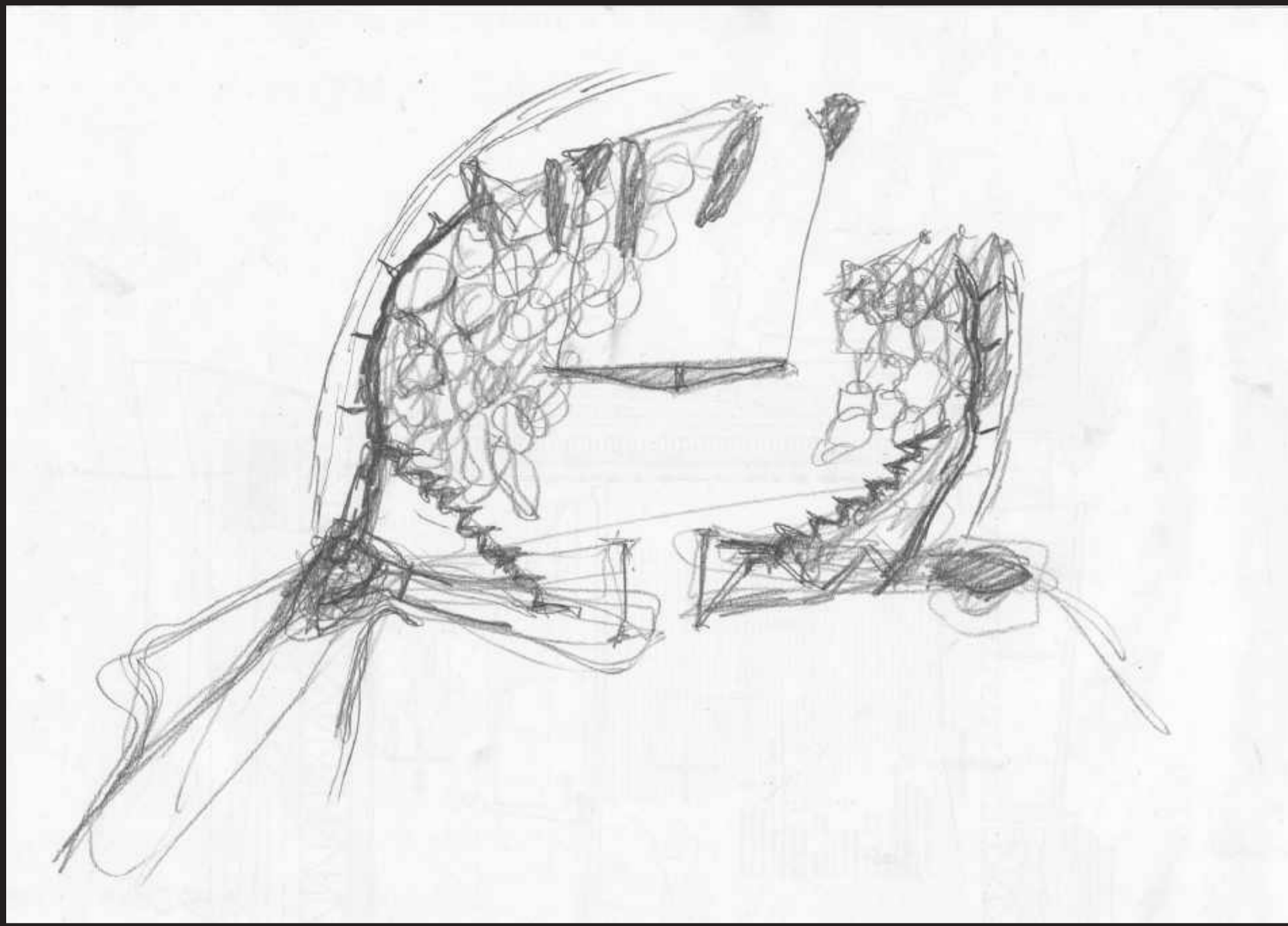


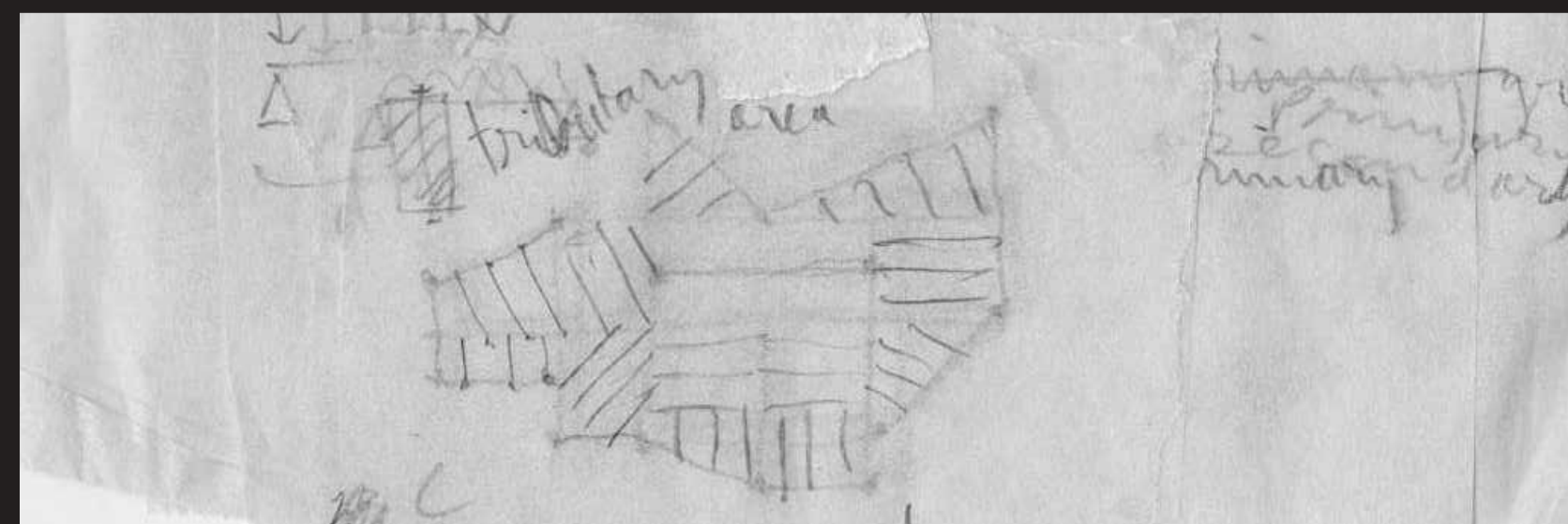
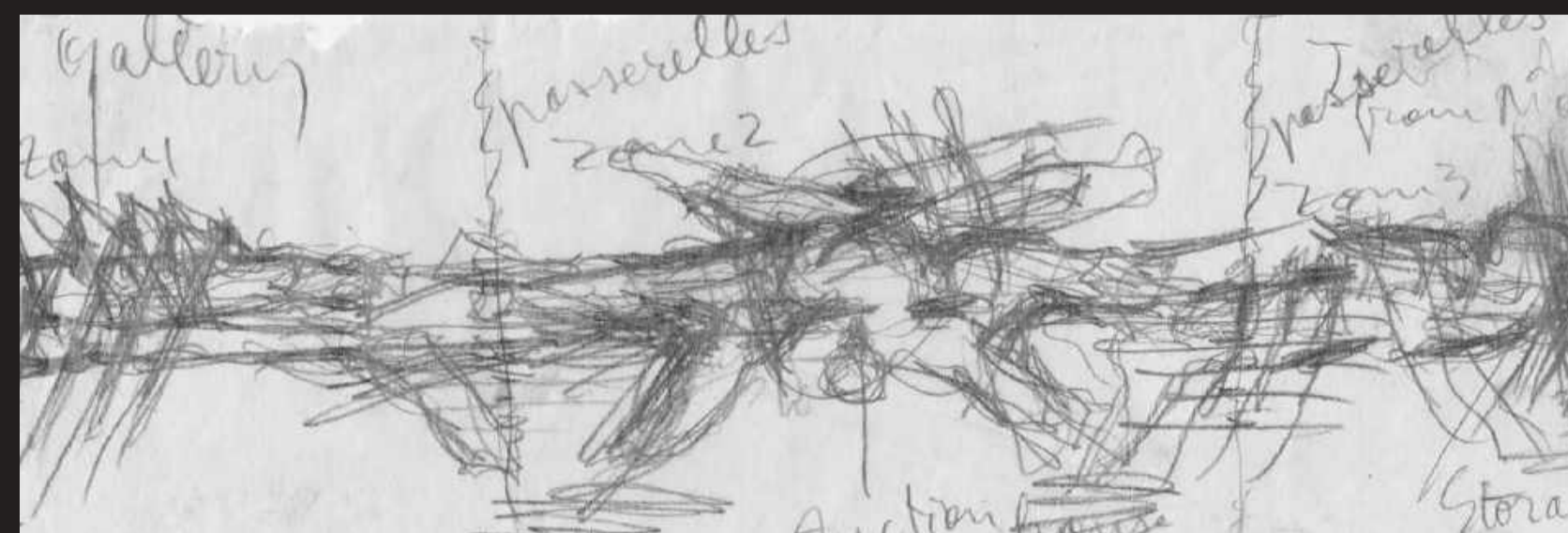
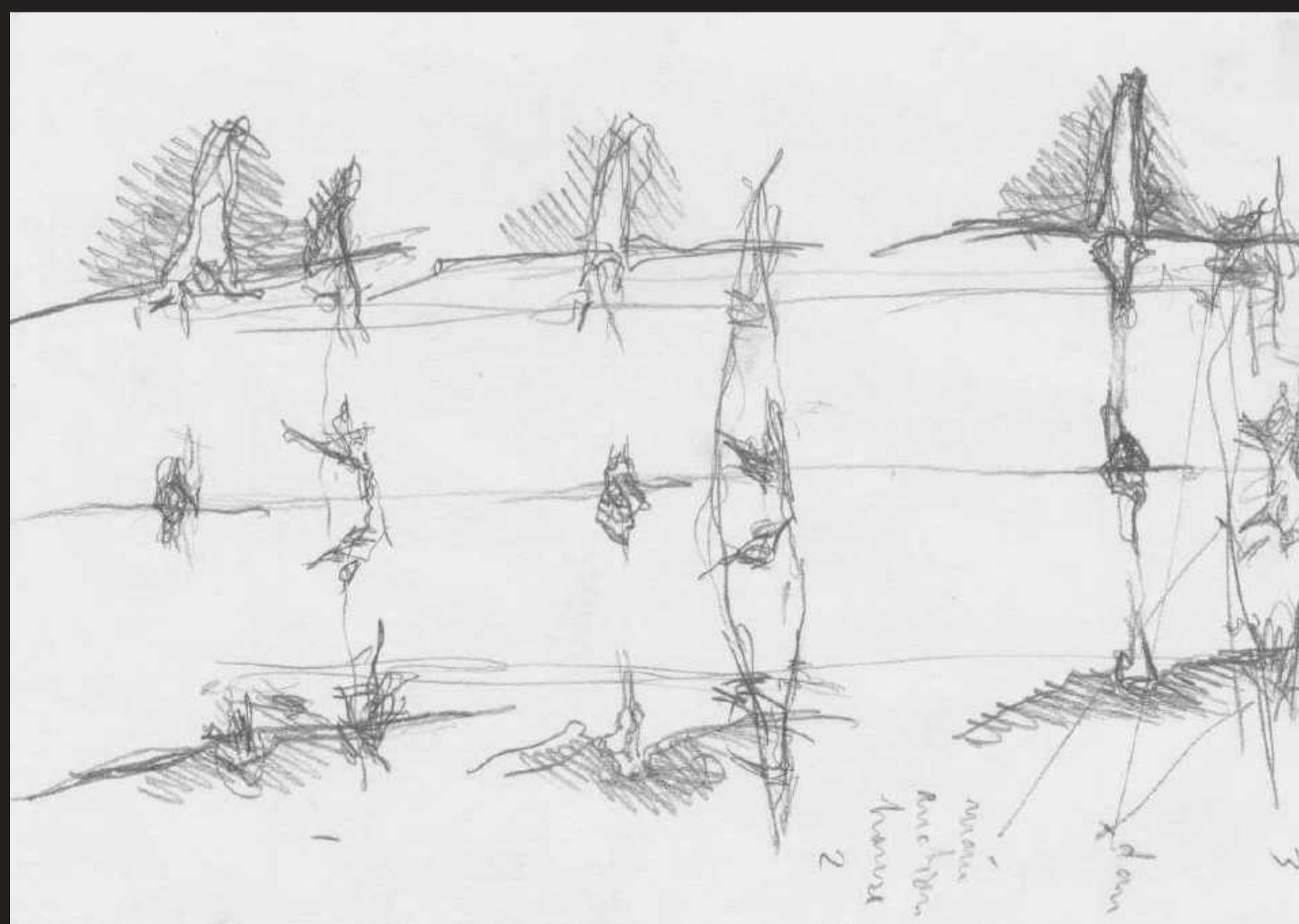
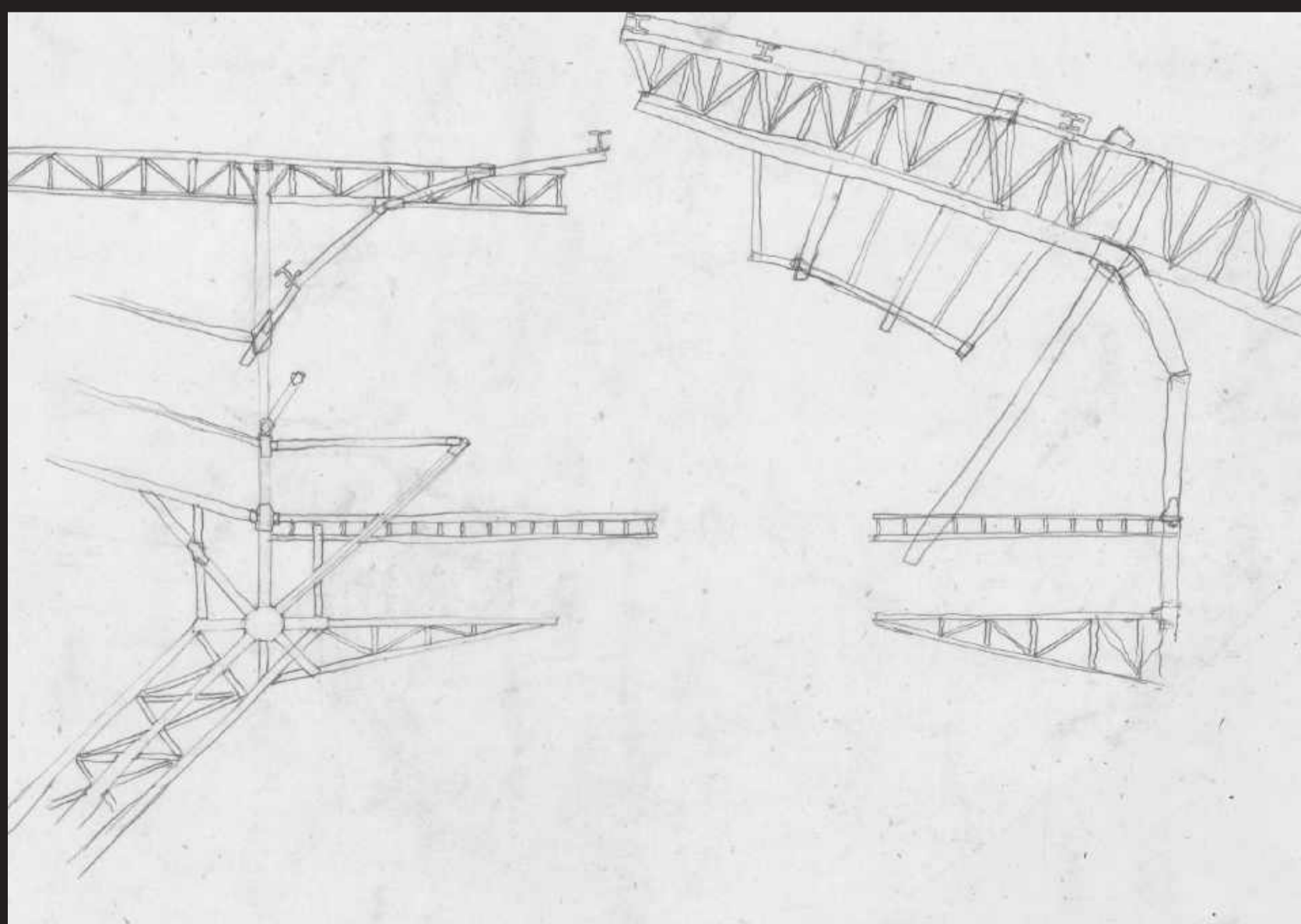
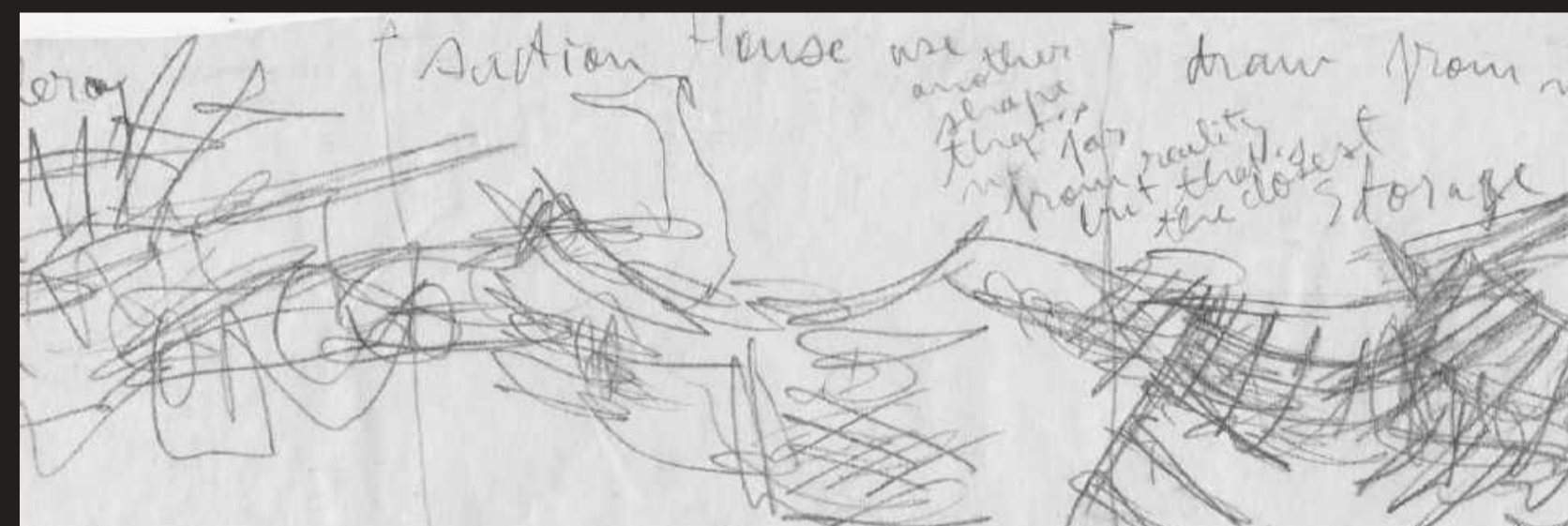
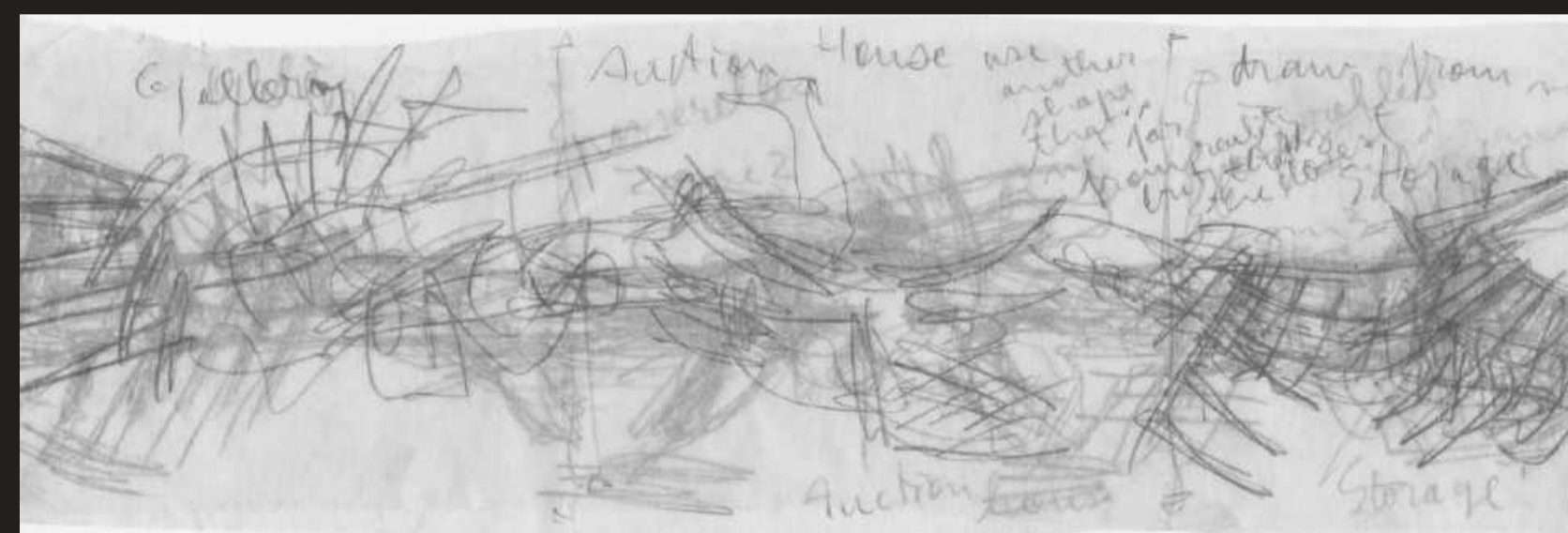
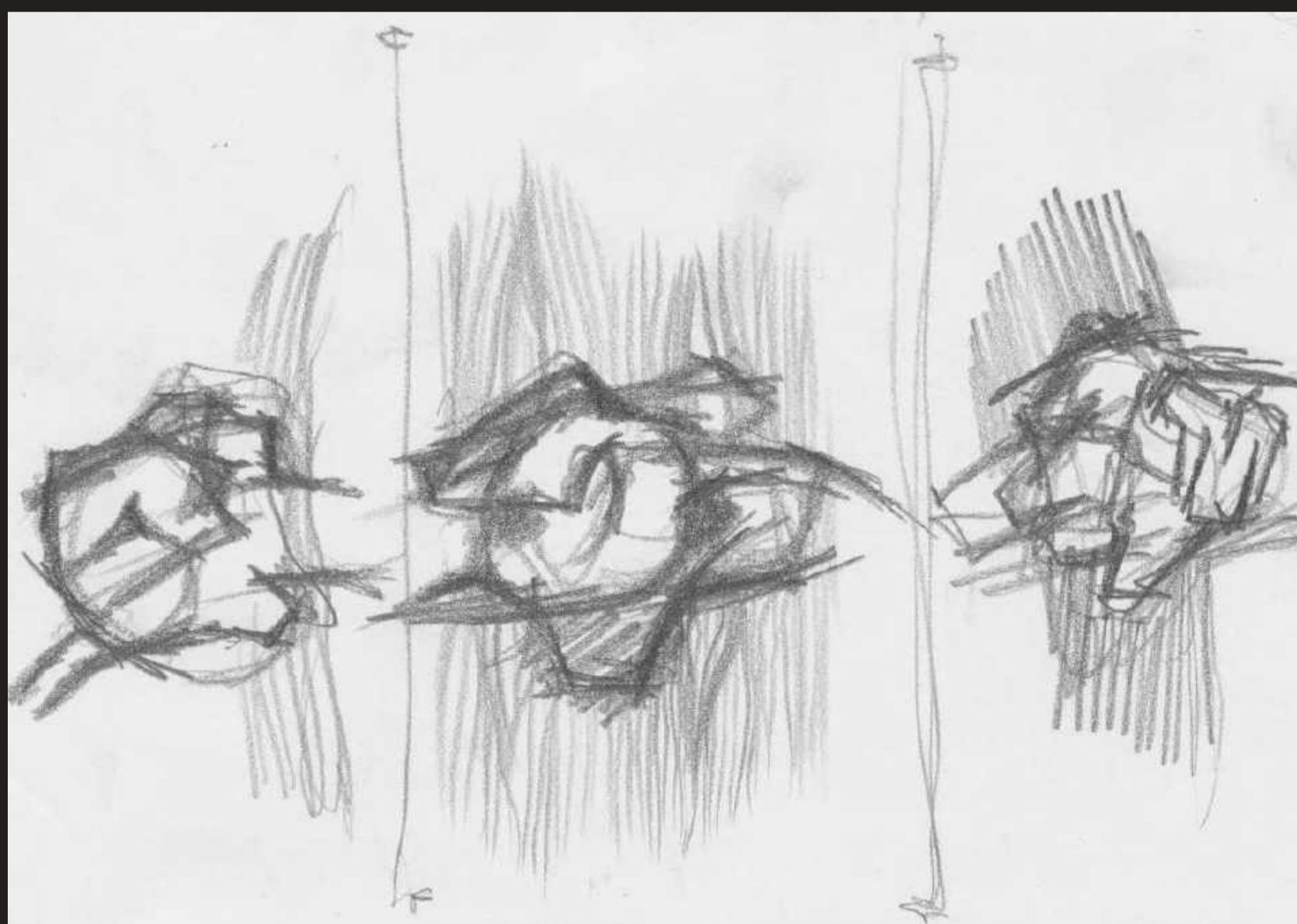
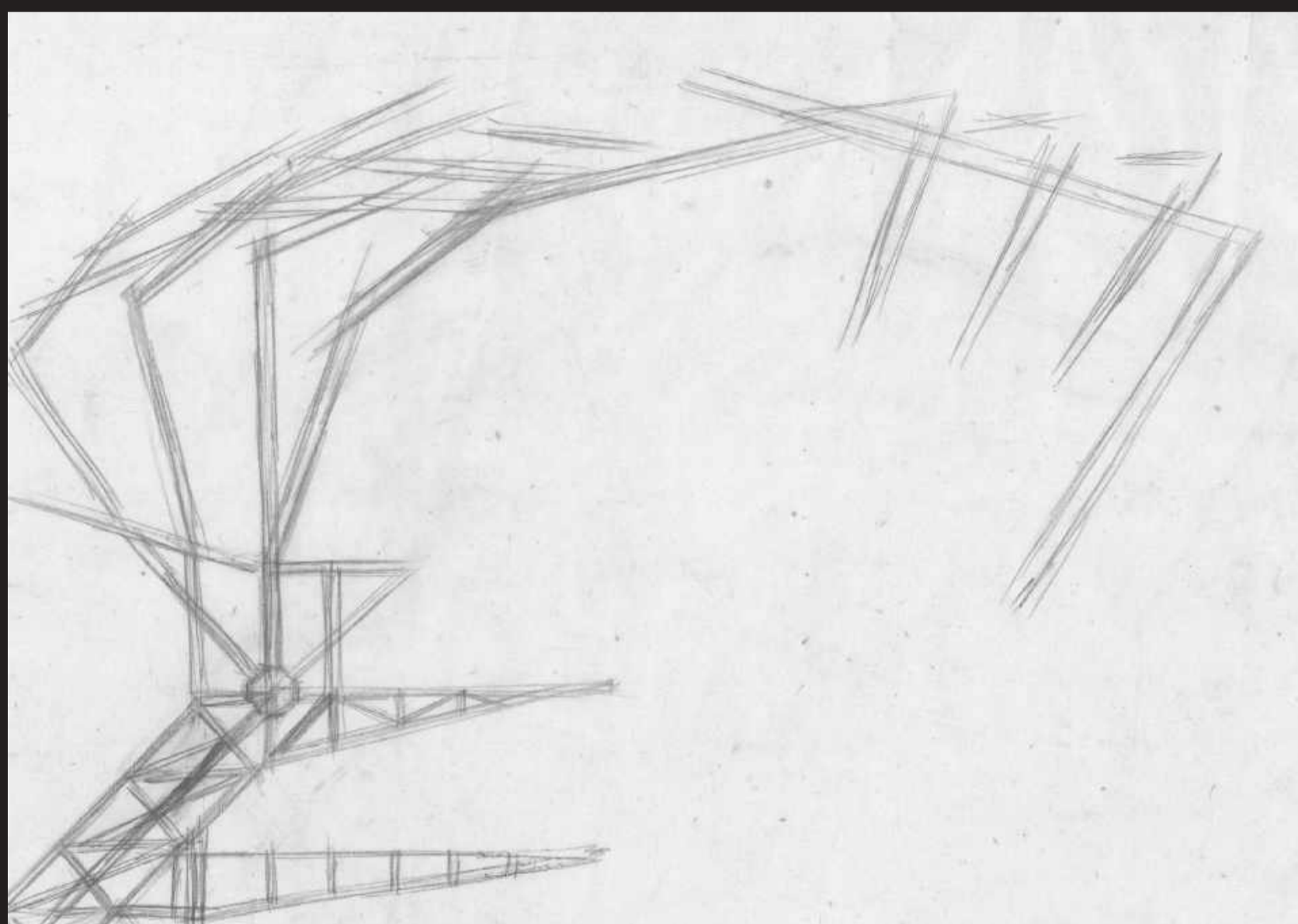
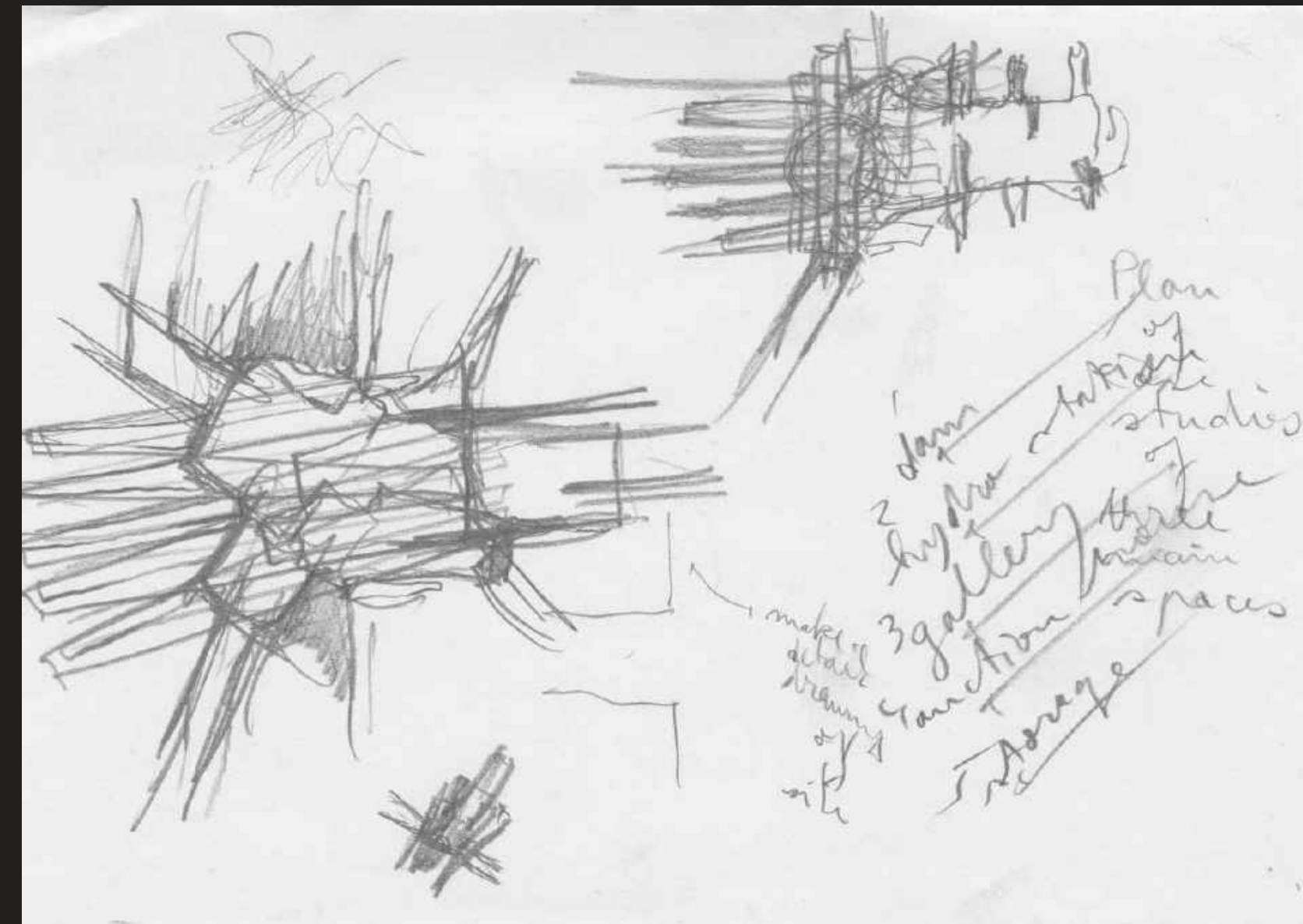
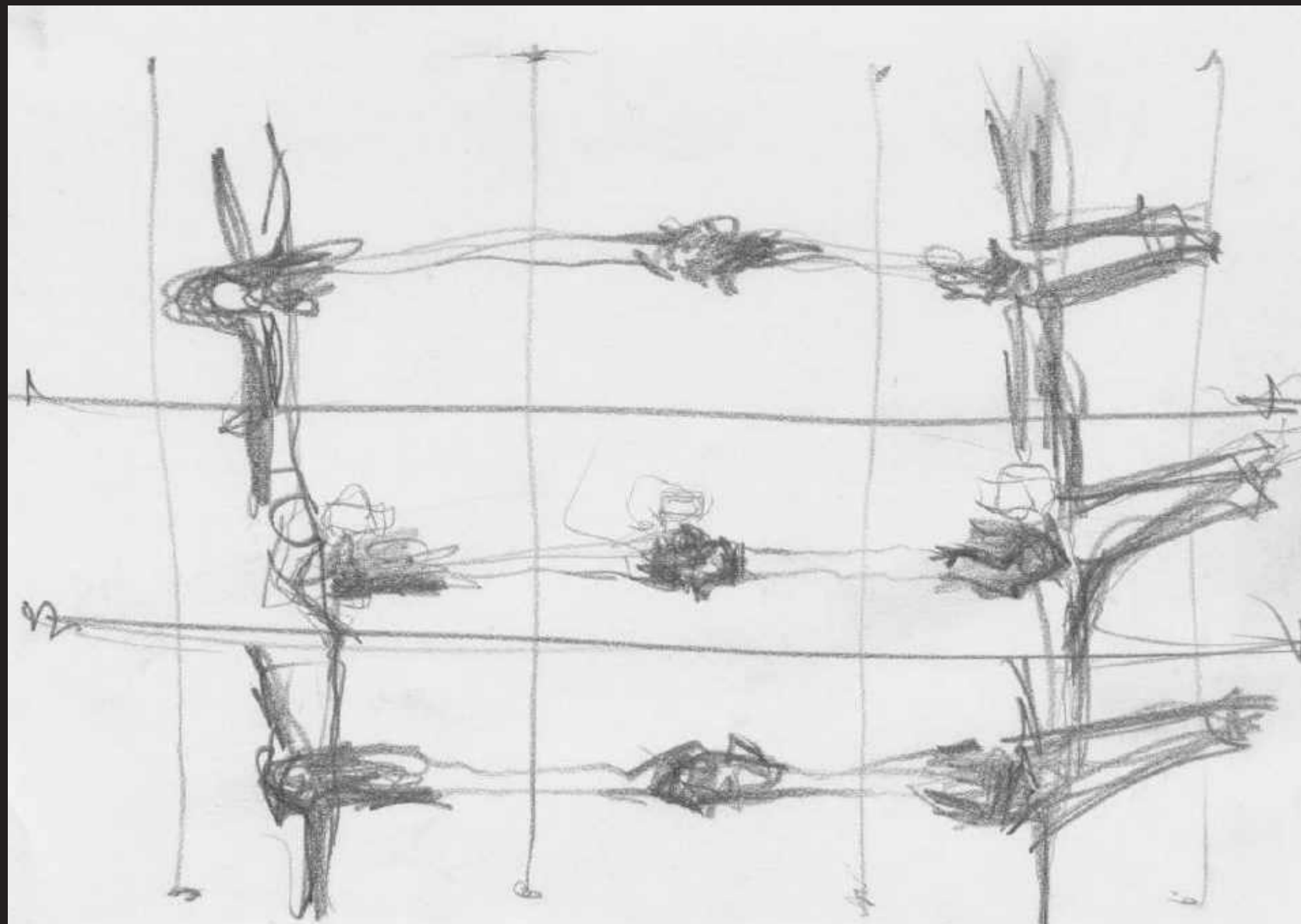
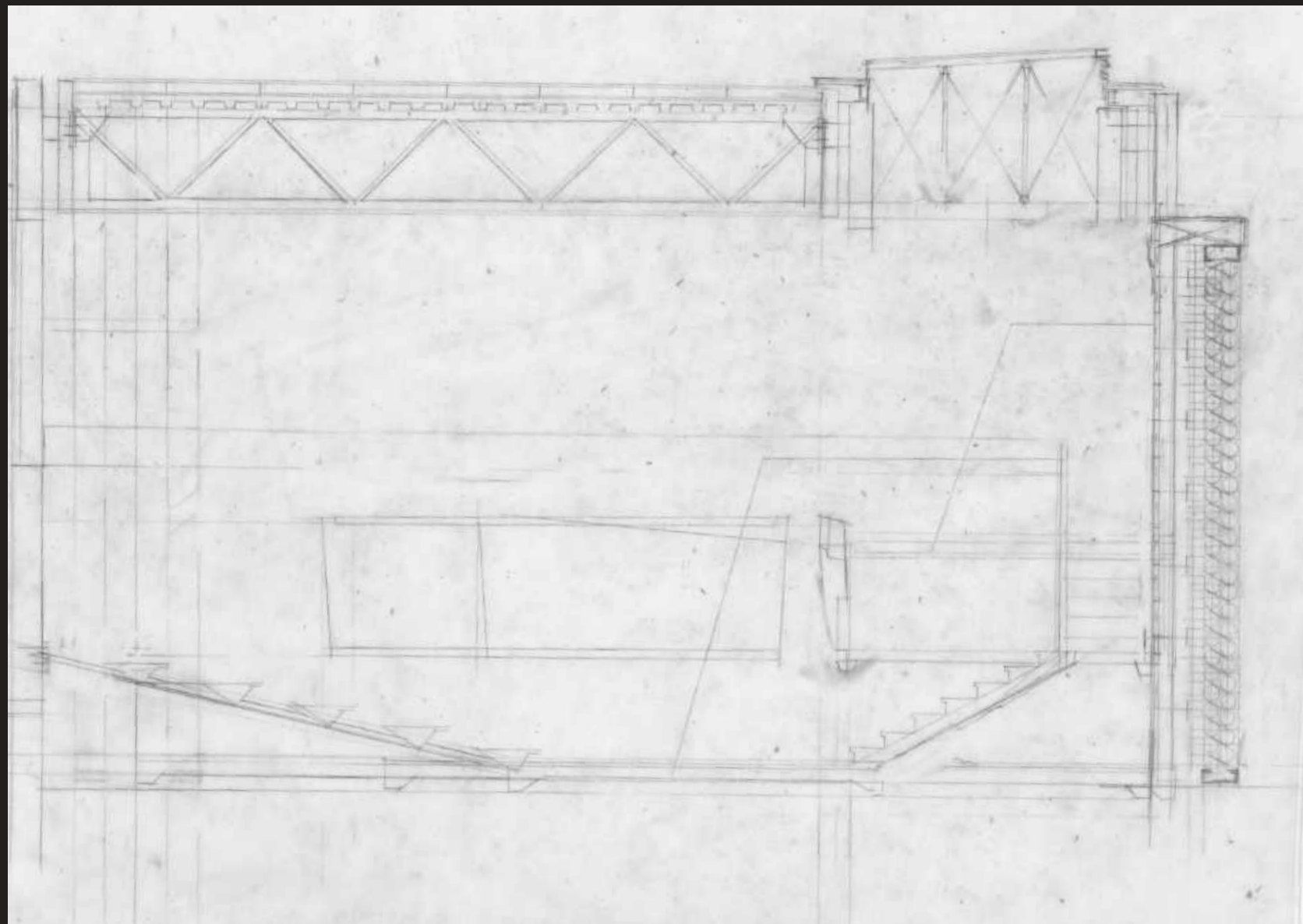


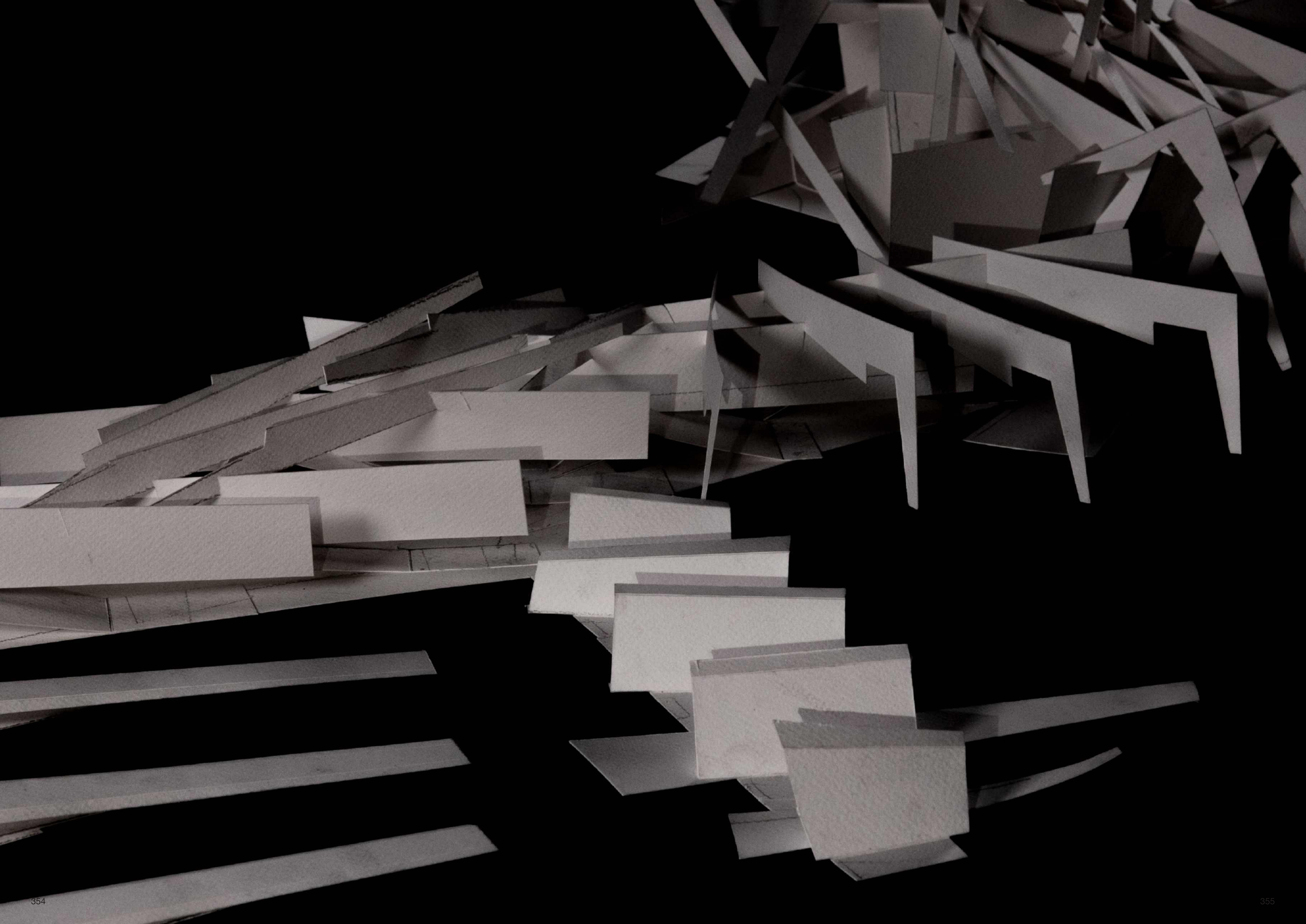


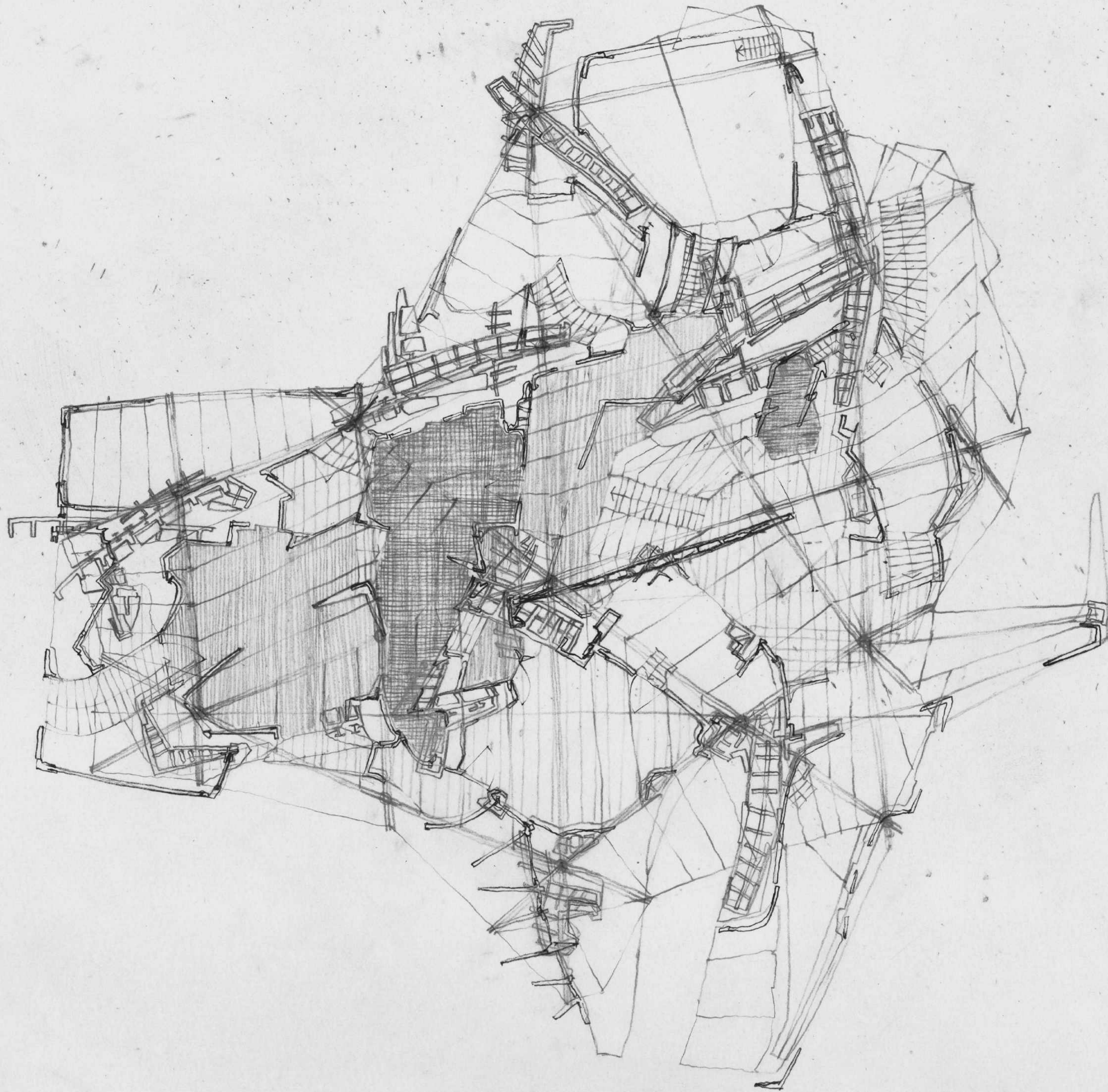


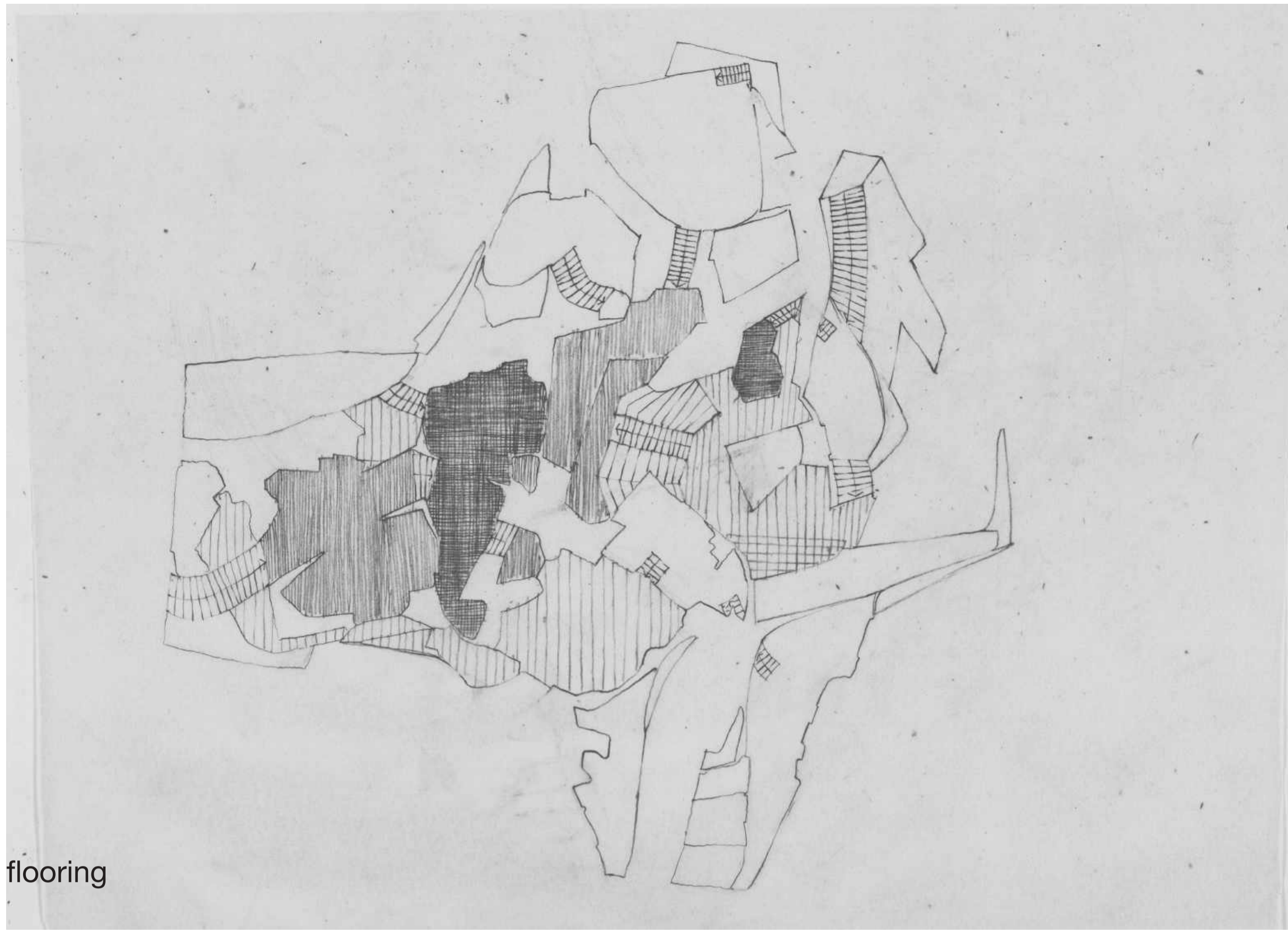








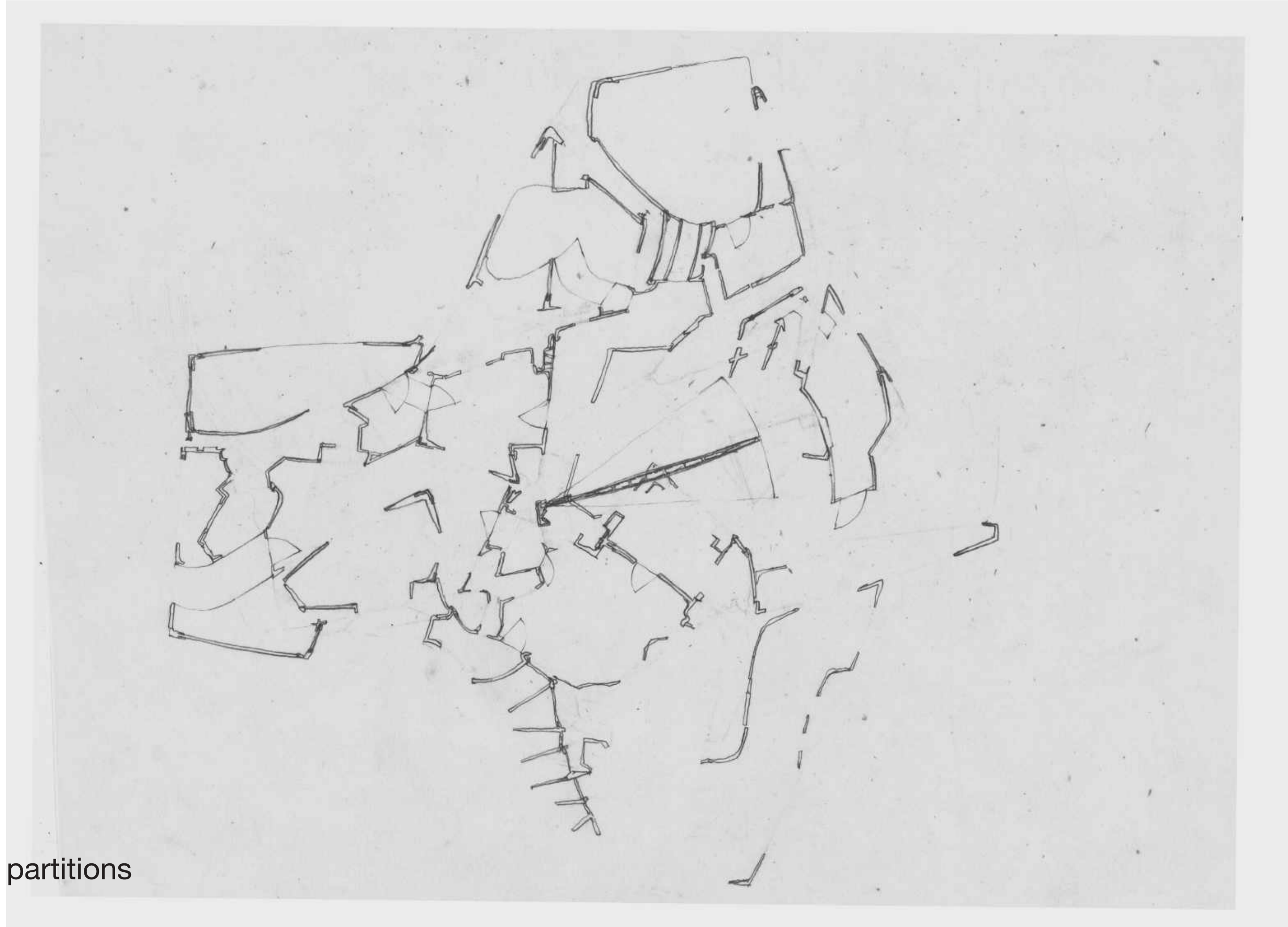




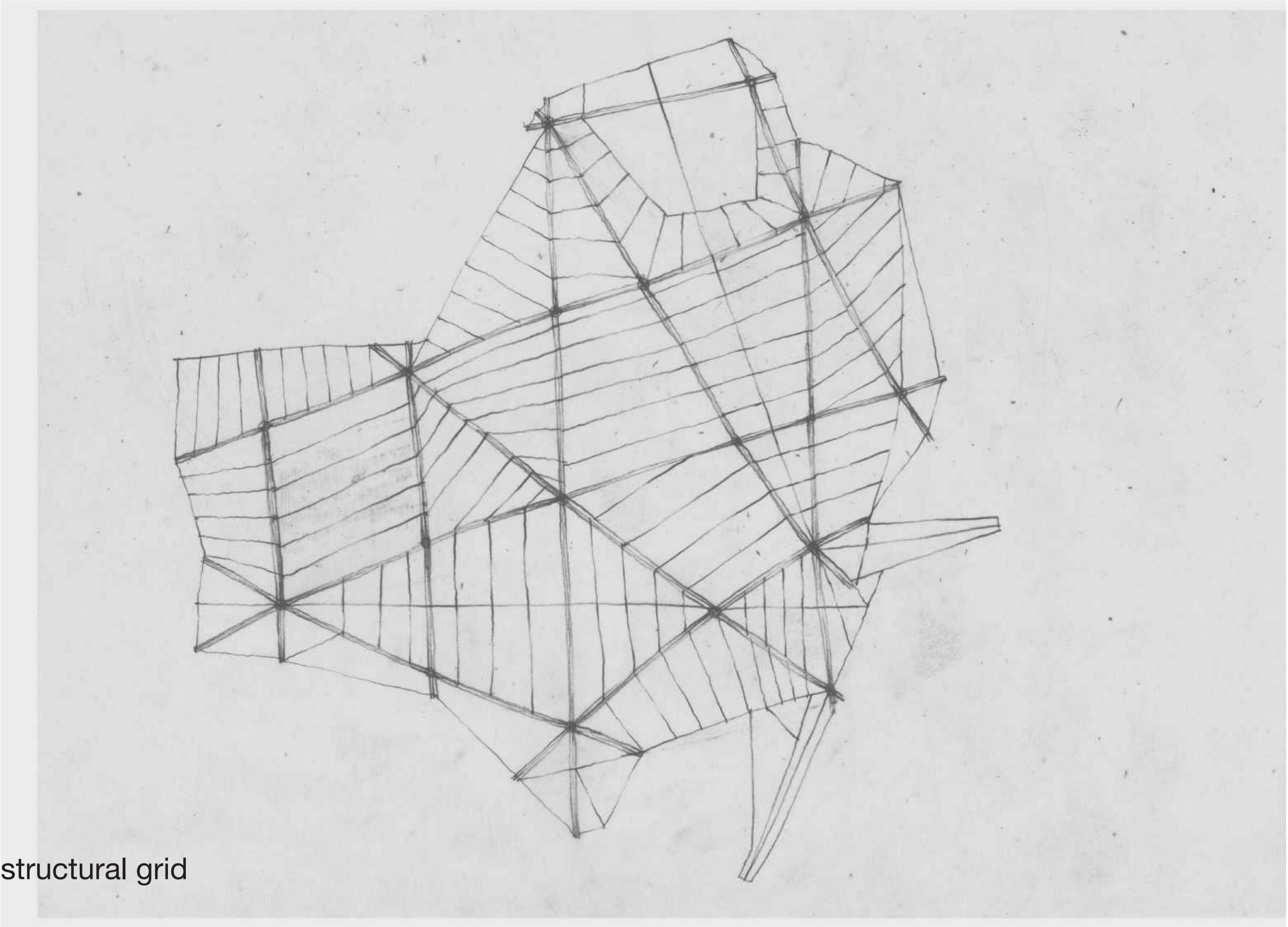
flooring



bearing walls

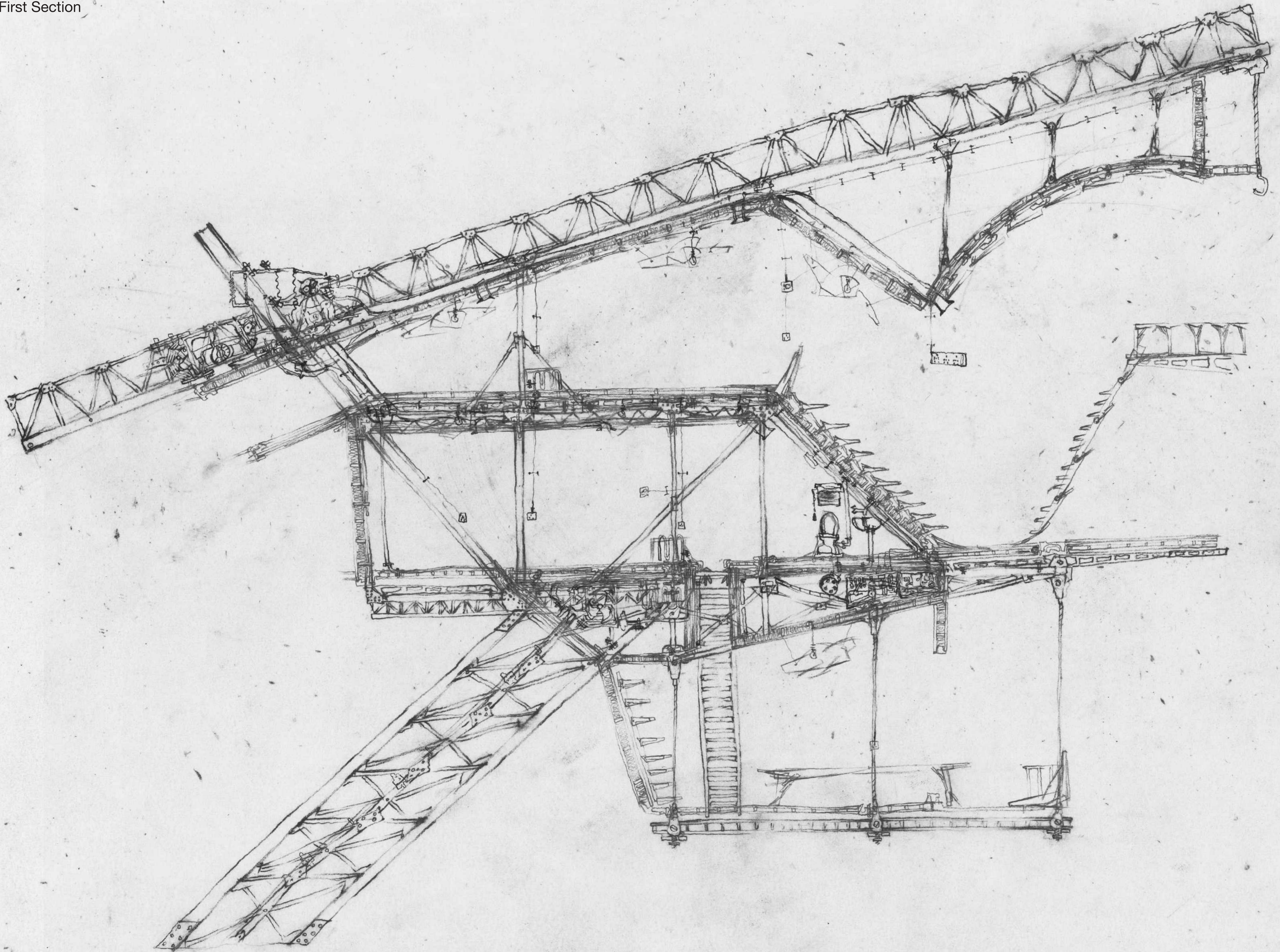


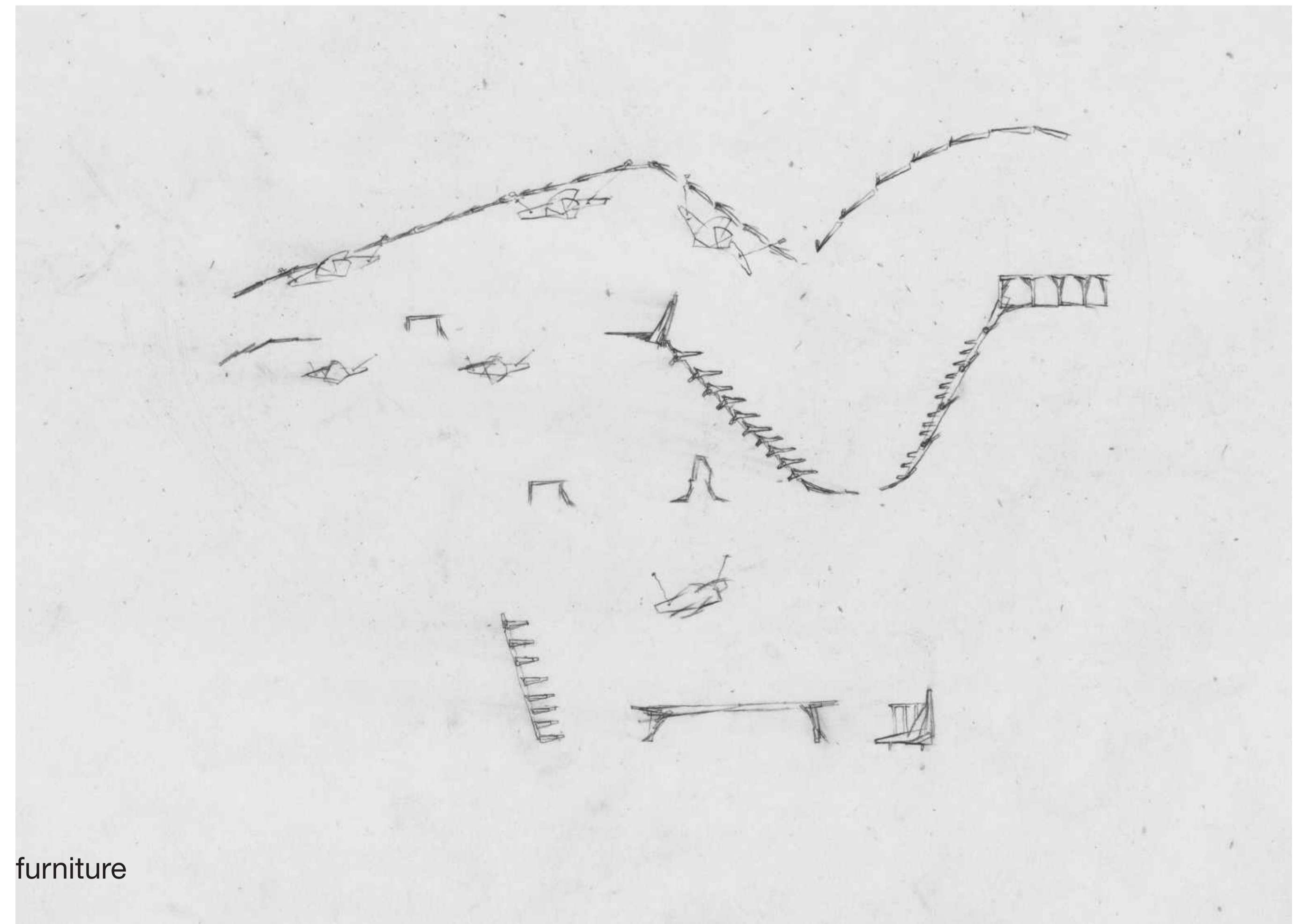
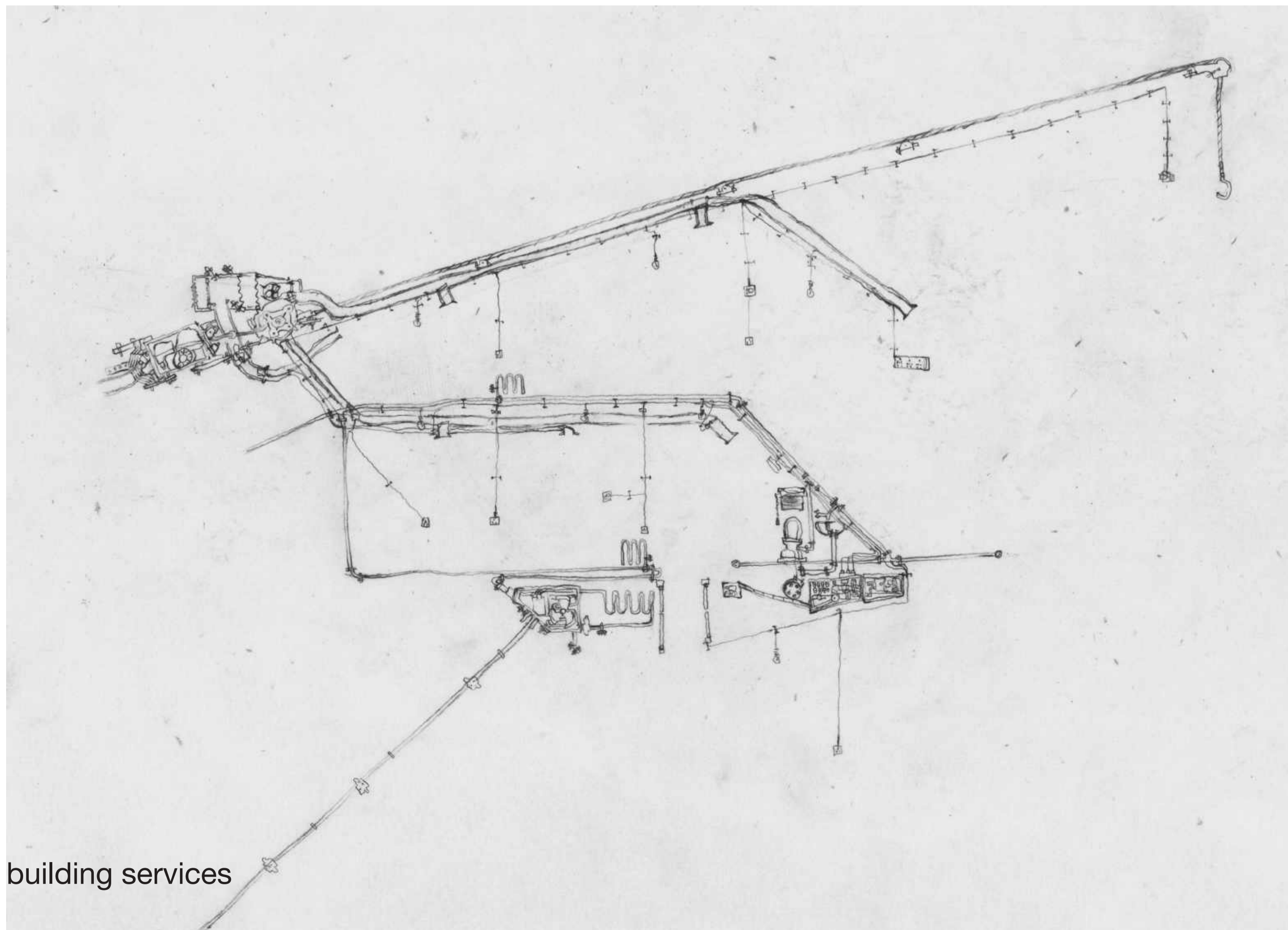
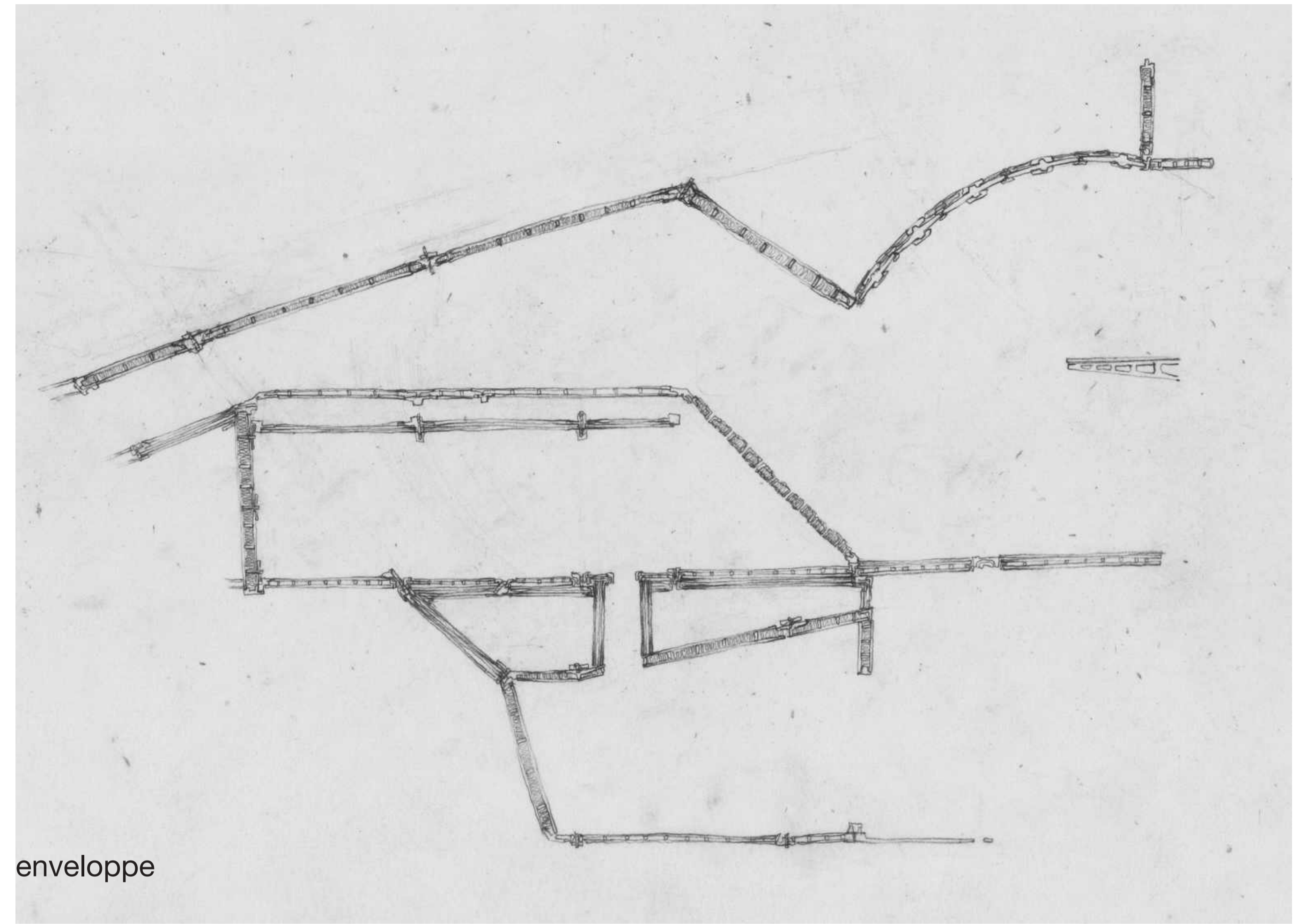
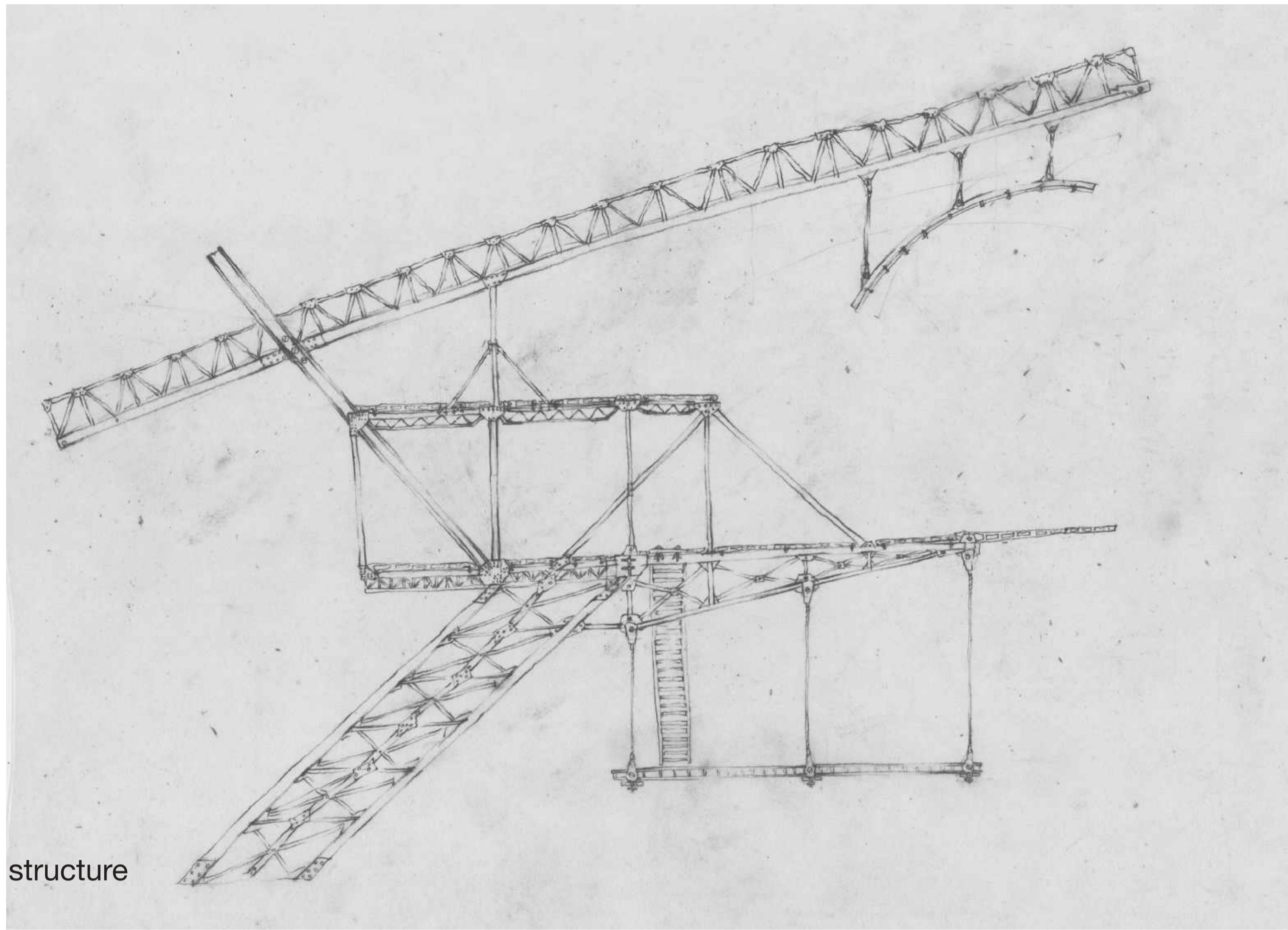
partitions



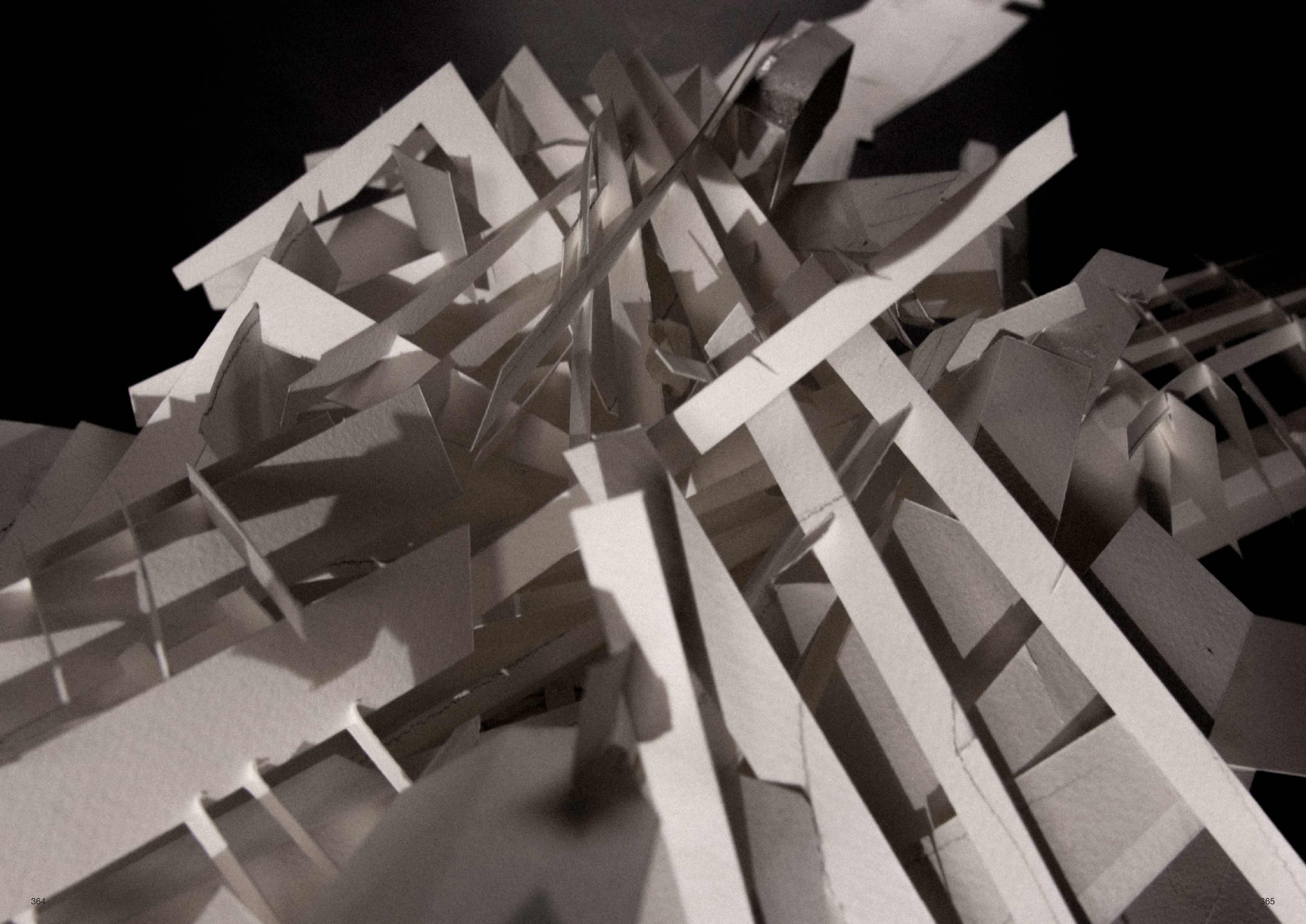
structural grid

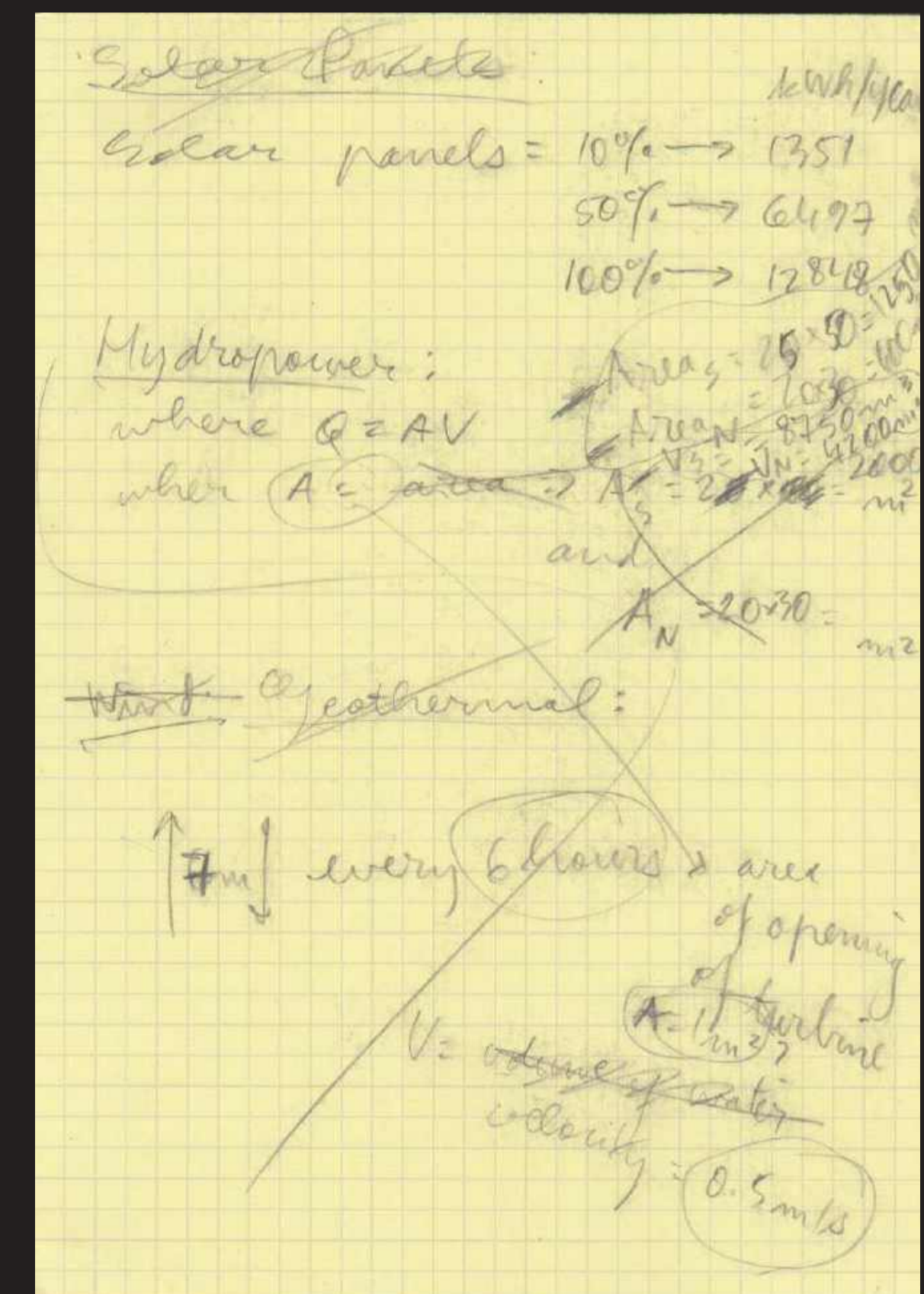
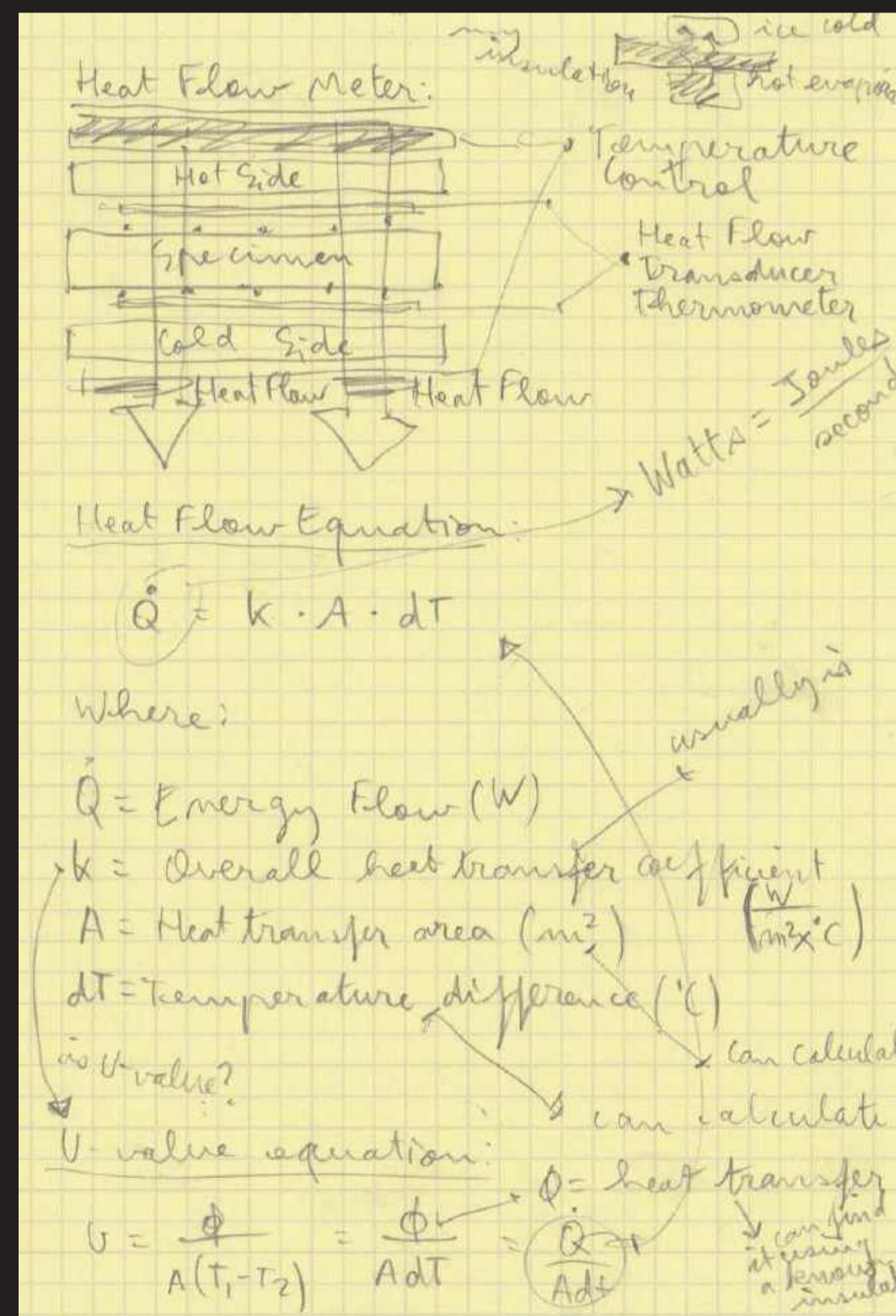
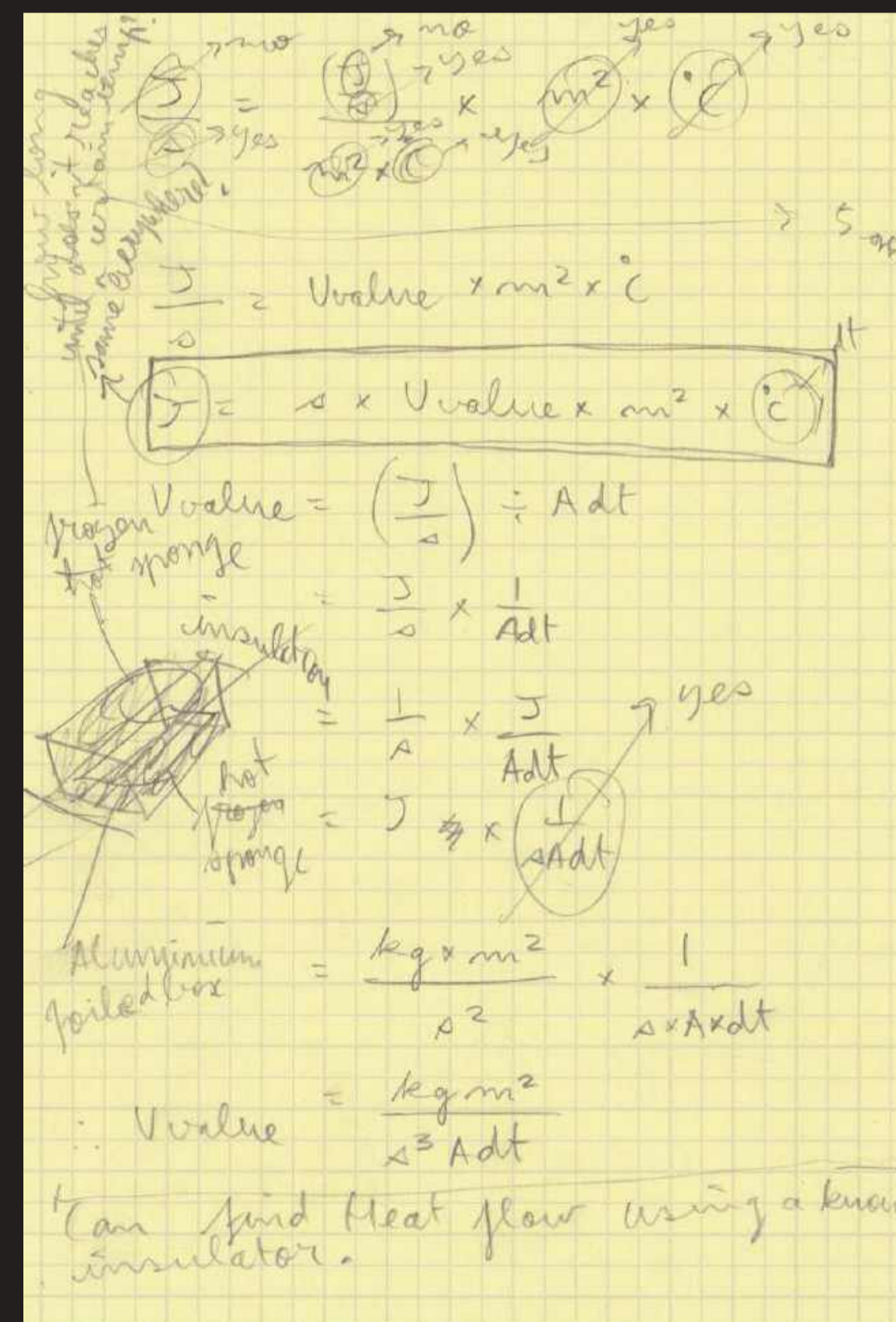
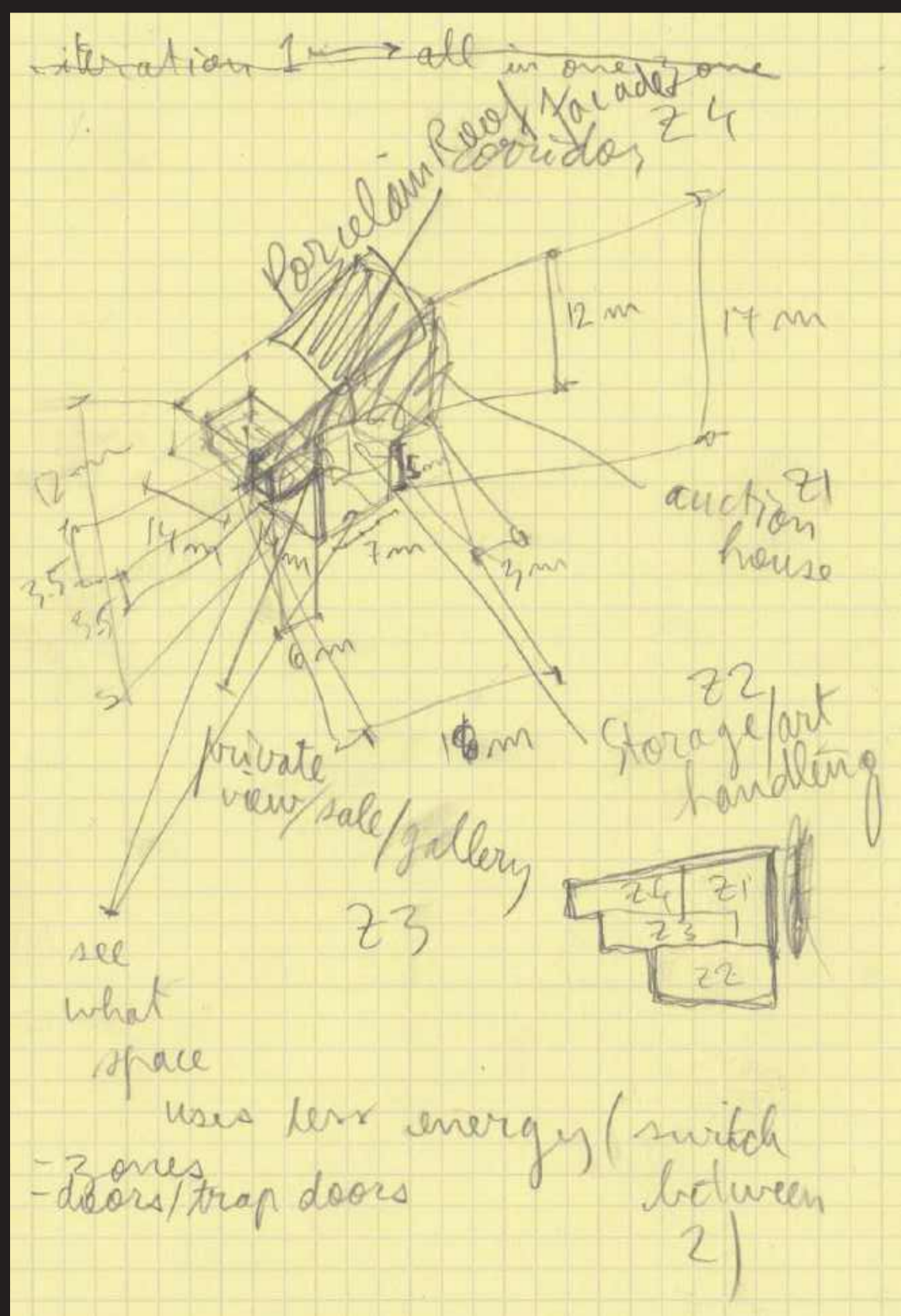
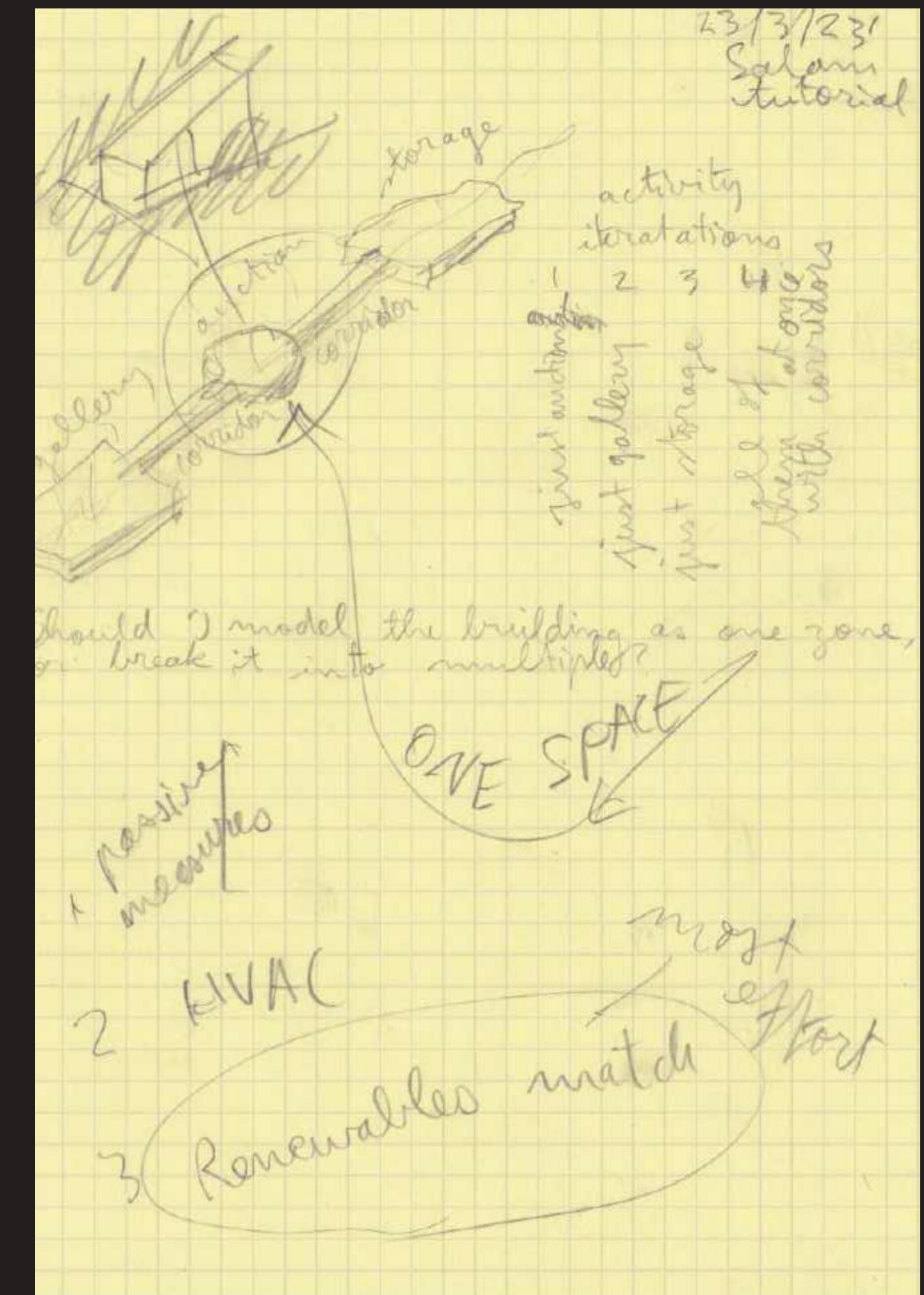
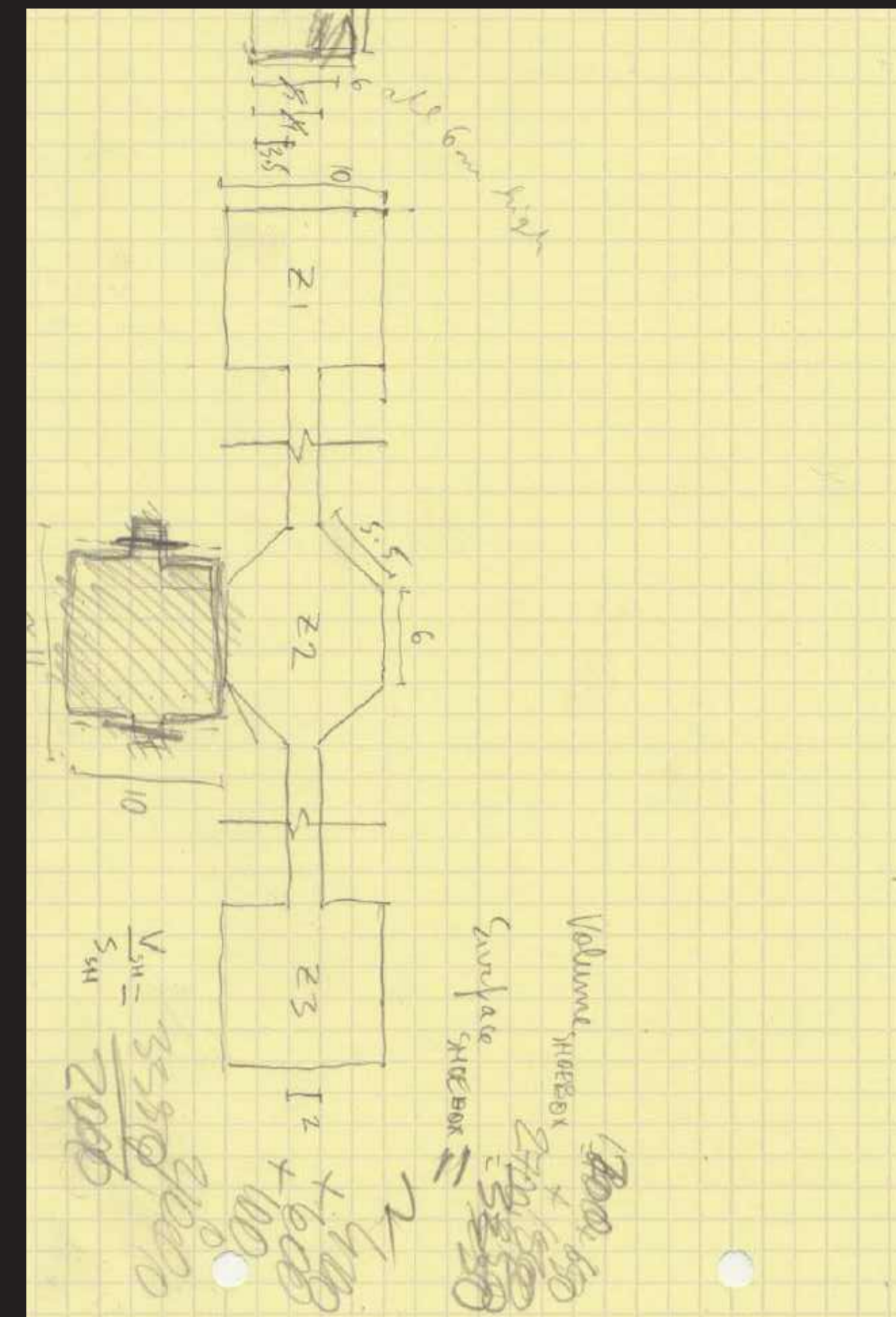
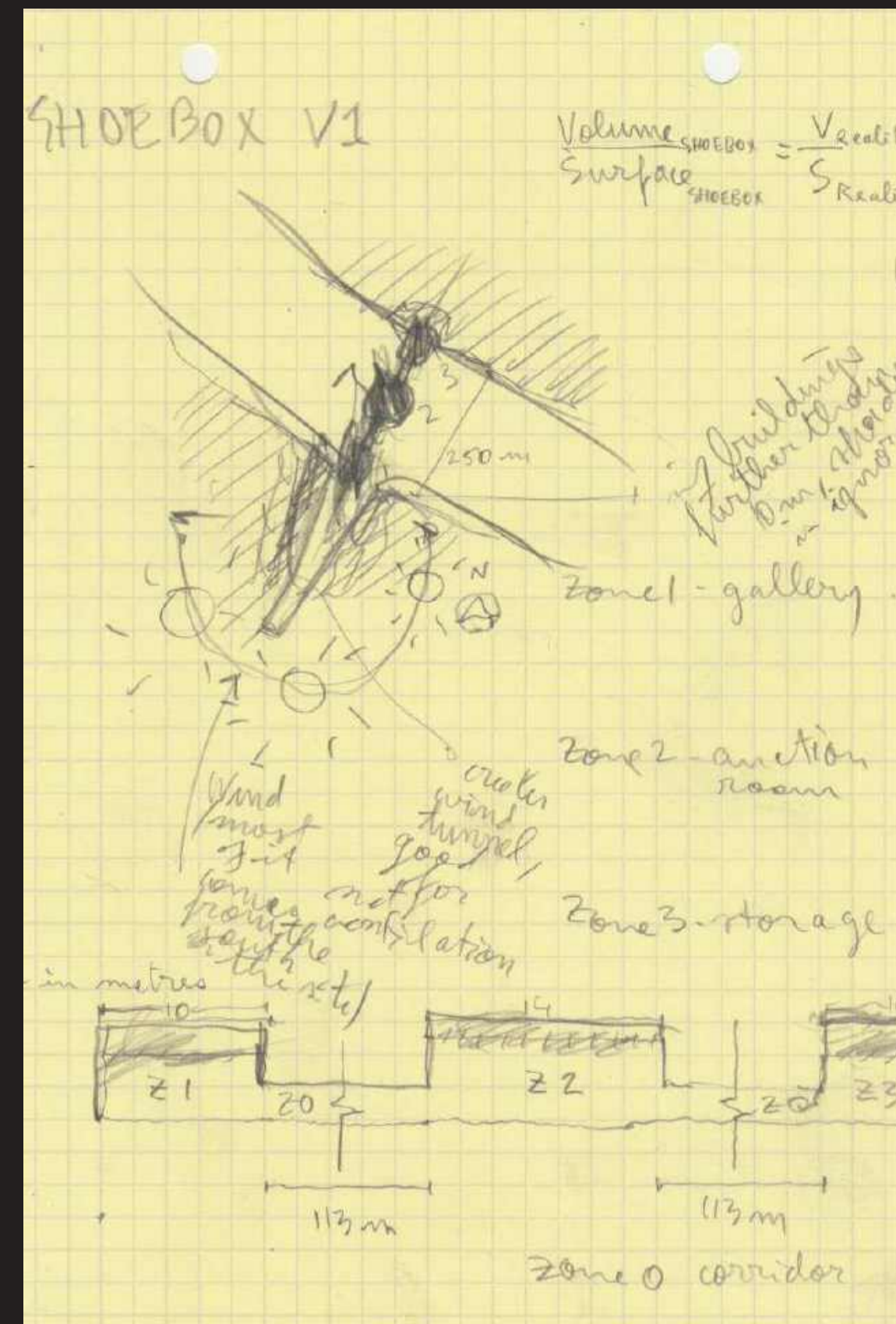
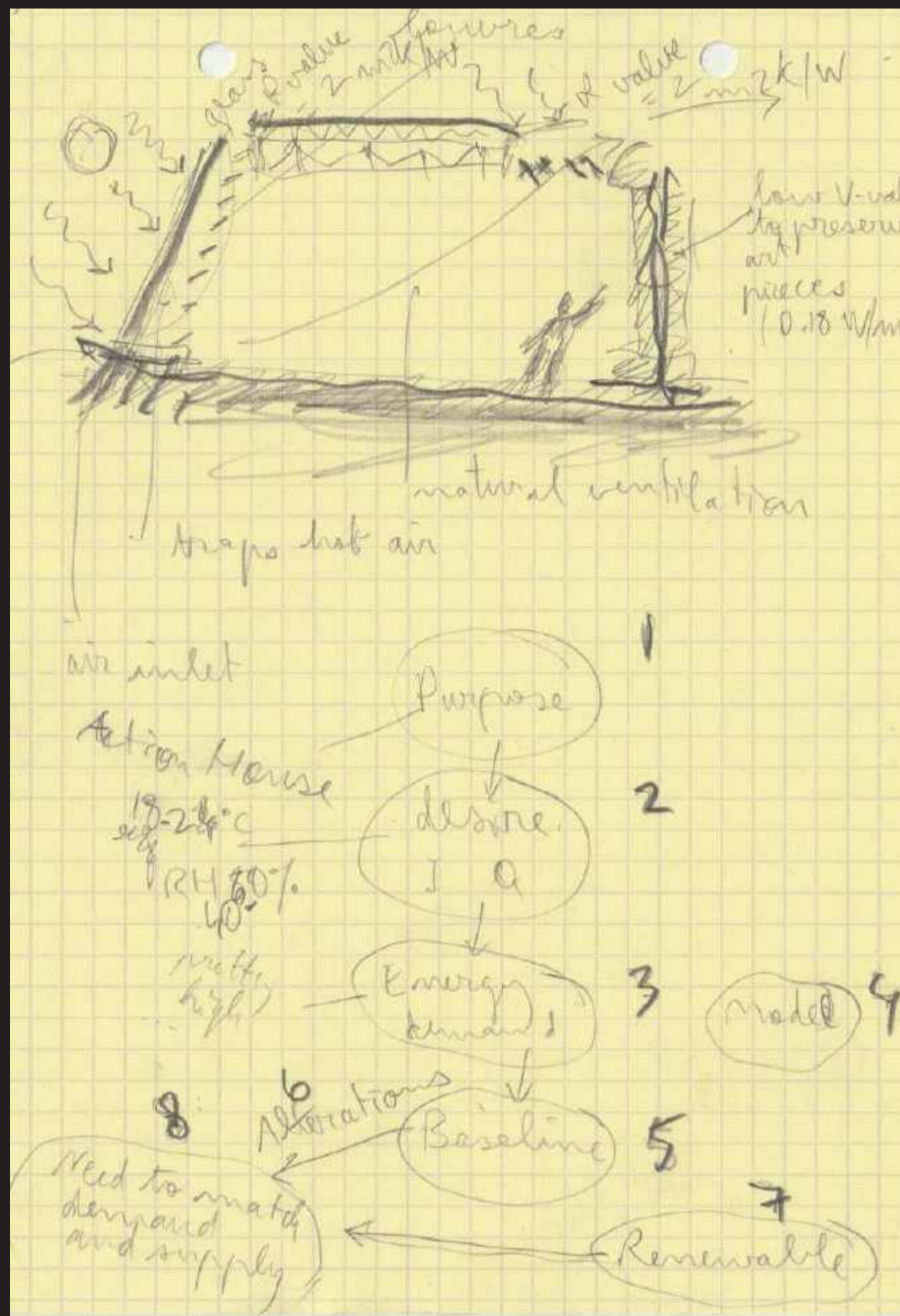
1:100 on A3





1:50 on A3





Hydropower: how to lower energy demands with ON-SITE RENEWABLES

Practical lighting

Reservoir
River
generator
turbine

Area 1
Area 2

$P = \eta \rho g h Q$
 where η = turbine efficiency = 90%
 ρ = water density = 1000 kg/m³
 g = gravity = 9.8 m/s²
 h = height of dam = 7m
 Q = discharge flow = 0.5 m³/s

$P = 0.9 \times 1000 \times 9.8 \times 7 \times 1 \times 0.5$

$\rho g h$ = potential energy per unit volume

frequency

capacity

$P = \eta \rho g h Q = 0.9 \times 1000 \times 9.8 \times 7$
 where $Q = AV$

$V = 0.5 \text{ m/s}$

80m
25m

$\times 4$ (in a day)

10 000
2500
2000
1500
1000

1/4
1/5
1/5

60
15
9
25
12

1/4
3/10
5/12
1/5

Hydropower
South dam:
 $P = 0.9 \times 1000 \times 9.8 \times 7 \times 1 \times 0.5$
 $= 30,870$
 $= 30.87 \text{ kW}$

0.5 m/s
0.9
litres per second

NORTH
 $\times 2$
 $\times 2$ in a day...
 $\therefore 124 \text{ kW/day}$

South
 $\times 2$
 $\therefore 62 \text{ kW/day}$

Renewables:
 Dams: 2981
 Solar: 6497
 Geothermal: 227
 TOTAL: 10205

62
24
 $\therefore 2.6 \text{ kWh/day}$
 $\therefore 5.16 \text{ kWh/day}$

365
 $\therefore 1886 \text{ kWh/yr}$

Detailed HVAC

GRIP water to water heated + chilled beams
 Heated floor etc...

Environmental:

11 - Passive measures (Baseline becomes Baseline 2)

13 - HVAC (Baseline 2 becomes Baseline 3 with drawings)

14 - Renewables (Baseline 3 then Hybrid)

* add sketches of strategies and diagrams of dam + geothermal etc...

South dam

25m 80m
h = 7m

capacity = 25 x 80 x 7 = 14000 m³

$Q = 0.5 \text{ m/s}$
 1 min $\rightarrow 300 \text{ m}^3$
 24h $\rightarrow 1800 \text{ m}^3$

Power = $\eta \rho g h Q$
 where $Q = AV$
 where A = outlet area
 and V = velocity of water

Power = $0.9 \times 1000 \times 9.8 \times 7 \times 1 \times 0.5$
 $= 30,870 = 30.87 \text{ kW}$

TOTAL in a day = 30.87 x 24 = 740.88 kWh

TURBINE TYPE = hydroelectric

North dam

25m 20m
h = 7m

capacity = 25 x 20 x 7 = 3500 m³

$V = 0.5$

Power = 22 kW
 in a day = 22 x 24 = 528 kWh
 in a year = 528 x 365 = 192,720 kWh

graphs needed:

total site energy
 discomfort hours
 energy break down
 baseline chart (5 baselines)

energy break down
 energy break down
 energy break down
 energy break down
 energy break down

SCAN

Blue yellow

5 Baselines test

Dam:

using the same water flow rate as the Thames

V = 0.5 m/s

with an outlet

A = 1 m²

and efficiency

n = 0.9

ρ = 1000 kg/m³

g = 9.8 m/s²

height of dam

h = 5 m

we get

$$P = n \cdot \rho \cdot g \cdot h \cdot V = 22050 \text{ W} = 22 \text{ kW}$$

if the dam is filled twice a day

it can discharge water for 4h

as the south dam has a bigger reservoir, it can discharge for 2h at a time, 4h in day and the north dam being 4 times smaller, can discharge for 1h a day

therefore the south dam generates:

$$P_{\text{south}} = 22 \cdot 4 \cdot 365 = 32120 \text{ kWh/year}$$

$$P_{\text{north}} = 22 \cdot 1 \cdot 365 = 8030 \text{ kWh/year}$$

in total it generates

40150 kWh/year

Geothermal

three different types of loops were tried

it seemed that the closed loop horizontal was the best one

Solar Power:

<https://www.fmb.org.uk/homepicks/solar-panels/best-solar-panels/>
has good charts about average kWh for angle and placement in UK

$$W \text{ rating} \cdot \text{hours of sunlight per day} \cdot \text{efficiency} = \text{kWh / day}$$

- adds a lot of weight!! calculate weight
think about angle and position

calculating average sunlight in the UK:

<https://britishbusinessenergy.co.uk/blog/sunshine-duration-the-sunniest-cities-in-the-uk-and-europe/>

- average sunlight in UK = 4.1 hours/day

- average sunlight in London = 1410 hours/day = 3.8 hours/day

<https://www.architecturaldigest.com/reviews/en-gb/solar-panels/sunpower-maxeon-3-solar-panels-review>

SunPower Maxeon is popular in the UK

- 350 pounds (more expensive, but better warranties)

- 430W output

- 22.7% efficiency

- 98% output during the first year, then 0.25% degradation per year (lowest on the market)

- weight = 21.2 kg

- area = 1812 mm x 1046 mm = 1.90 m²

□

CALCULATIONS

$$W \text{ rating} \cdot \text{hours of sunlight per day} \cdot \text{efficiency} = \text{kWh / day}$$

$$430 \text{ W} \cdot 3.8 \text{ hours/day} \cdot 0.227 = 370.92 \text{ Wh/day/ panel}$$

Our roof = 181 m²

area of one solar panel = 1.90 m²

one solar panel = 0.37 kWh/day

and this is 135 kWh/year

approximately, this is 0.185 kWh/m²/day

10% of roof = 18.1 m² = 10 solar panels = 3.70 kWh/day

- cost = (3500 pounds)

- weight = 212 kg

total kWh/year = 1350.5

50% of roof = 90.5 m² = 48 solar panels = 17.8 kWh/day

- cost = (16800 pounds)

- weight = 1,018 kg = 6497

total kWh/year

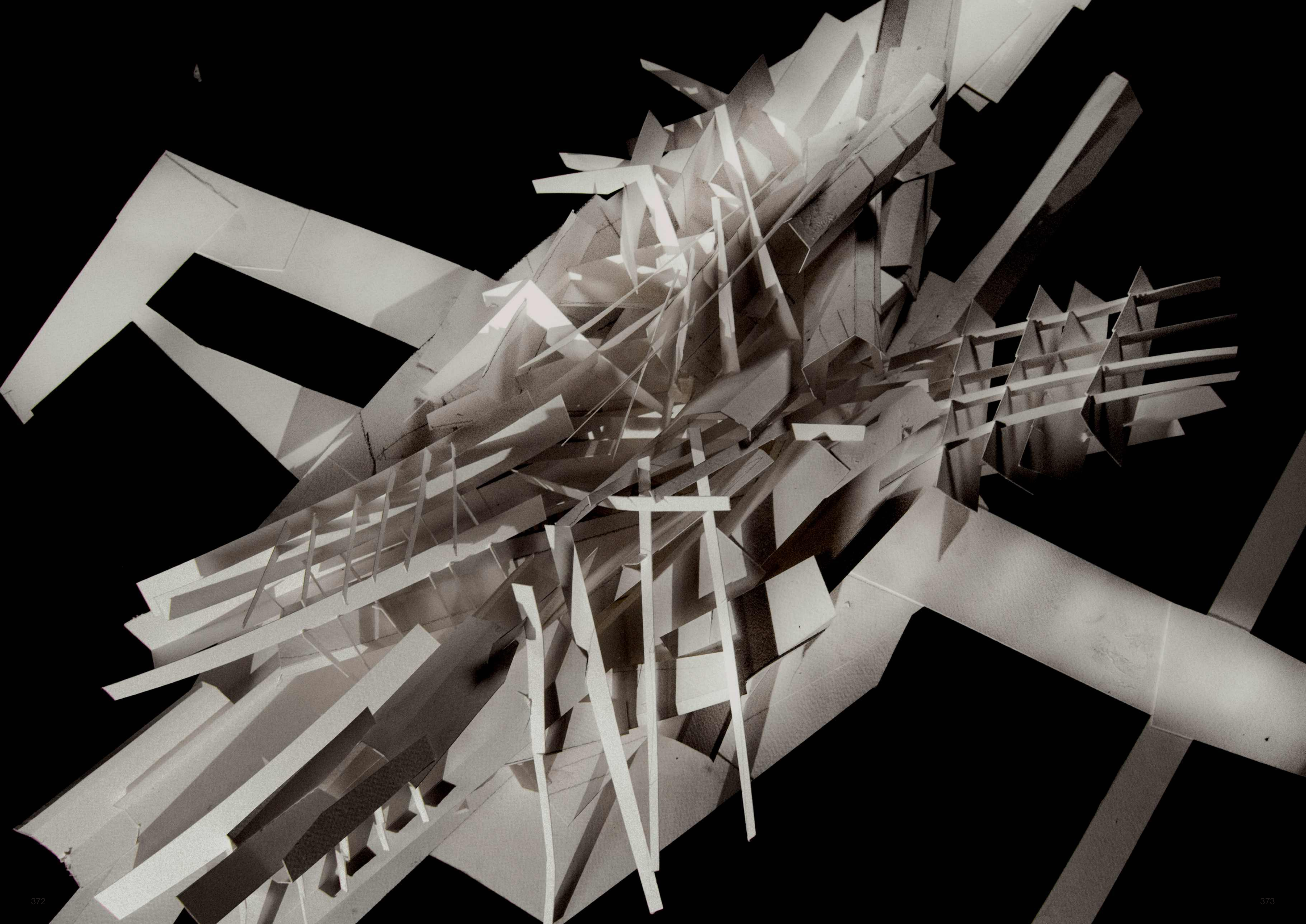
100% of roof = 95 solar panels = 35.2 kWh/day

- cost = (33,250 pounds)

weight = 2014

total kWh/year = 12848

Remember this will decrease 0.25% per year.



Structural Calculations

option 4

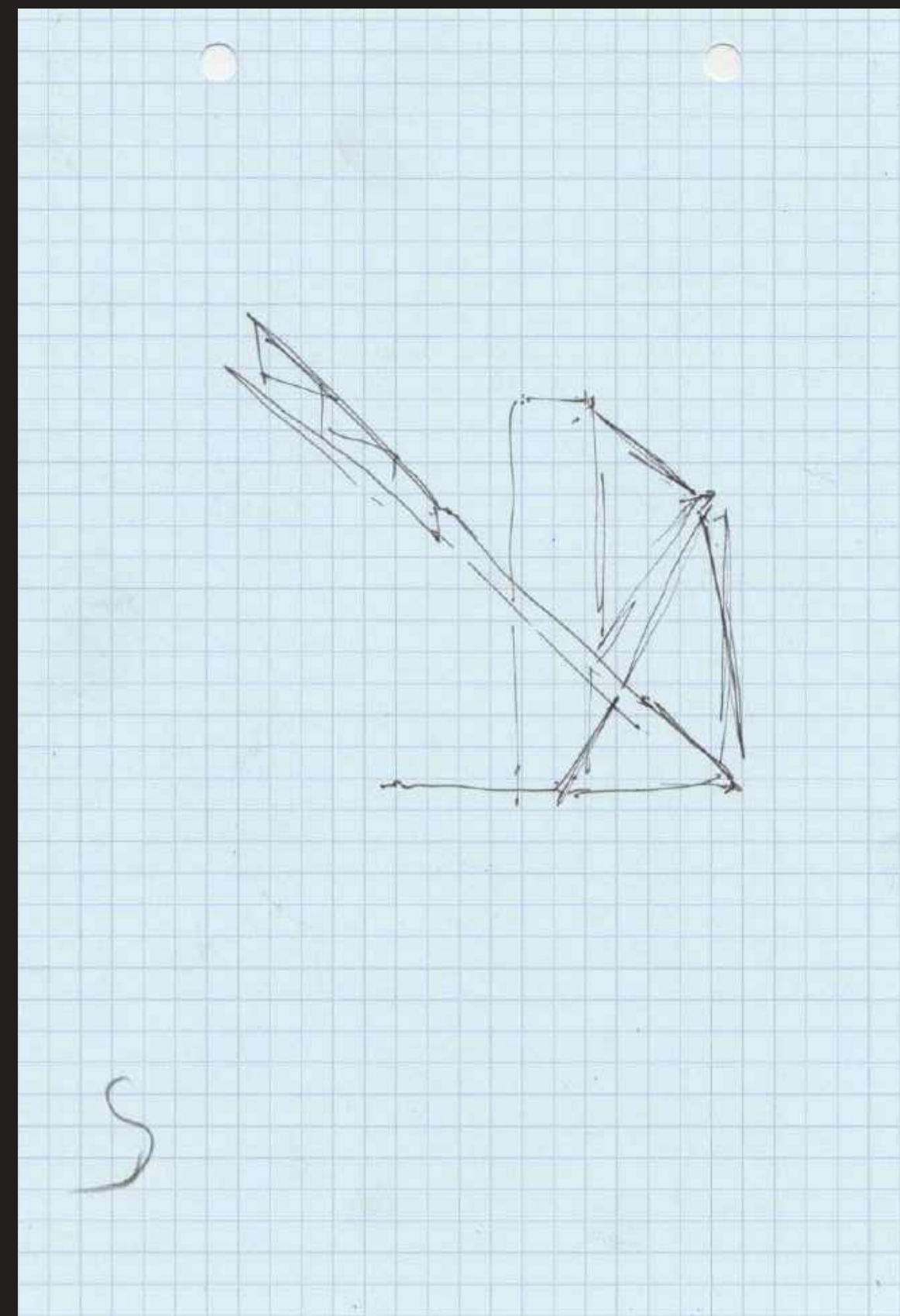
with this we can minimize the depth of beam. But beam will affect the design.

Beam 1 = 12.5m
Beam 2 = 17.5m

span = d
depth = 12.5m = $\frac{12500}{294} = 625 = 0.625m$

10
20 = 0.5m
15
17.5
20 = 0.875
110 x 305 x 200
150 x 305 x 200

Eurocodes so consider good for bridges



Span/Depth

option 1 section Full thick bottom

plan

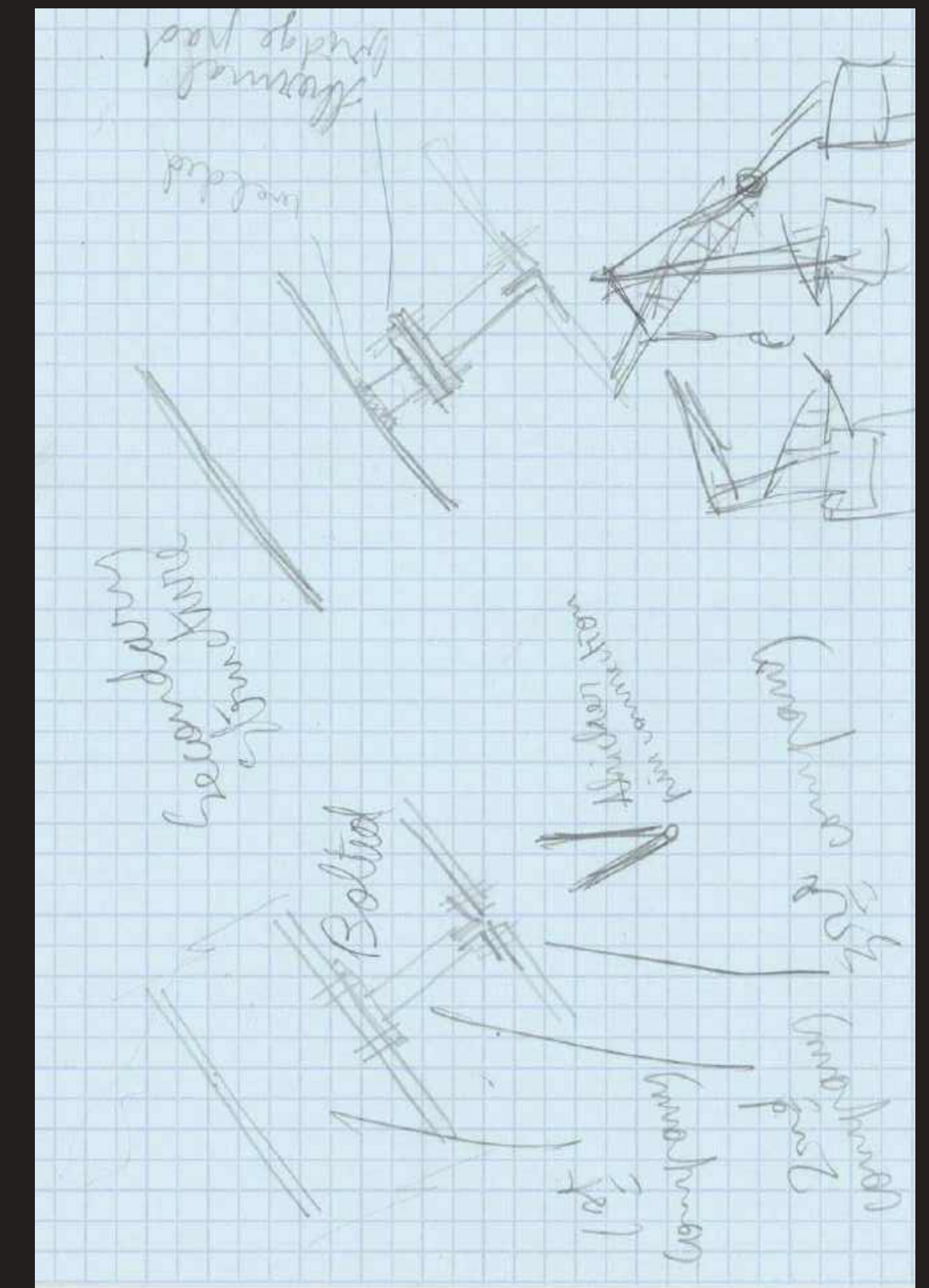
width

need to try and maximize x

option 2

option 3

Here we are using low steel which is more sustainable and makes our structure lighter.



Hoory 4/3/23

Structural depth in thick for a big project

structural grid

beam

pile

vertical

Bending moment force height

Members in Eurocodes just low diff in height or beam

Wrocodes

600 x 200

commit to a decision

design force + length

$M = \frac{wL^2}{2}$

$L.A = \frac{M}{d} = \text{Axial 'push'}$

$A = 800kN$

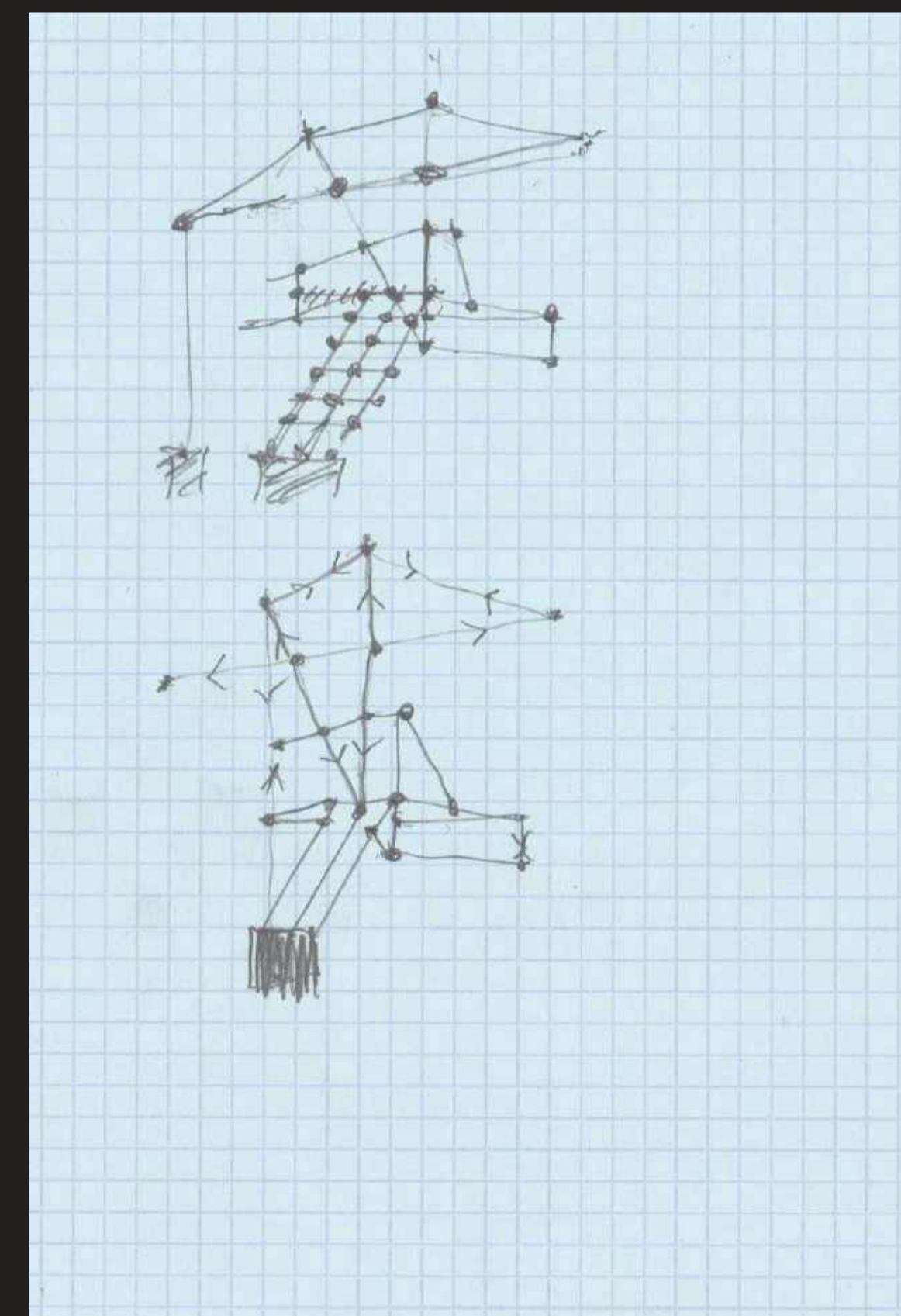
$L_e = 4000mm$

Blue Book SIZE

$V = wL$

$F = \frac{N}{\cos \theta}$

UC352



level 3

level 2

level 1

River Bed

STEVEUR WANTS TO GO THAT WAY

Here I want to go down

3

2

1

River Bed

3.5m

7m

3.5m

3.5m

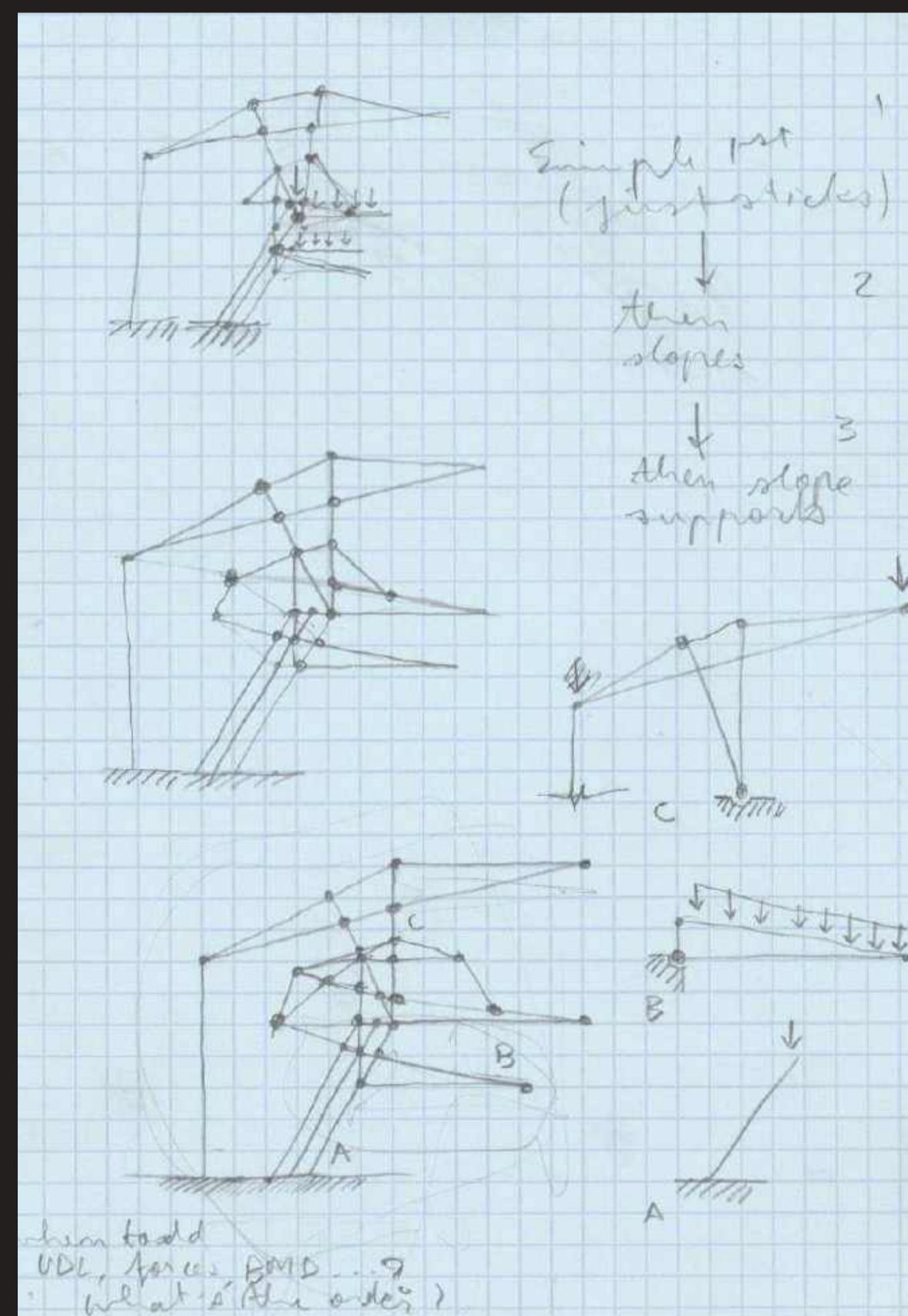
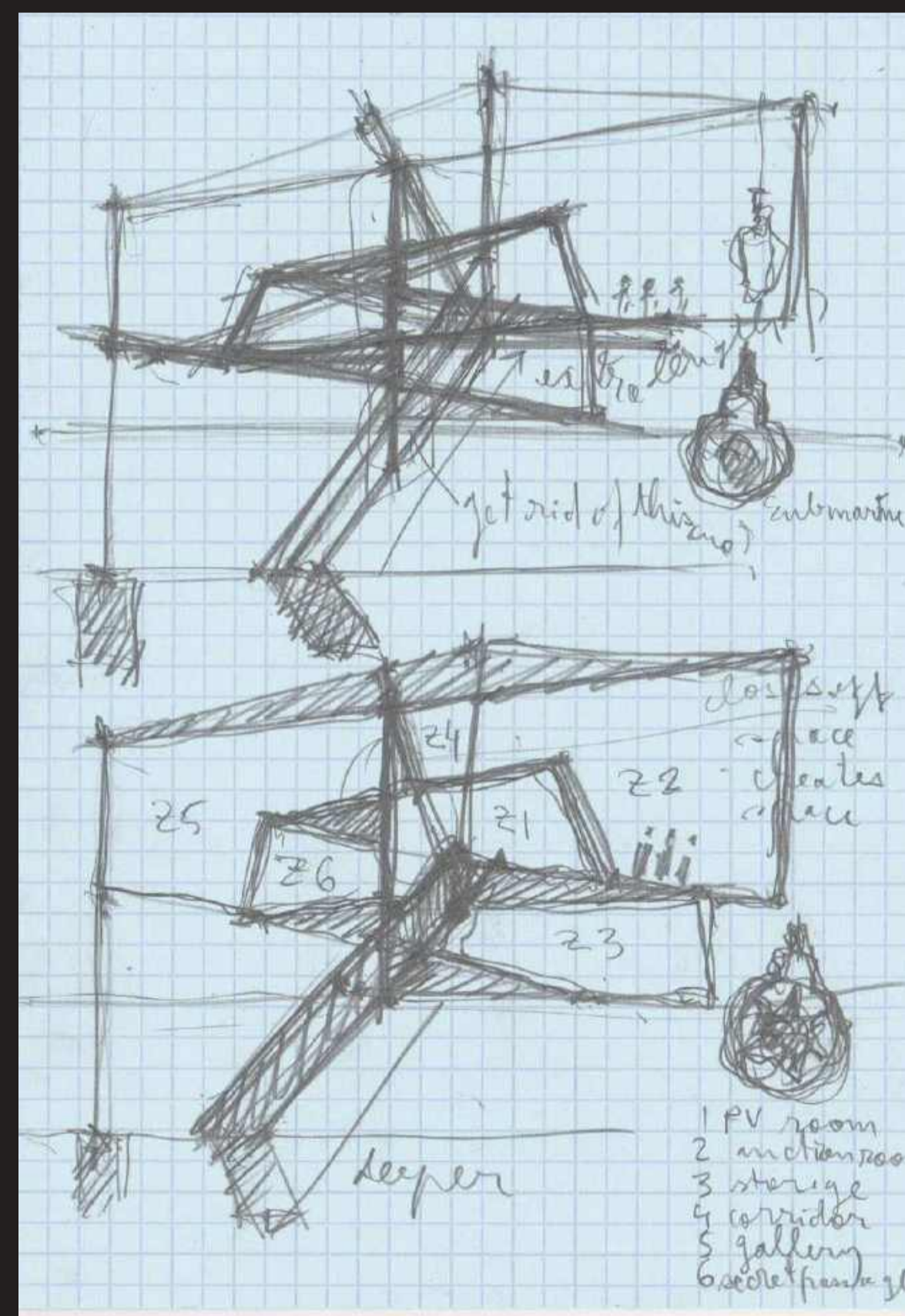
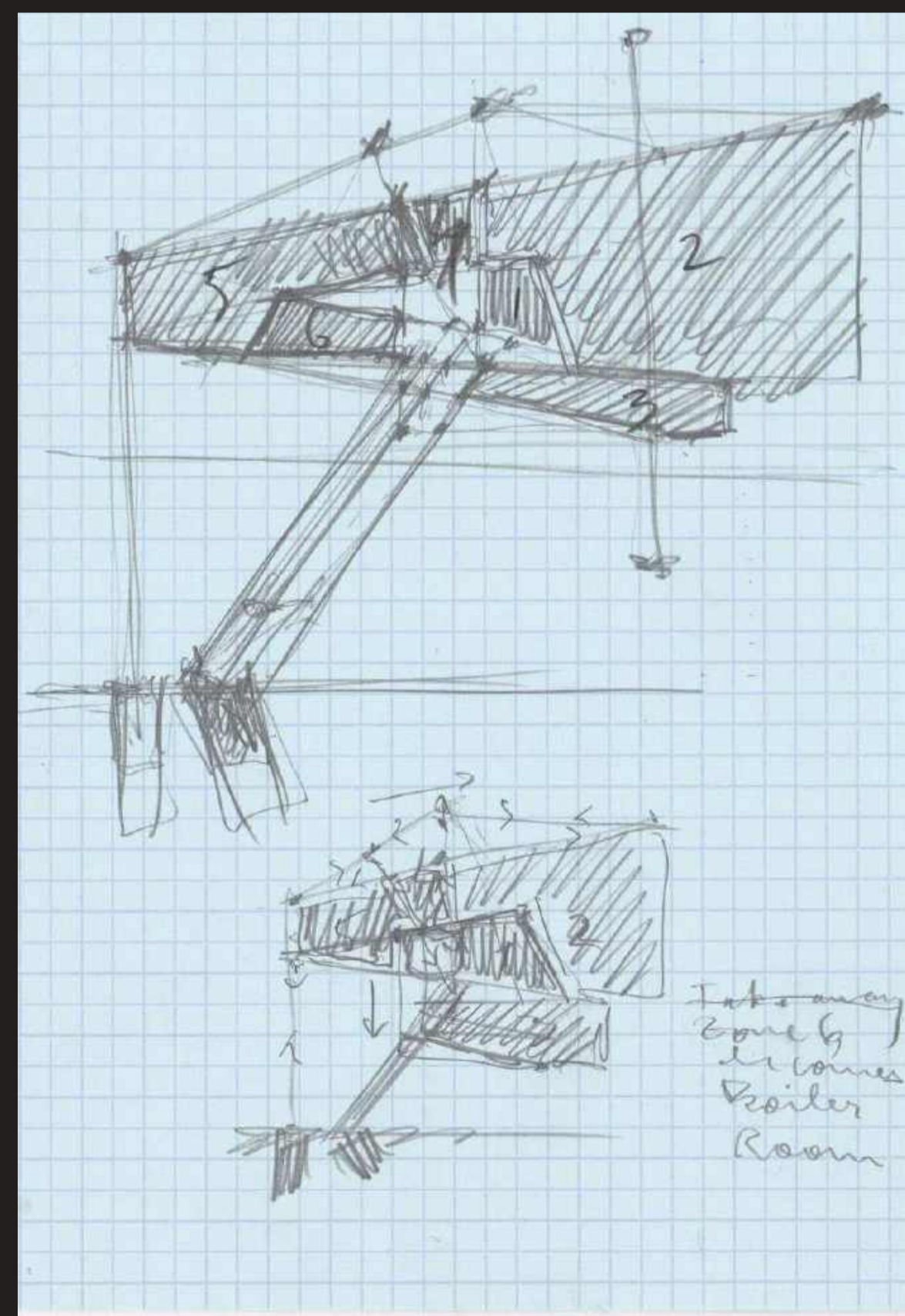
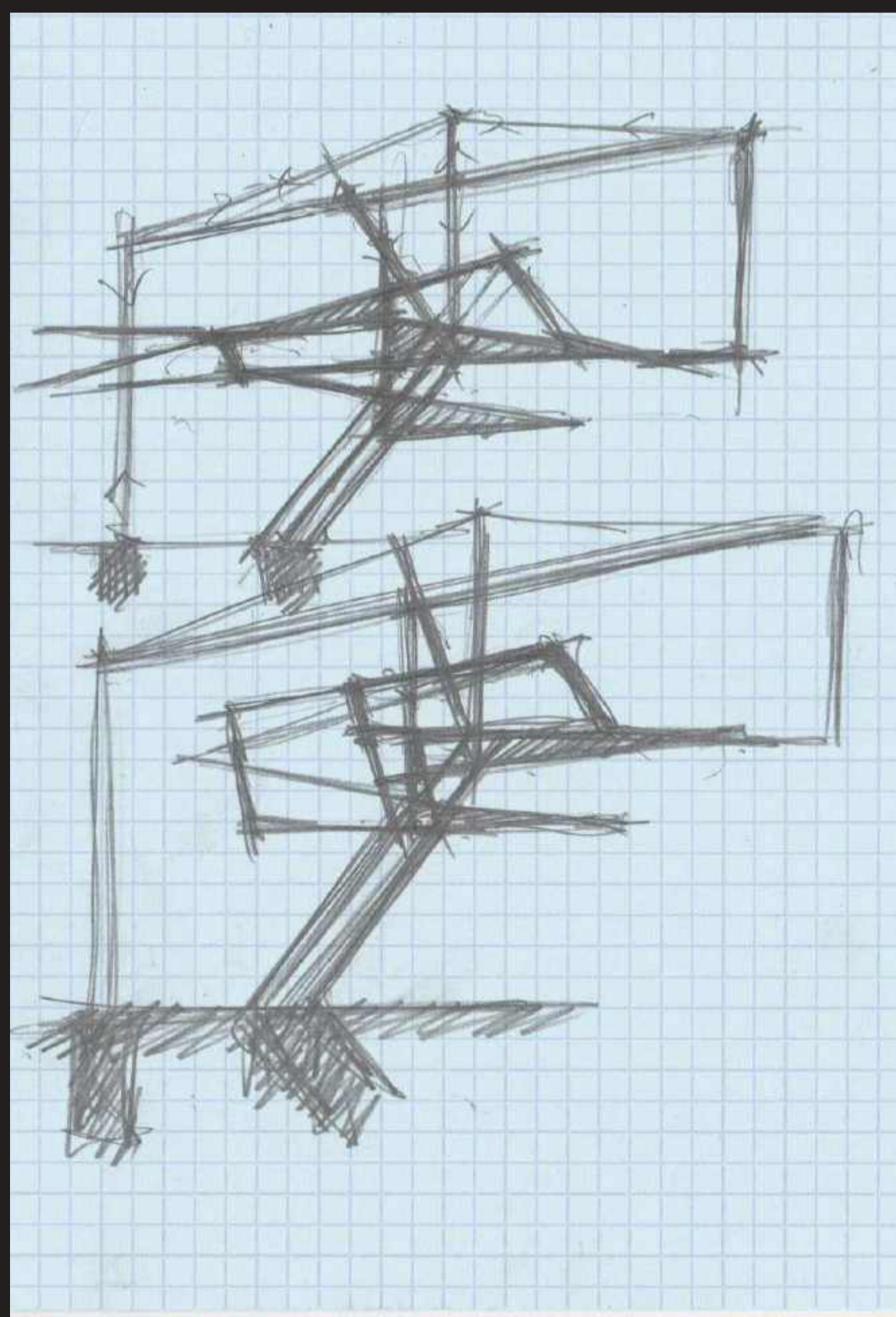
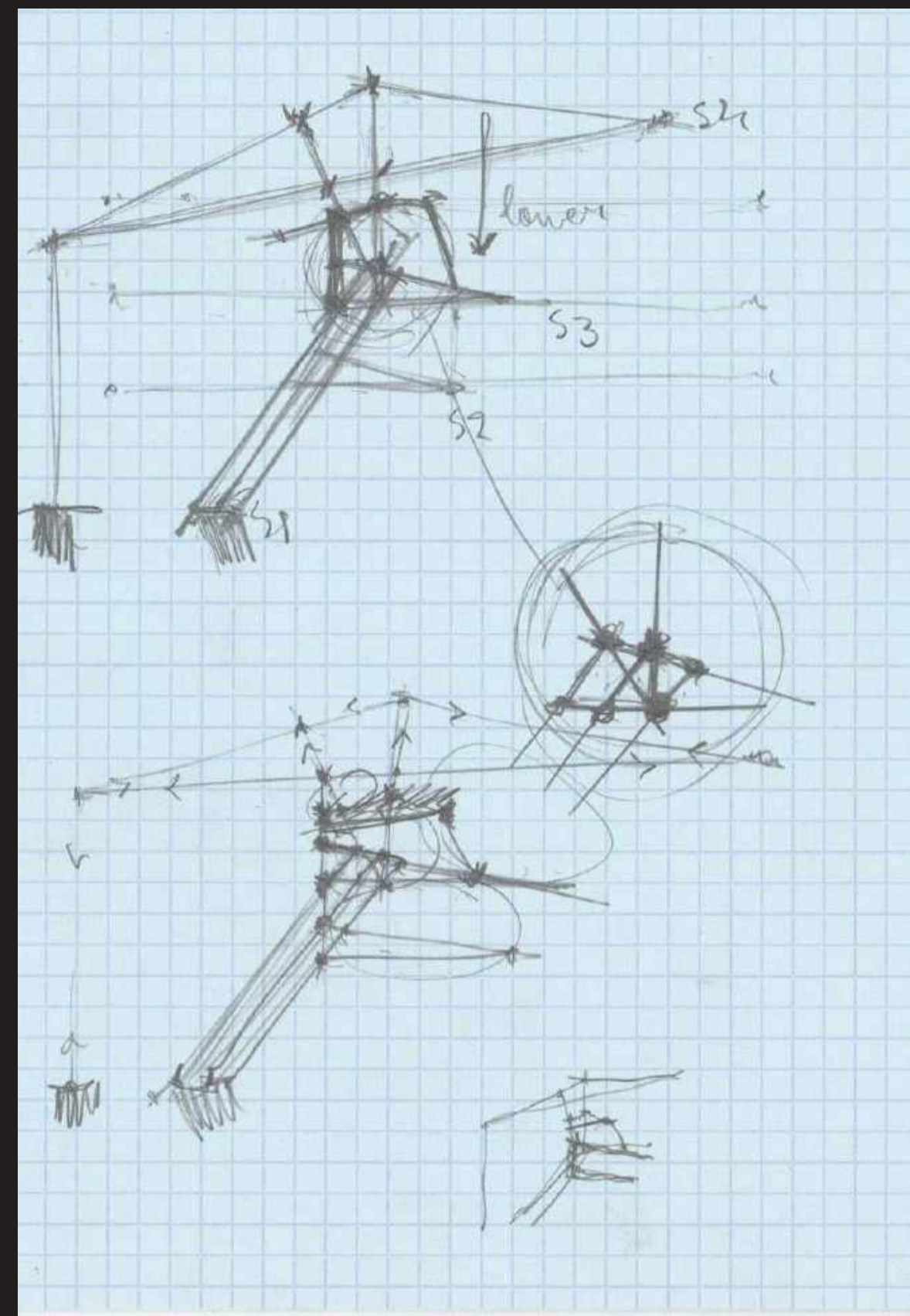
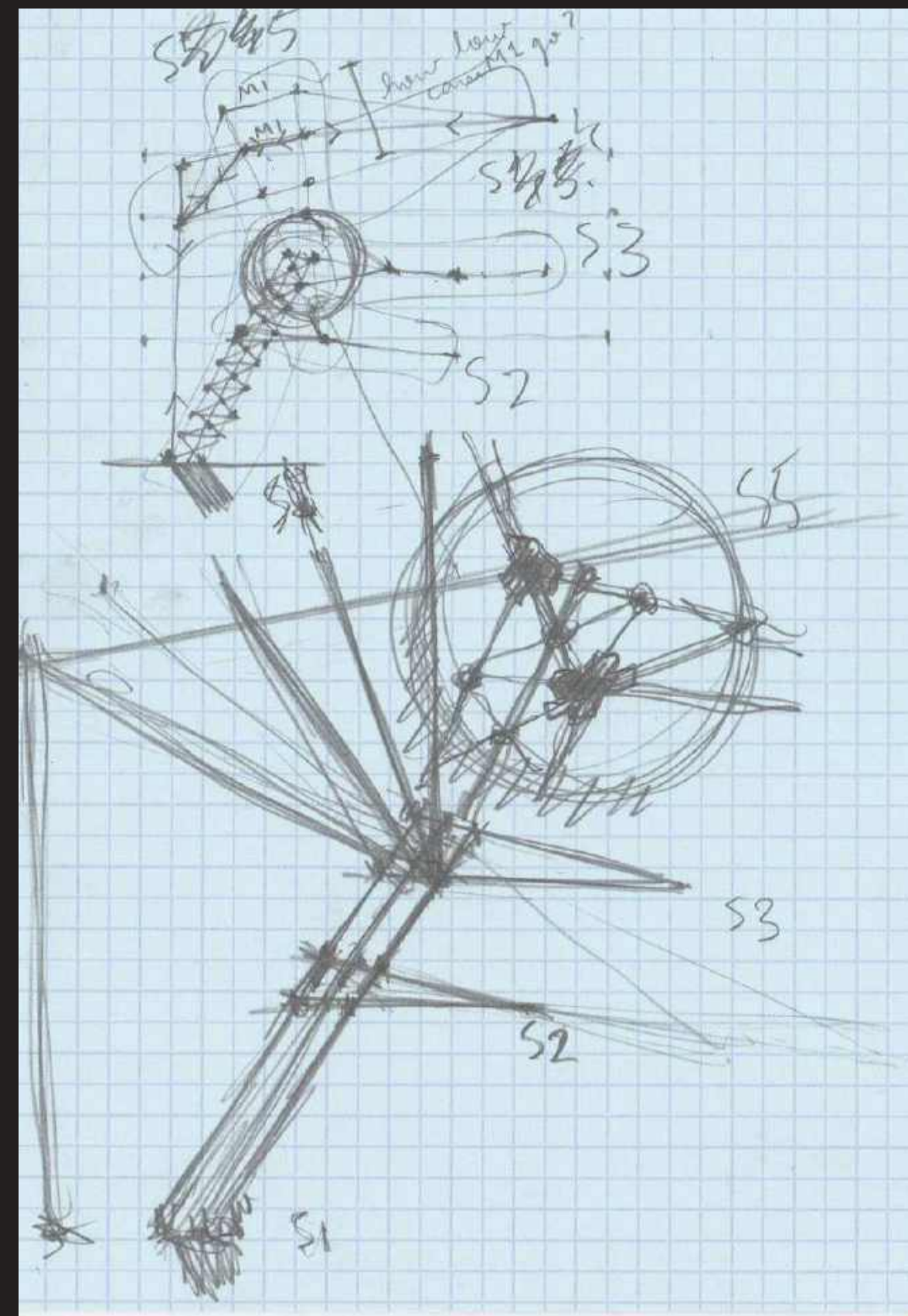
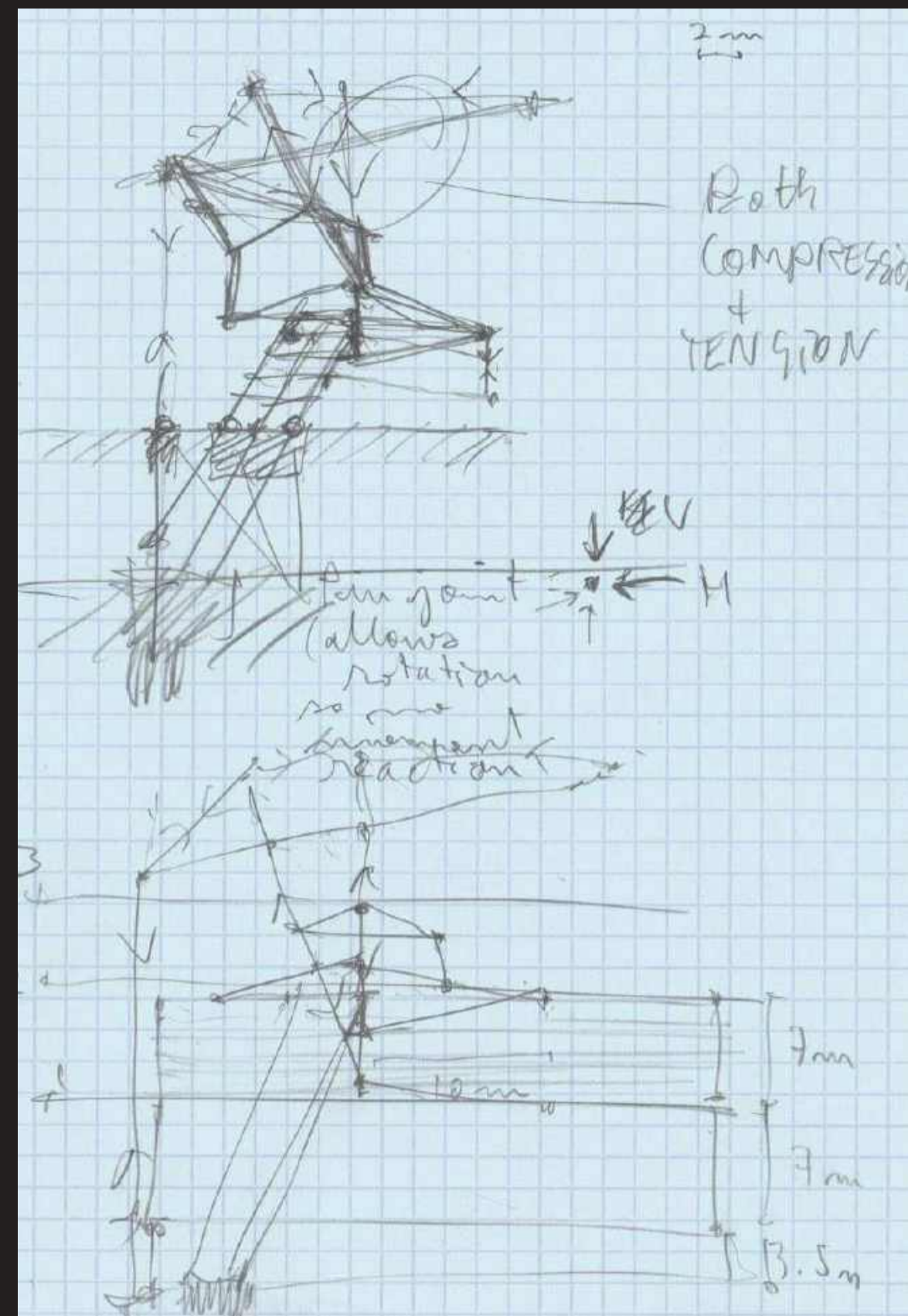
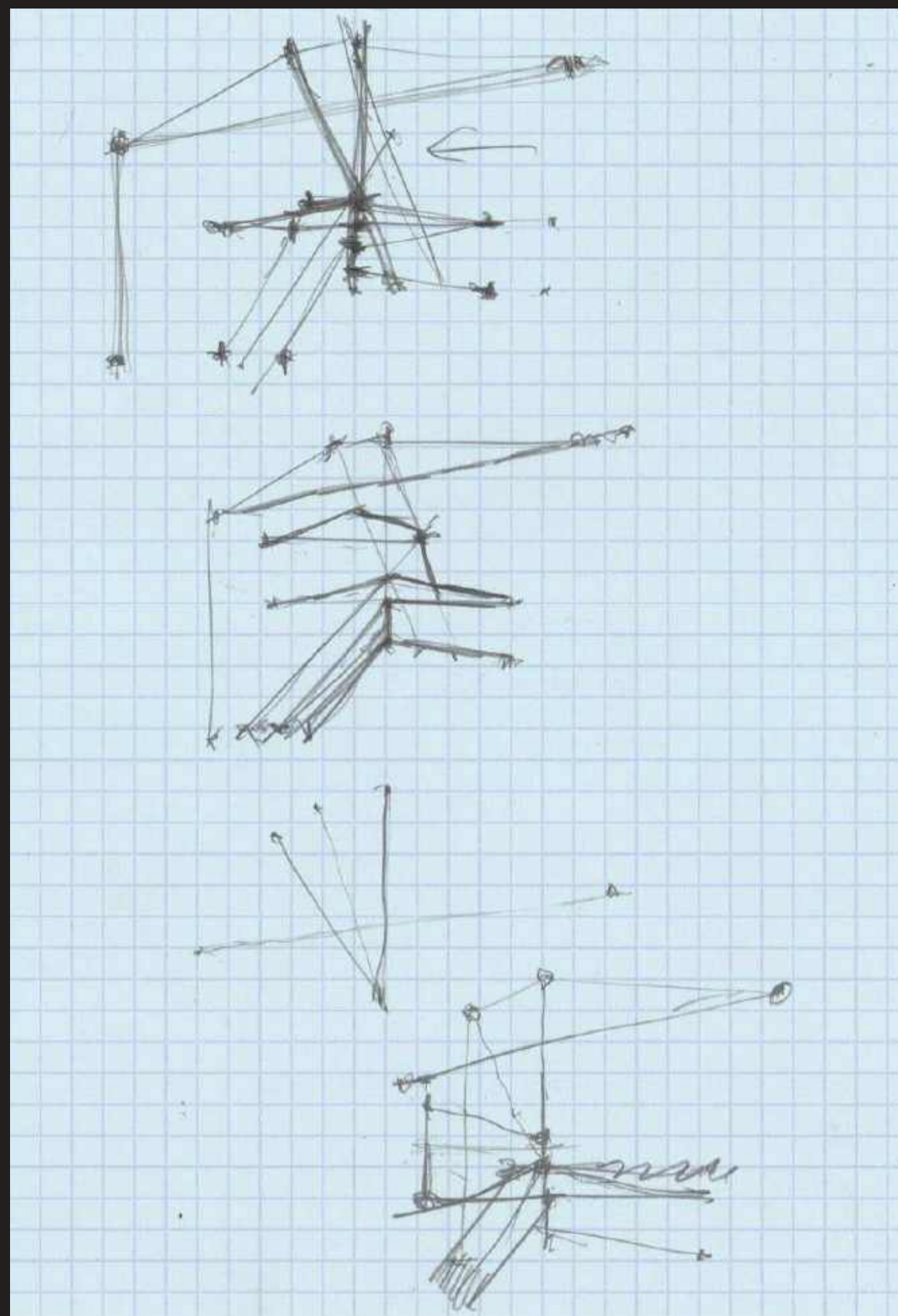
Tides

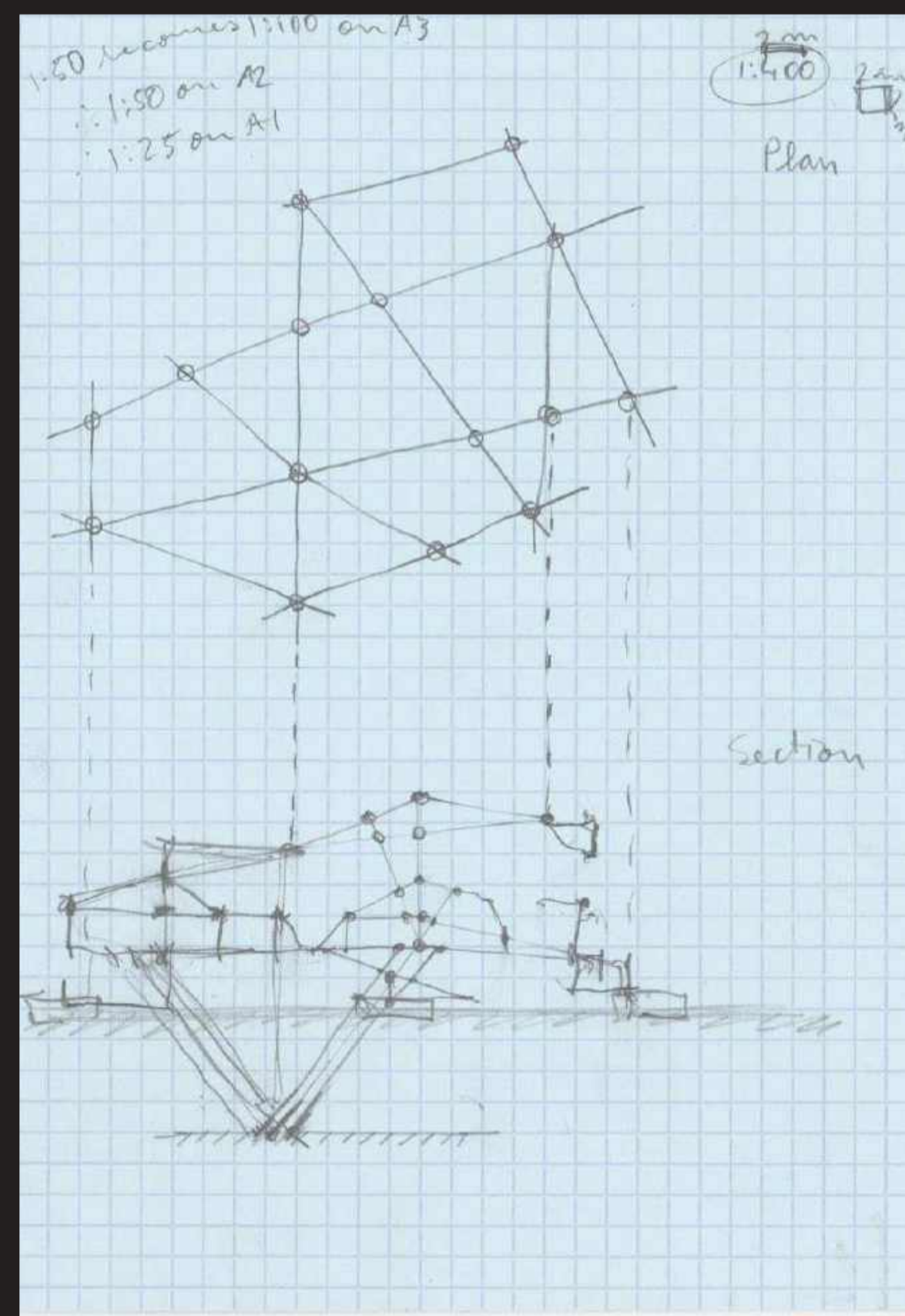
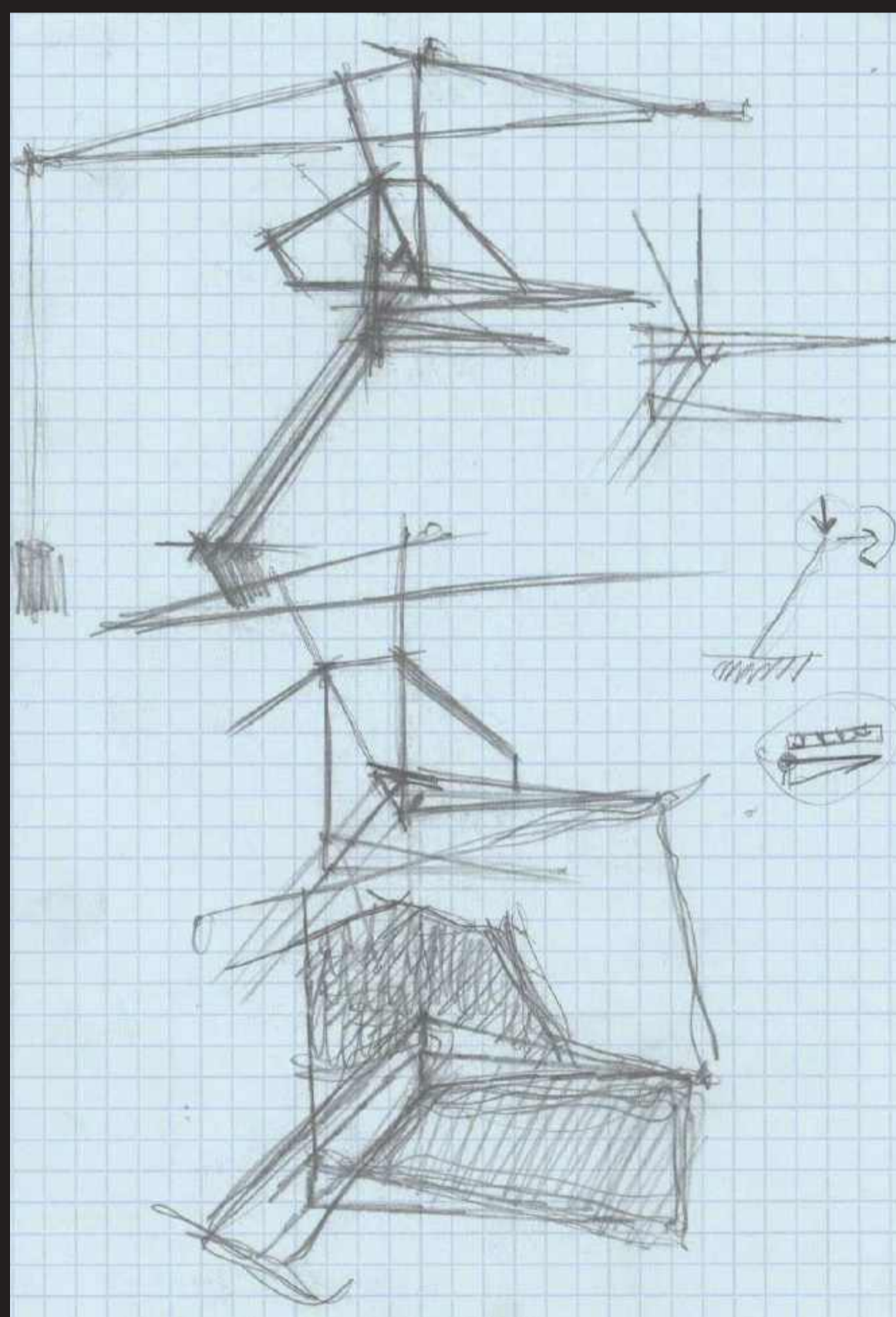
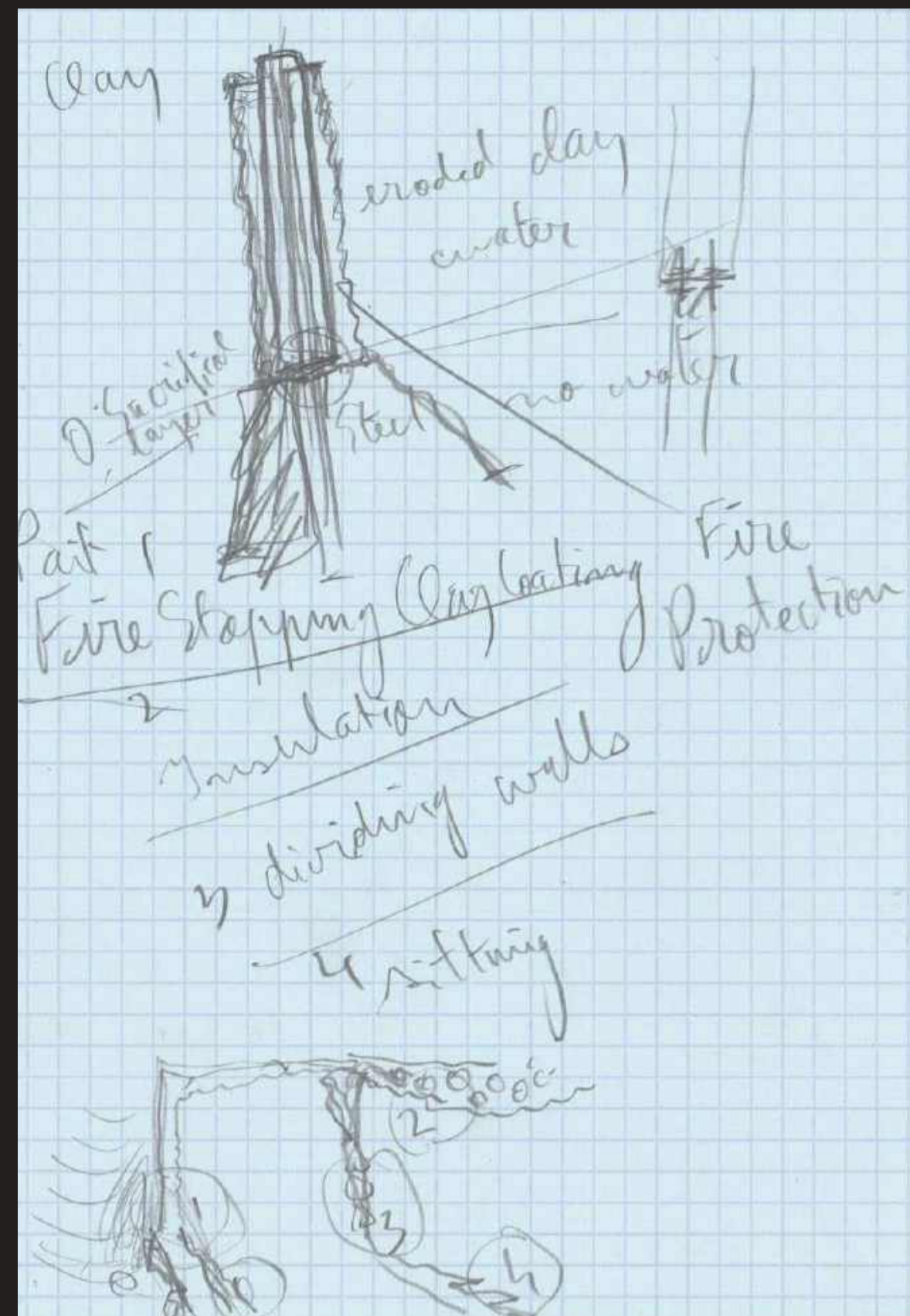
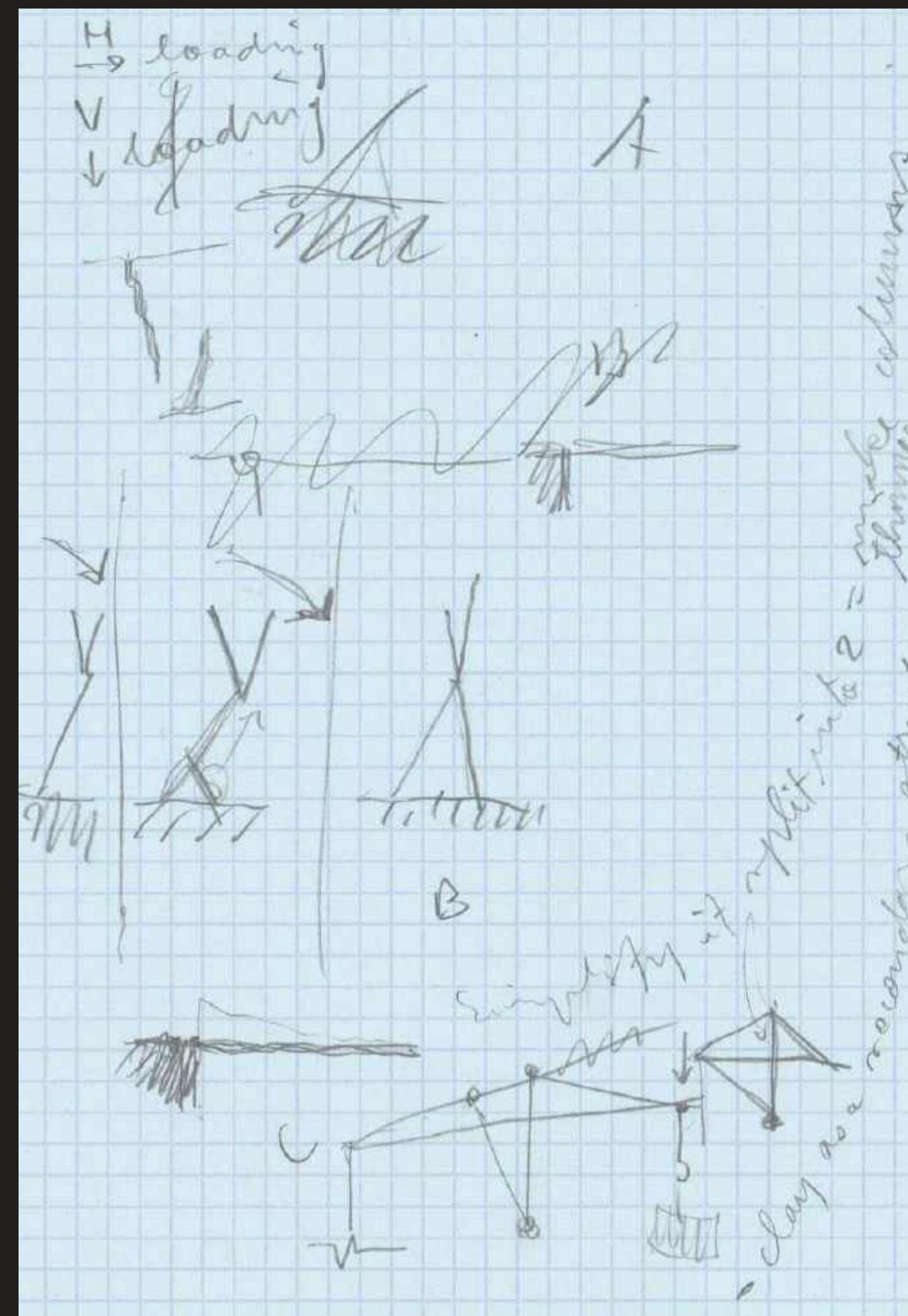
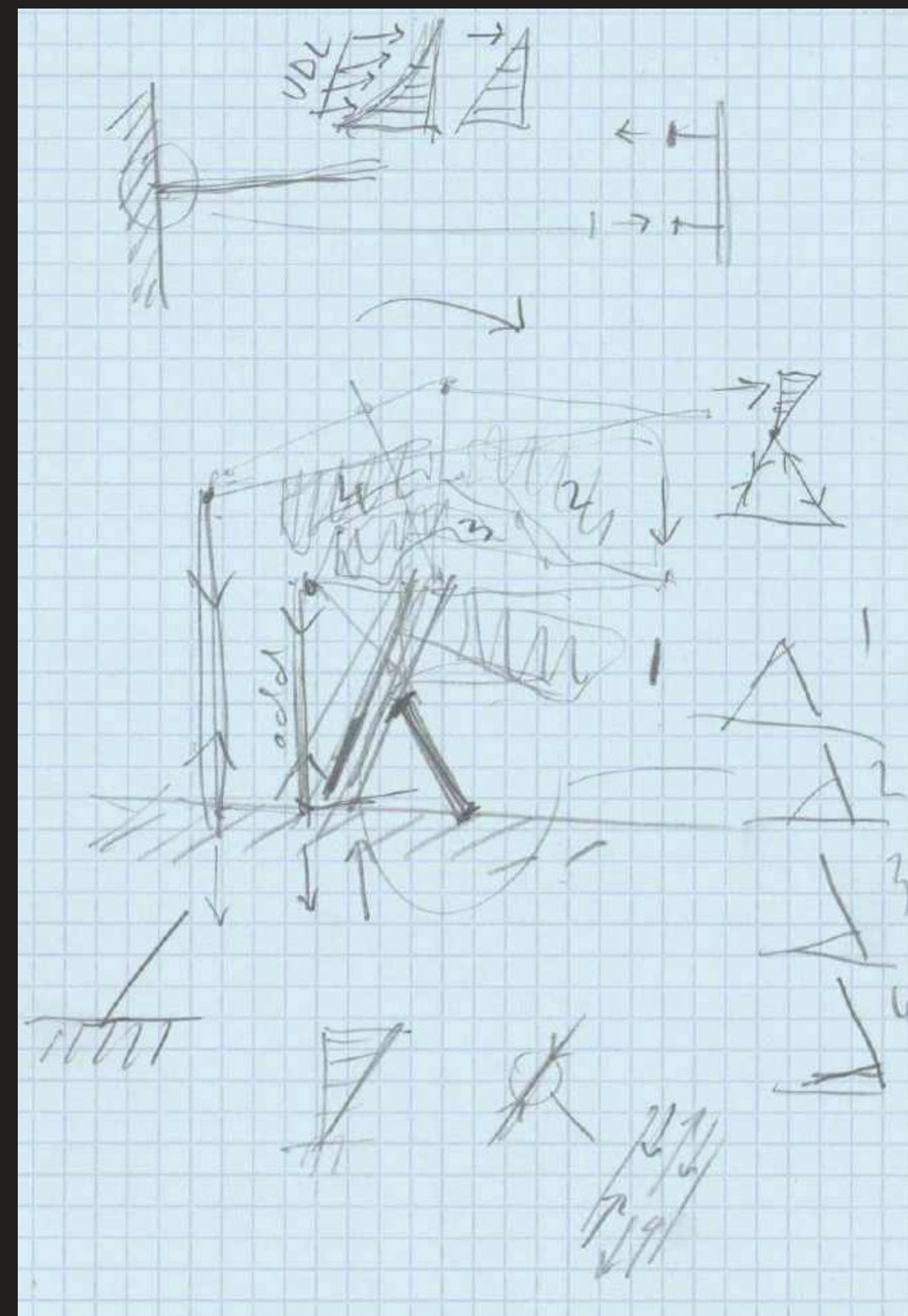
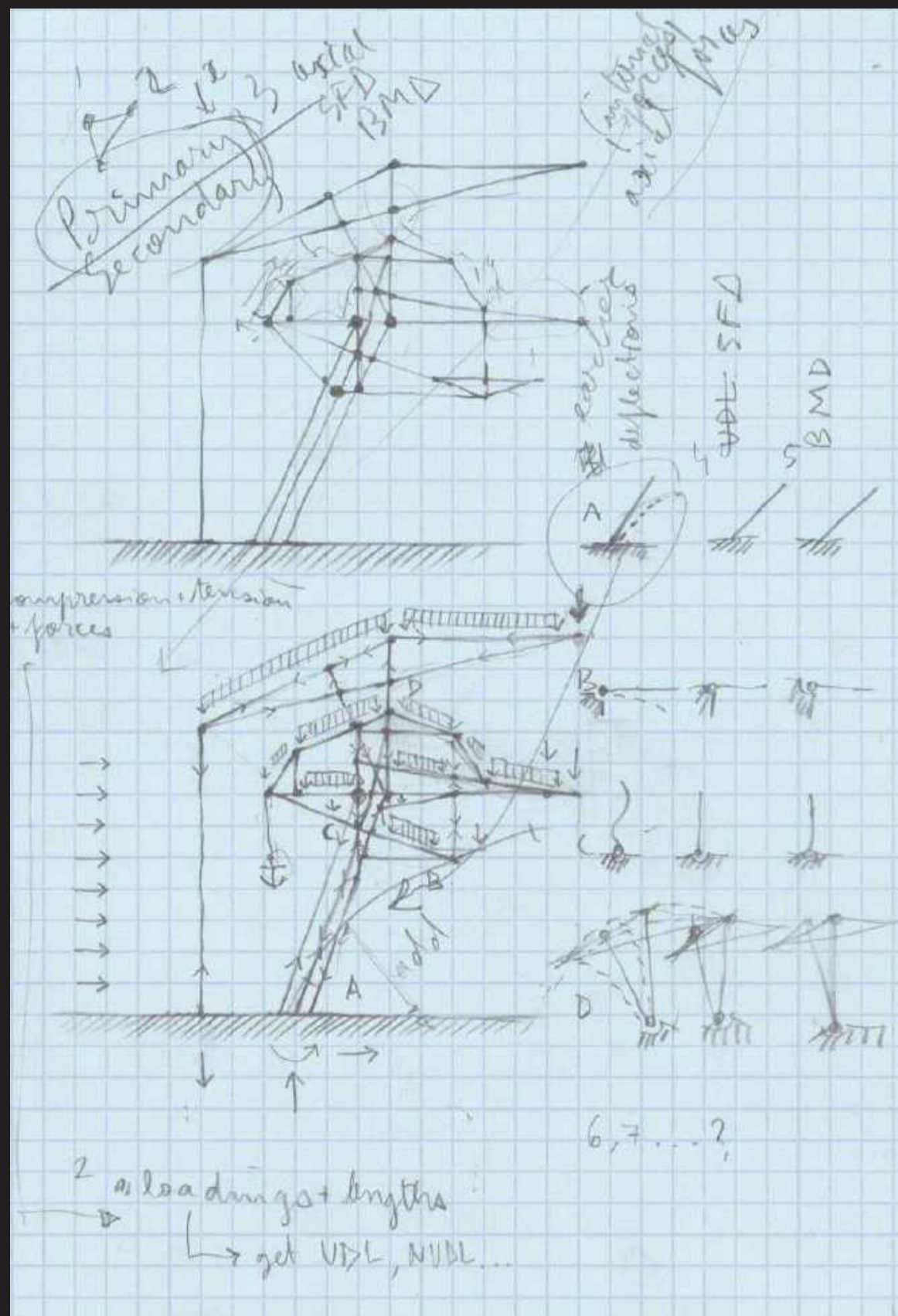
compression

compression

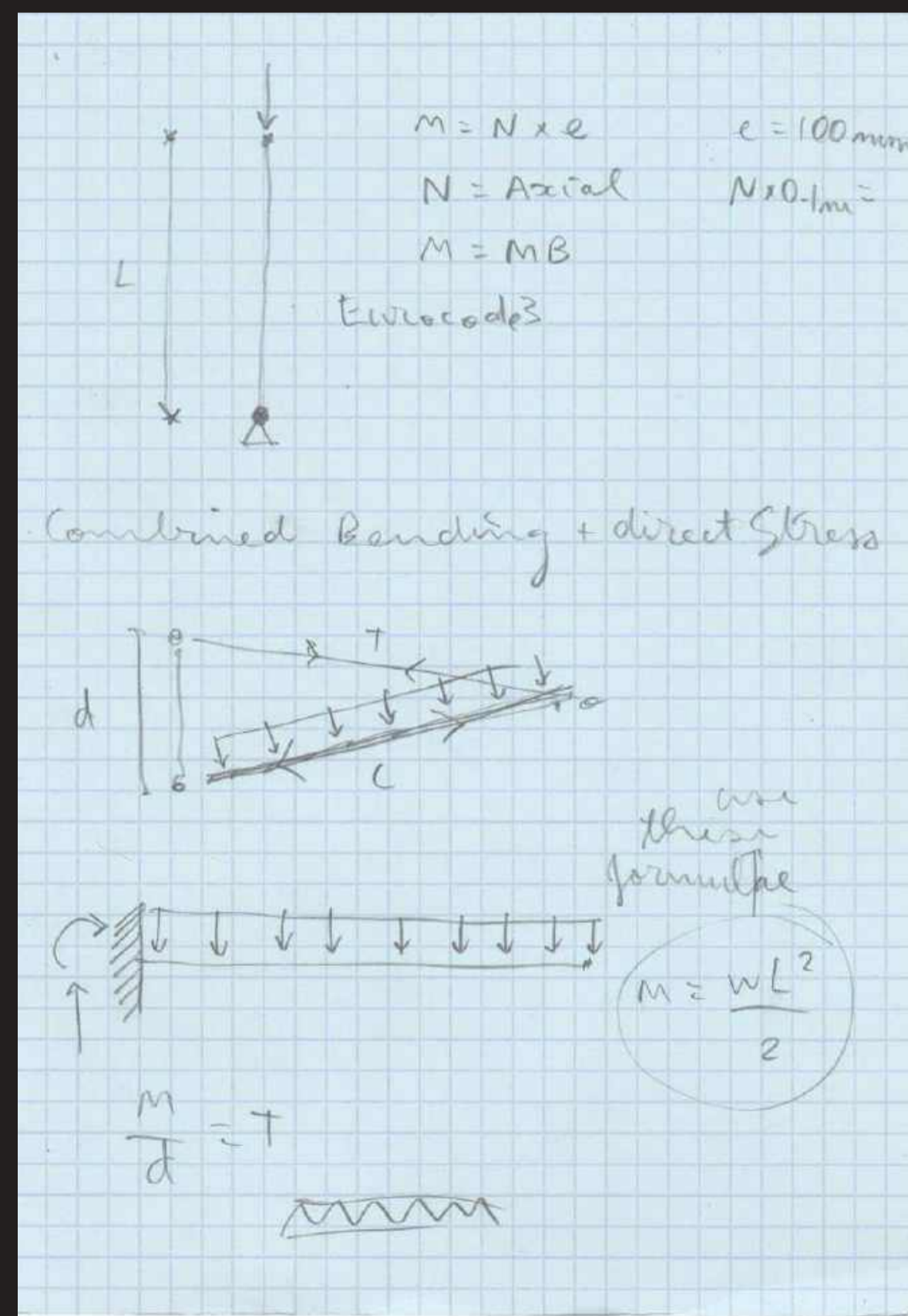
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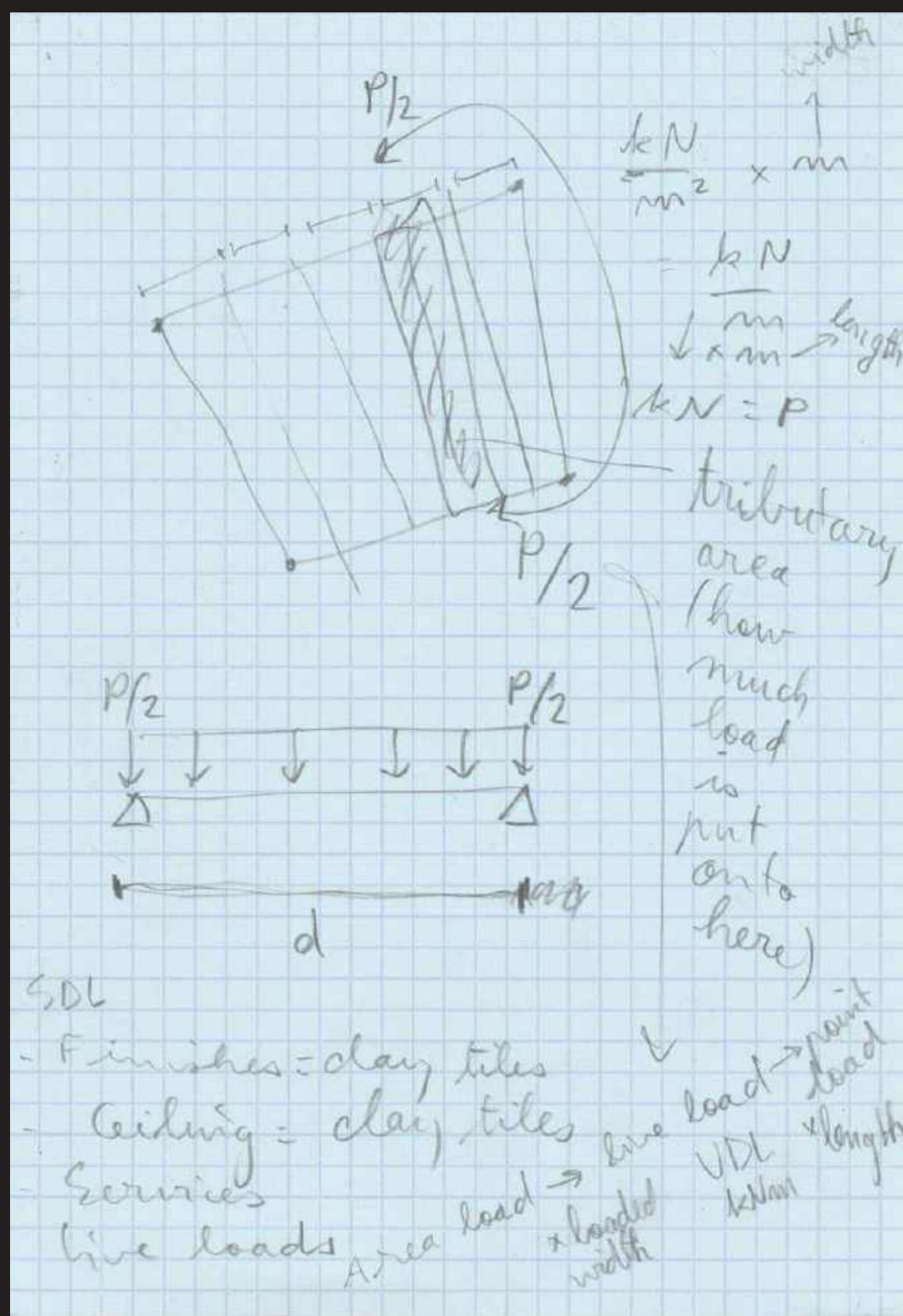
compression





- Sticks + BMD + SFD
 - 2- Loadings + Eurocode
 - 3- divide into sub elements + Resolve
 - 1- (Eurocode) (Blue Book, Eurocode)
 - 5- Tech / Axial Drawings
- 1- stick diagrams (multiple sections)
2- BMD, SFD / key deflections for general
3- key constraints, design problems
4- long spans + overall stability
5- divide into sub elements, sub elements
6- BMD, SFD, key deflections sub elements
7- prelim sizing for sub elements
8- draw key connections + key structural components (knitting pretty)
9- foundation reactions forces for design
10- final drawing presentation





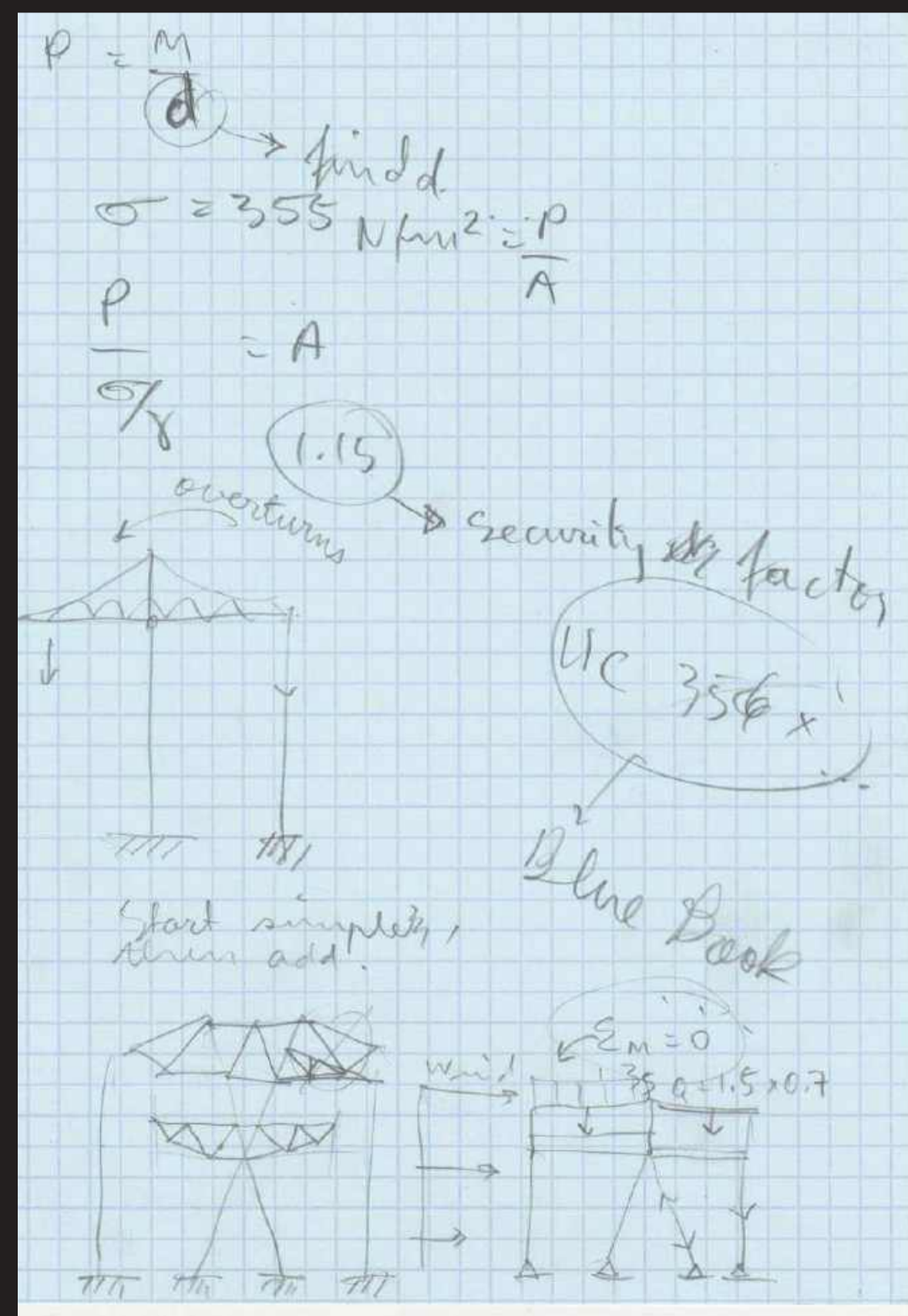
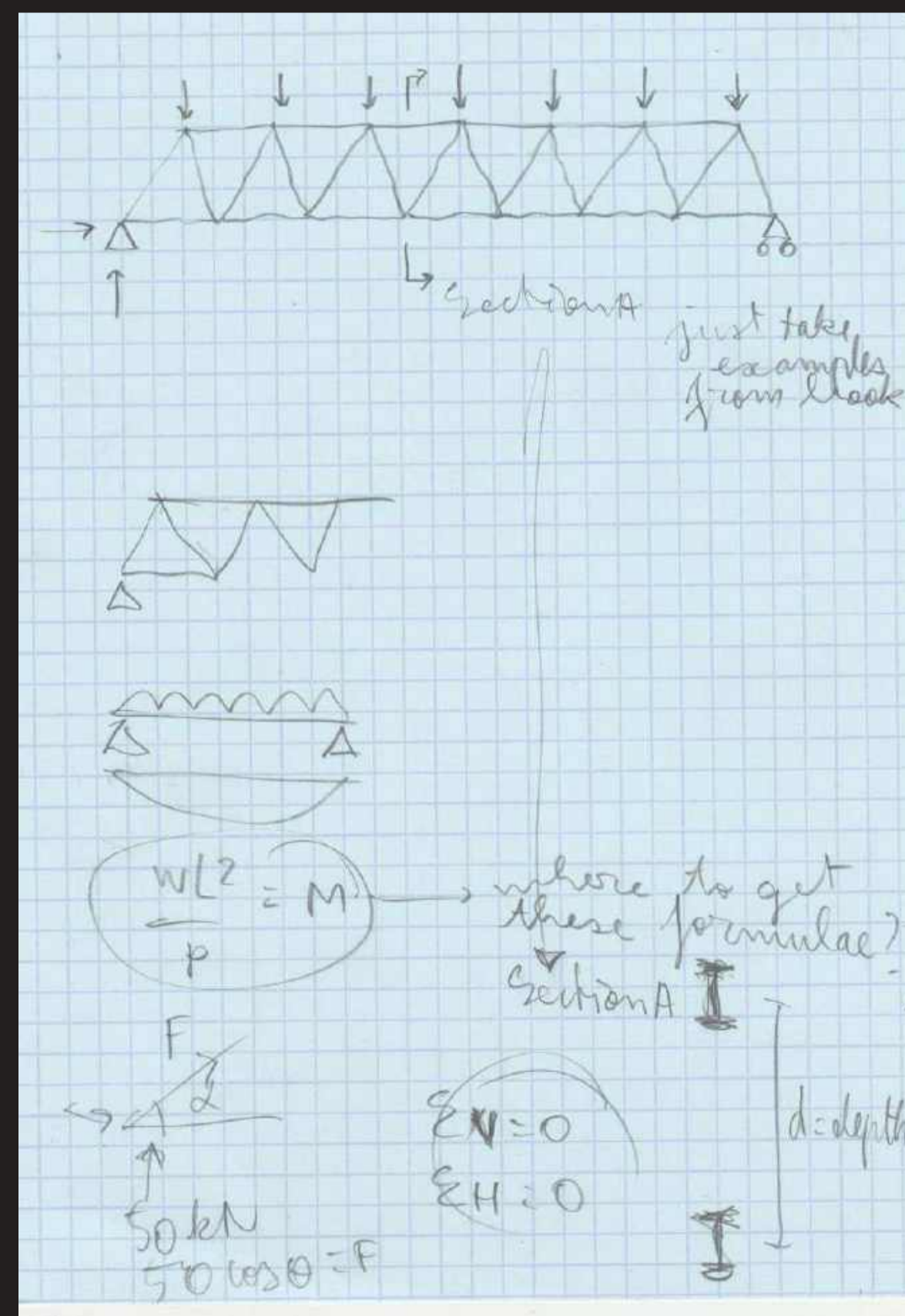
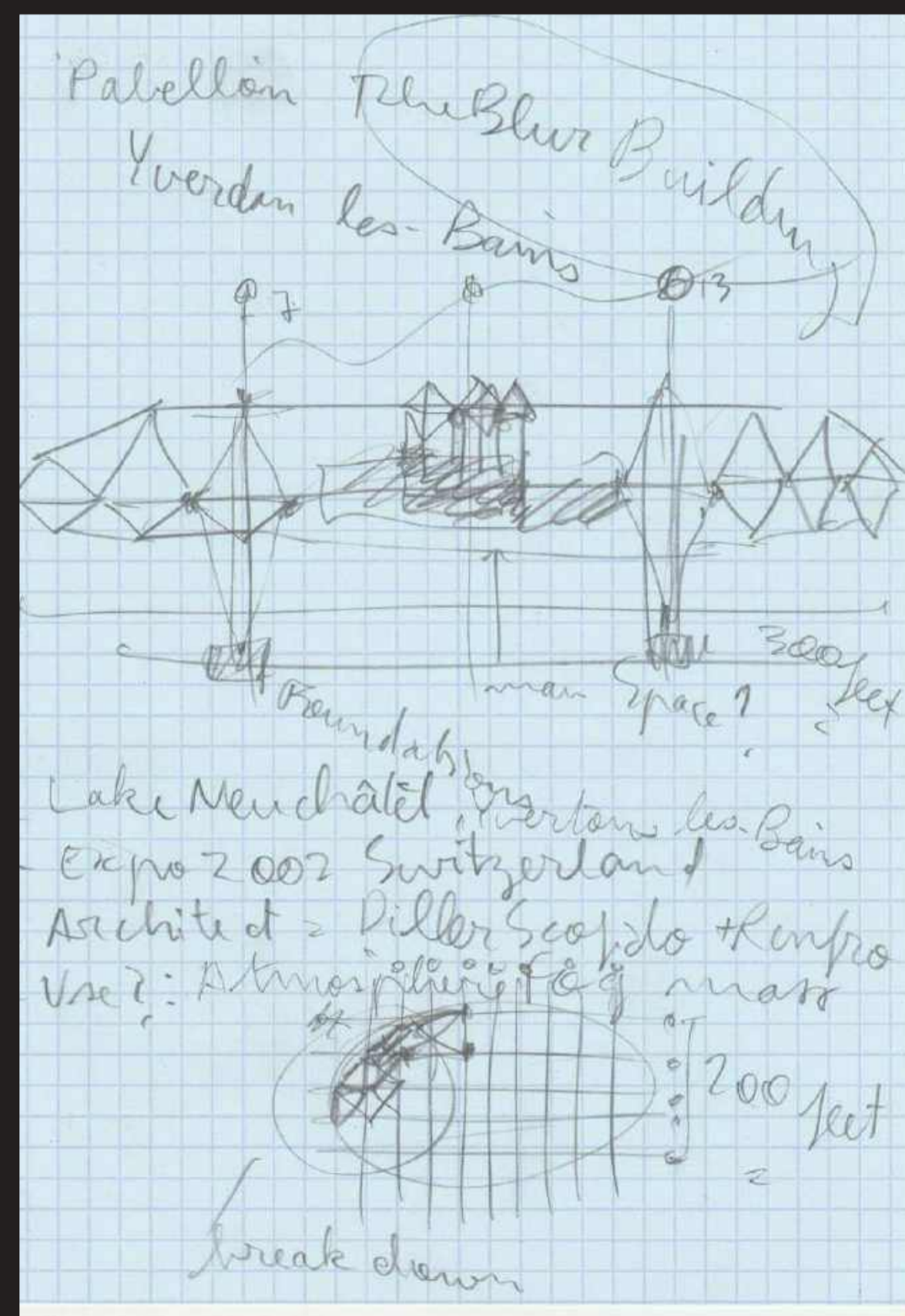
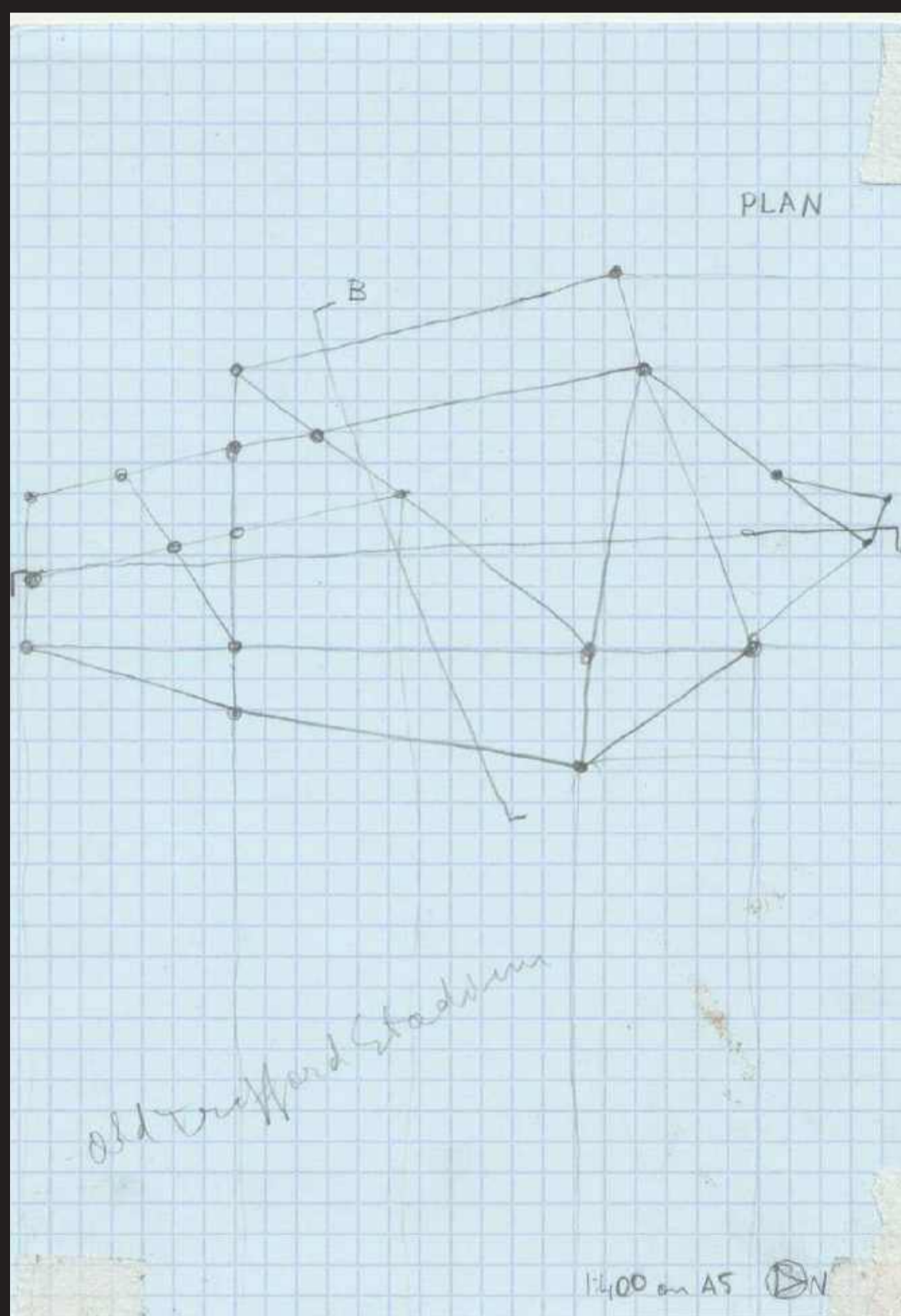
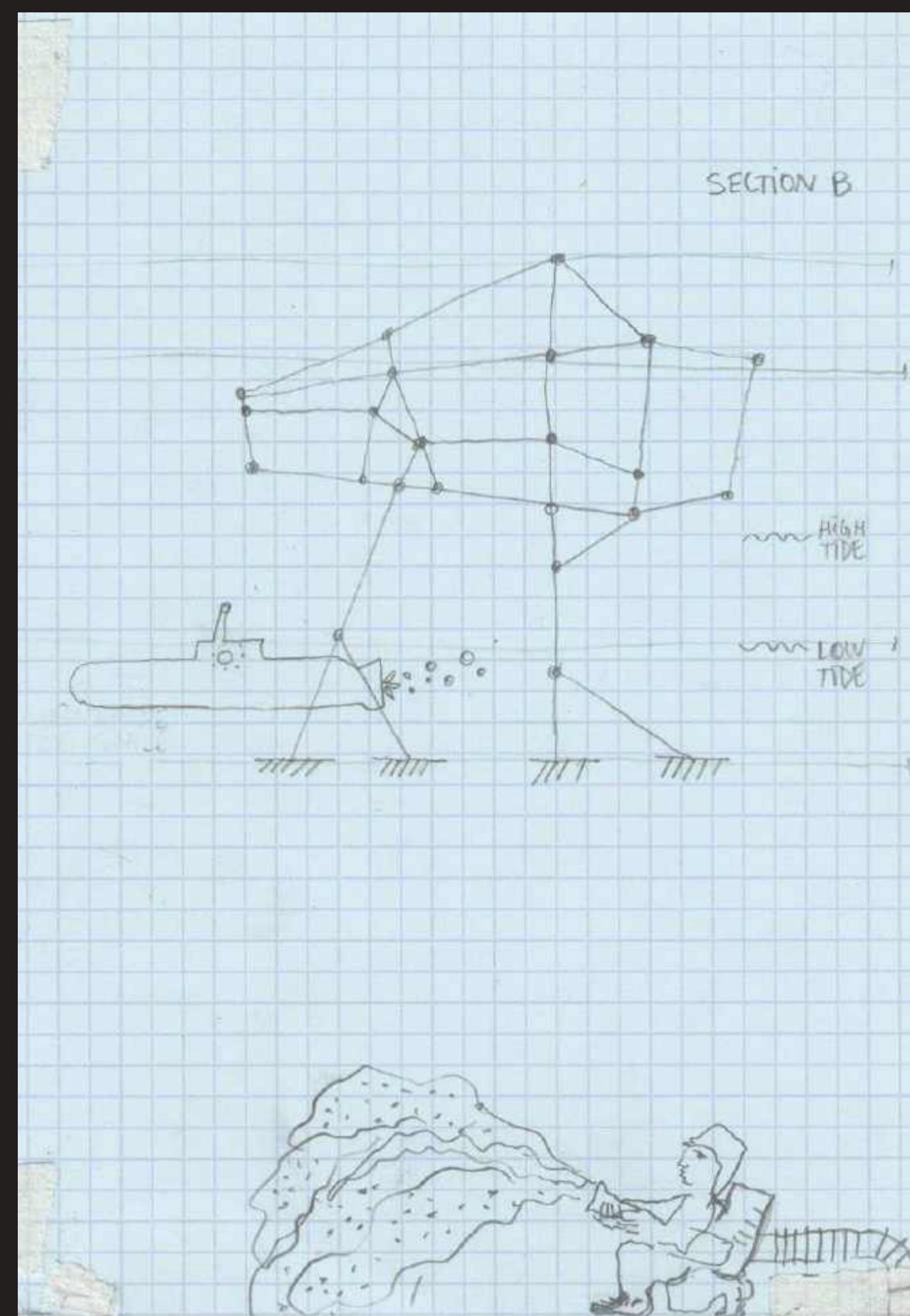
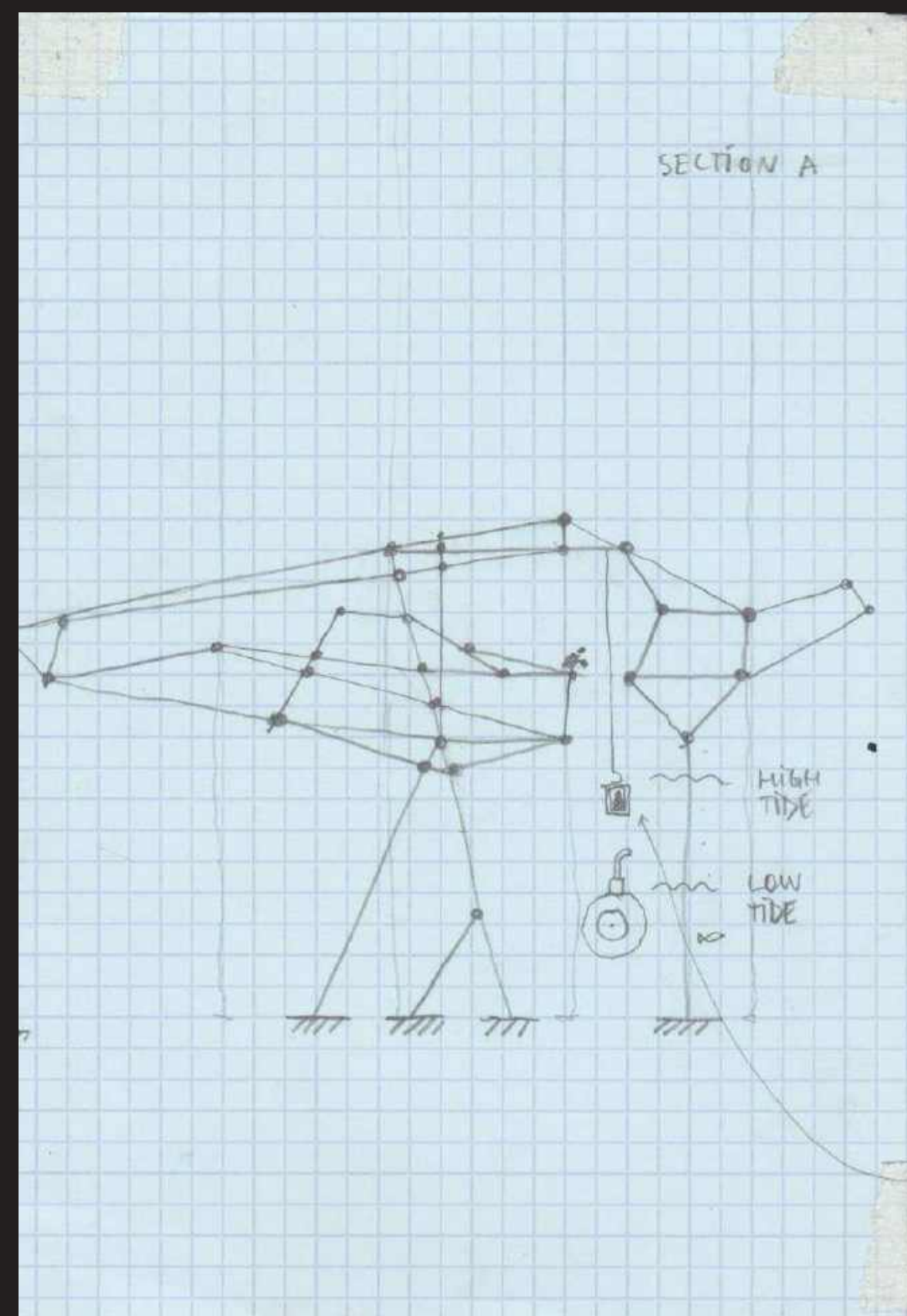
LOADINGS*

* STREAM OF THE RIVER IS NOT TAKEN INTO ACCOUNT

CONSTRUCTION	MATERIAL/HEIGHT	DENSITY (kN/m³)	UNIT LOAD (kN/m)
ROOF	SNOW/WIND UPLIFT	n/a	0.8
	MUD	18	0.6
	INSULATION	9	0.05
	PURLIN	78	0.1
	STEEL ROOF PANELS	78	0.25
	PRIMARY STEEL WORKS	78	0.1
	SERVICES	78	0.1
	PRIMARY STEEL BEAMS	78	0.5
	SECONDARY STEEL BEAMS	78	0.05
	STEEL CABLES	78	0.025
WALL	PORCELAIN TILES	22.5	0.05
	GLASS WINDOWS	16	TOTAL = 2.175 kN/m²
	INSULATION	9	0.05
	MUD	18	0.6
	CERAMIC TILES	25	0.05
	PRIMARY STEEL COLUMNS	78	0.05
	SECONDARY STEEL COLUMNS	78	0.05
	TOTAL W		0.95 kN/m
	TOTAL R		3.175 kN/m
	TOTAL		3.175 kN/m

USE	CONCENTRATED LOAD (kN)	UNIFORMLY DISTRIBUTED LOAD (kN/m²)
GALLERY	3.6	4
ART STORAGE	7	2.5
AVIATION SPACE	3.6	2.5
PRIVATE SALES	2.7	2.5
WALKWAYS	4.5	2.5
ART ON CRANE	0.5 - 10	n/a
TOTAL	25.1	2.5

TOTAL DESIGN LOAD = 25.1 kN/m²



1- Stick diagrams
 2- Loads
 3- Forces, reaction forces, key connections, SFD, BMD
 4- Long spans and overall stability
 5- Preliminary sizing
 6- Key connections
 7- Foundation reaction forces

height = 25m
 width = 19m
 length = 14m

plan
 5 5 5 5 5

main grid
 2nd grid
 3rd grid

Put in precedent studies... - WestHam
 - Stadium cantilever
 - Smoke mist building
 crane

Start Simple: Right Front

25m
 20m

5m
 25m

Load
 Roofing
 how many
 beams
 how many
 beams

Longest span
 size C
 Beams
 load on beams

5m
 25m

single span
 double span
 longest span = 10m
 5m each

Primary structure
 Secondary structure
 2nd degree structure

need to calculate space
 reaction

Calculations:
 A = roof sheets for 2nd degree structure
 B = primary beam loads
 C = longest span
 D = key connections
 E = loads onto columns / floor slabs

Roof sheets calculations

2.5m

width = 2m
 thickness = 80mm (core), overall = 11mm
 weight = 10.5 kN/m²
 cover width = 1000mm
 core thickness = 80mm

Standard length = 1.9-14.5m
 I need 2.5m, fits in standard length

25 years structural and thermal performance guarantee

Load/Span table
 load = 2.85 kN/m²
 VDL = 2.85 kN/m

Roof panels calc
 SINGLE SPAN

force applied a VDL of 2.85 kN/m²

single span
 double span
 triple span

double spans: 1.85 kN/m²
 triple span: 2.19 kN/m²

just in case
 as most spans are 2.5m

I want to have sheets width 5m / 3 = 1.6m

but can only have 1m width...

tributary area = 1.6m x 2.5m
 2.85 kN/m² x 1m = 2.85 kN/m
 2.85 kN/m x 2.5m = 7.125 kN

How to resolve this and size it?

Force Reactions
 deflections
 SFD
 BMD

2.85 kN/m = w kN/m
 VDL = 2.85 kN/m

Total load on beam = 7.125 kN
 Reactions
 V_A = 3.56 kN
 V_B = 3.56 kN

deflection
 max bending moment = $\frac{wL^2}{8} = \frac{2.85 \times 2.5^2}{8} = 2.23$ kNm
 max deflection = $\frac{5wL^4}{384EI}$

Now that we know the loads, we can calculate the max deflection to find the right beam in the blue book.

Max deflection = $\frac{5wL^4}{384EI}$

where
 w = weight = 2.85 kN/m
 L = length = 2.5m
 E = Young's modulus = 180 kN/mm² = 180 x 10³ kN/m²
 I = Moment of Inertia

We also need to find
 δ_{limit}
 from Eurocodes 3 → δ_{limit} ≤ $\frac{L}{200}$

What units to use?
 Where to find I in Blue Book



$$I \geq \frac{1000}{384} \times \frac{WL^3}{E}$$

$$I \geq \frac{1000}{384} \times \frac{(2.85 \times 10^3) \times (2.5)^3}{(180 \times 10^9)}$$

$$I \geq \frac{1000}{384} \times \frac{2.85 \times 2.5^3}{180 \times 10^9}$$

$$I \geq 6.44 \times 10^{-7} \text{ m}^4$$

$$I \geq 6.44 \times 10^{-7} \text{ m}^4$$

$$I \geq 6.44 \text{ cm}^4$$

too low? unit issue?

VB 256.17 x 1.69

WHAT UNIT DO I HAVE TO USE?

STRENGTH REDUCTION FACTOR = 0.85

$$I \geq \frac{1000 \times 2.85 \times 10^3 \times 2.5^3}{384 \times 180 \times 10^9}$$

$$I \geq 0.00000644$$

$$\delta_{max} = \frac{5WL^4}{384EI}$$
 where:

$W = \text{load per unit weight} = 2.85 \text{ kN/m} = 2850 \text{ Pa}$
 $L = \text{length} = 2.5 \text{ m}$
 $E = \text{Elastic Modulus} = 180 \text{ GPa} = 180 \times 10^9 \text{ Pa}$

$I = \text{Moment of Inertia}$

$$\delta_{limit} \leq \frac{L}{200}$$
 where:

$L = \text{length} = 2.5 \text{ m}$

$$\delta_{max} \leq \delta_{limit}$$

$$\frac{5WL^4}{384EI} \leq \frac{L}{200}$$

$$\frac{5WL^3}{384E} \leq \frac{1}{200}$$

$$5WL^3 \times 200 \leq I$$

Questions for Harry:

Look at (1-1) and (2-2) in blue beam.

is $\frac{L}{200}$ in mm? or m?

$$\delta_{max} = \frac{5WL^4}{384EI}$$
 where:

$W = \text{load per unit weight}$
 $L = \text{length}$
 $E = \text{Elastic Modulus}$
 $I = \text{Moment of Inertia}$

All in mm? or convert

$$\delta_{limit} = \frac{L(\text{mm})}{200} = \frac{2.5 \text{ m}}{200} = \frac{2500 \text{ mm}}{200} = 12.5 \text{ mm}$$

$$\delta_{max} \leq \delta_{limit}$$

$$0.0125 \geq \frac{5(2.85 \times 10^3)(2.5)^4}{384(180 \times 10^9)I}$$

$$I \geq \frac{5(2.85 \times 10^3)(2.5)^4}{384(180 \times 10^9)0.0125} = 2.57 \times 10^{-6} \text{ m}^4 = 25.7 \text{ cm}^4$$

$$\delta_{max} = \frac{5WL^4}{384EI}$$

$$0.000644$$

$$6.4 \times 10^{-4}$$

B - Try longest span
 C - Try load onto columns

$$I \geq 6.4 \times 10^{-4} \text{ m}^4$$

x 0.85 safety factor

Mega is 10^6
 Giga is 10^9

$$I \geq 0.85 \times 6.4 \times 10^{-4} \text{ m}^4$$

$$I \geq 5.44 \times 10^{-4} \text{ m}^4$$

$$I \geq 6.4 \times 10^{-4} \text{ m}^4$$

UB 610 x 177 x 100 → 12500 cm⁴

Figure 1: What type of beam can withstand load from A?

First, let's define the dead loads:
 Looking at Kingspan metal roofing with a core thickness of 80 mm:

Roofing = 0.8 kN/m² (Suction)
 Services = 0.1 kN/m²
 Clay tiles = 0.85 kN/m²
 Wind uplift = 0.8 kN/m²

Total = 1.8 kN/m²

primary area = 555 m² → 1.8 kN/m² × 555 m² = 1000 kN

$$\sum F_H = H_A - H_B = 0$$

$$\sum F_V = V_A - V_B = \frac{WL}{2} - \frac{WL}{2} = \frac{(5 \times 10)}{2} - \frac{(5 \times 10)}{2} = 0$$

$$\delta_{max} = \frac{5WL^4}{384EI}$$

$$\delta_{limit} \leq \frac{L}{200}$$

As in section A we have 5 roof panels, the total load on the beam:

$$5 \times 9 \text{ kN} = 45 \text{ kN}$$

We will now size the beams on both sides of section A:

$$\delta_{max} = \frac{5WL^4}{384EI}$$

$$\delta_{limit} \leq \frac{L}{200}$$

where:

$W = 9 \text{ kN/m} = 9000 \text{ N/m}$
 $L = 5 \text{ m}$
 $E = \text{Young's modulus} = 180 \times 10^9 \text{ N/m}^2$
 $I = \text{Moment of inertia}$

$$\delta_{max} \leq \delta_{limit}$$

$$\frac{5WL^4}{384EI} \leq \frac{L}{200}$$

$$\frac{5(9000)(5)^4}{384(180 \times 10^9)I} \leq \frac{5}{200}$$

$$I \geq \frac{5(9000)(5)^4(200)}{5(384)(180 \times 10^9)}$$

$$I \geq 1.6276 \times 10^{-5} \text{ m}^4$$

$$\therefore I \geq 1627.60 \text{ cm}^4$$

A suitable beam would be:

UB 157 x 101 x 82
 as its $I = 1870 \text{ cm}^4$
 with a SF of 25% it's 1589 cm⁴
 1589 < 1627.6

We need another beam, let it be:

UB 203 x 102 x 23
 with $I = 2100 \text{ cm}^4$
 and applying a SF of 85%
 we get $I = 1785 \text{ cm}^4$
 as 1785 < 1628 cm⁴
 it is a suitable beam

3: Solving for section B, having already sized U section A, let's see if the steel beams have to be changed as they overlap with section A as seen in fig X.

The tributary load changes, from $5 \times 5m$ to $10 \times 5m$ (width).

$1.8 \text{ kN/m}^2 \times 10 = 18 \text{ kN/m}$

$\Sigma F_H = 0$
 $\Sigma F_V = 0$ where $V_A = V_B = \frac{wL}{2} = 45 \text{ kN}$

So there is a change in w in the formula:

$S_{max} \leq S_{limit}$
 $\frac{5wL^4}{384EI} \leq \frac{L}{200}$

$I \leq \frac{5(18000)(5)^4(200)}{3(374)(180 \times 10^9)}$
 $I \leq 3255 \text{ cm}^4$

with $I = 4000 \text{ cm}^4$
 with SF $I = 3400 \text{ cm}^4$
 $3400 > 3255$
 Beam is suitable

So we need to change section A from UB $203 \times 102 \times 23$ to UB $254 \times 102 \times 28$

Knowing that for a longer span in section C (needed because of crane space under ceiling in another space), the load per unit weight is smaller ($0.67 \text{ kN/m}^2 < 0.8 \text{ kN/m}^2$), therefore the UB $254 \times 102 \times 28$ is suitable.

However a thicker core thickness is used (150 mm instead of 80).

To avoid any sagging in the center, secondary wood purlins are put.

Wood purlin = 0.1 kN/m^2

added weight = $(5 \times 0.1) + (2 \times (10 \times 0.1)) = 2.5 \text{ kN/m}$

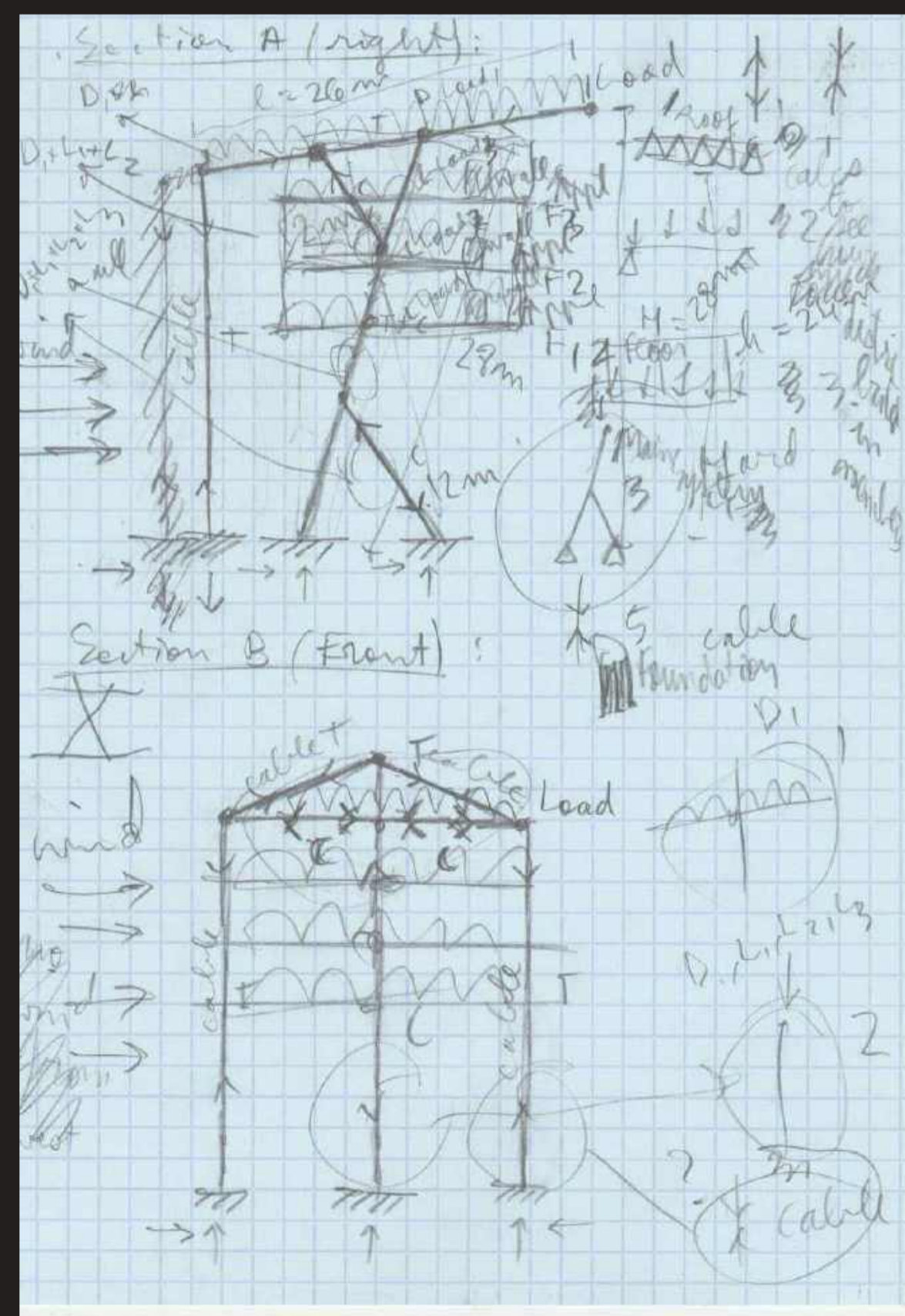
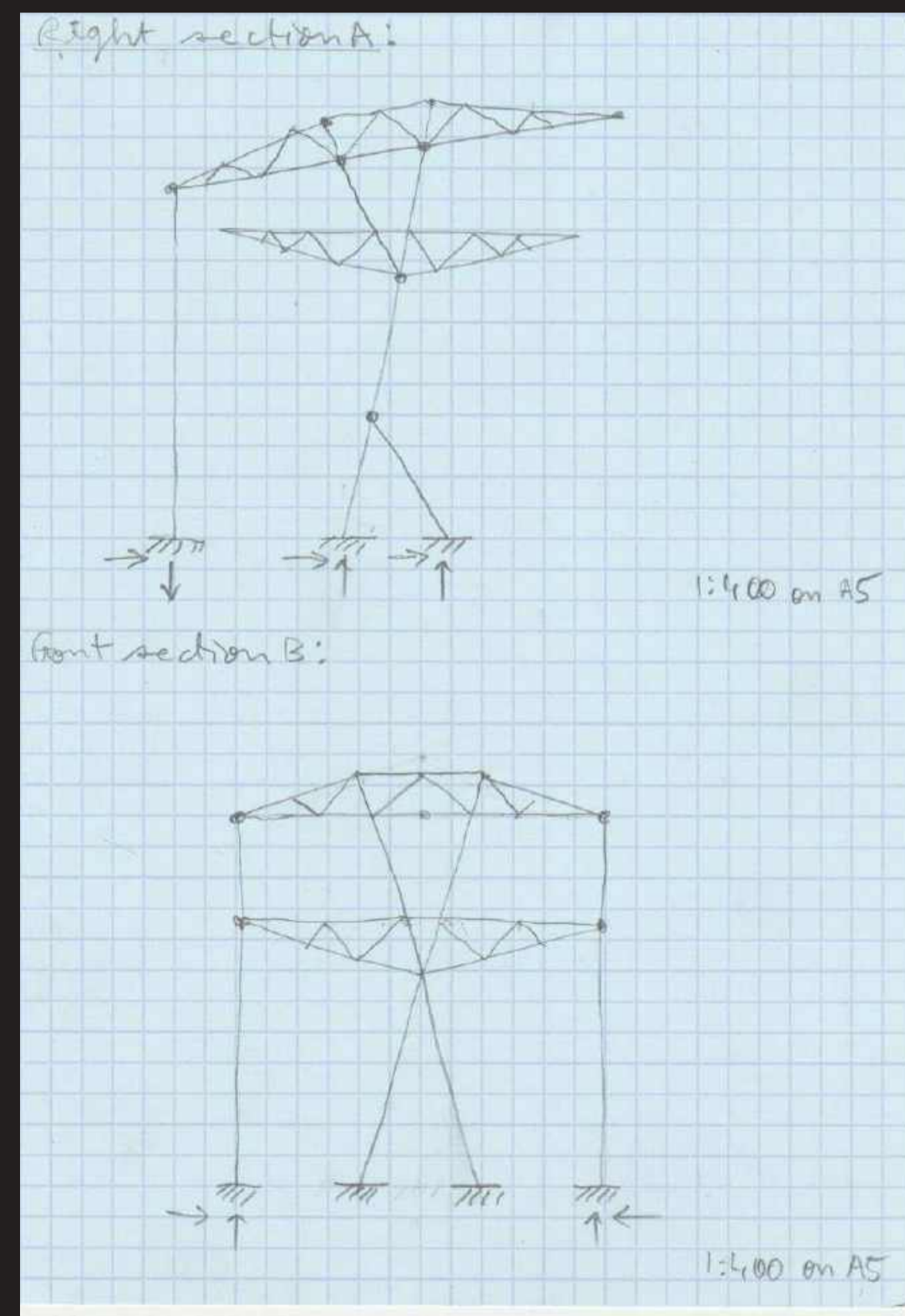
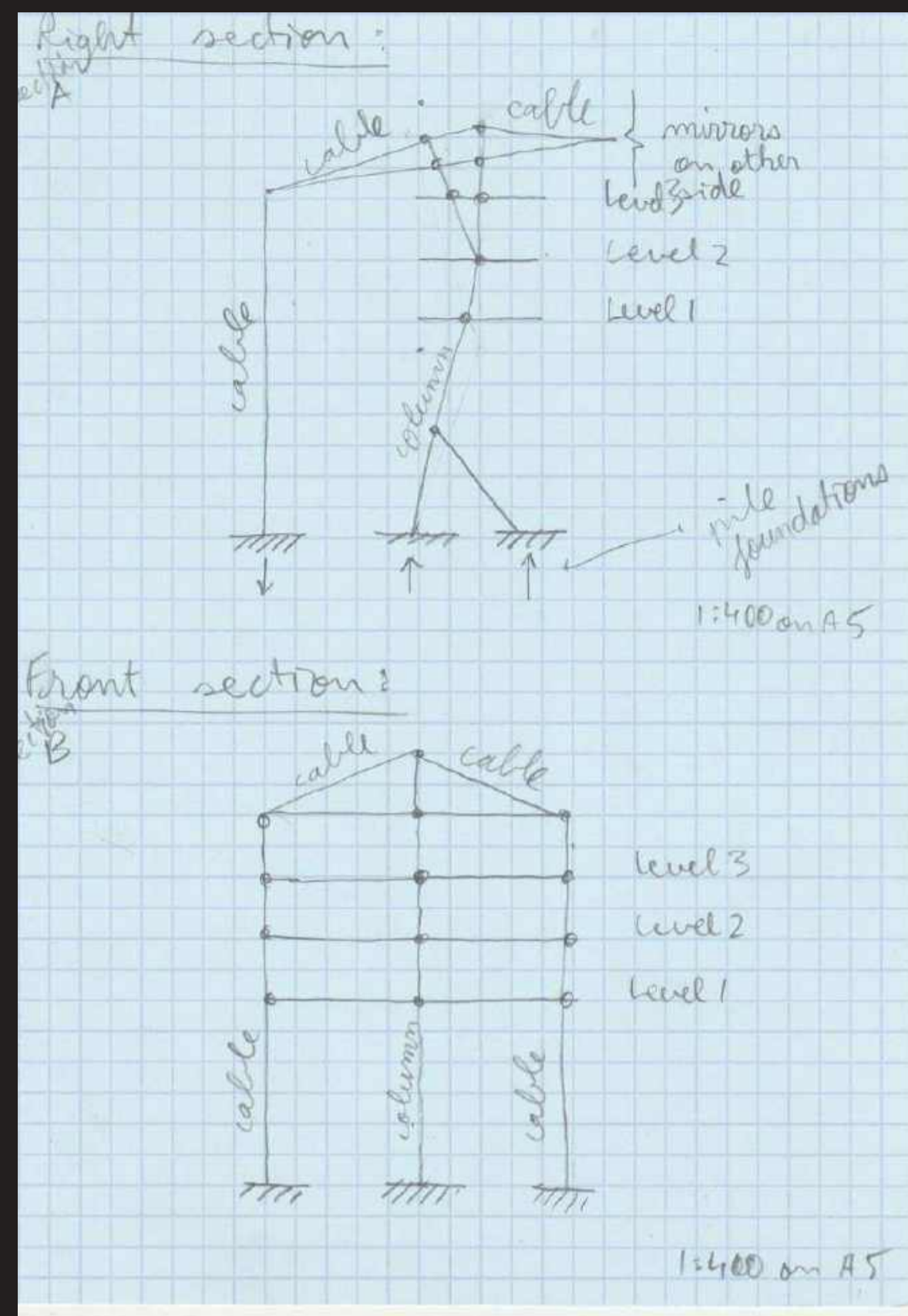
added weight = $5 \times 0.1 = 0.5 \text{ kN/m}$

5: How much weight is put onto columns?

loads

- Wind Purlin (with insulation) = 0.8
- Services = 0.1
- Secondary structure = 2.1
- Roof sheets = 0.1
- Self weight = 1.9
- Total = 4.0

at or below 0.1 kN/m^2



Combined Bending and Direct Stress

$N/P = 2.2 \text{ kN/m}^2$

assume 2.2 kN/m^2 arrives immediately from the top

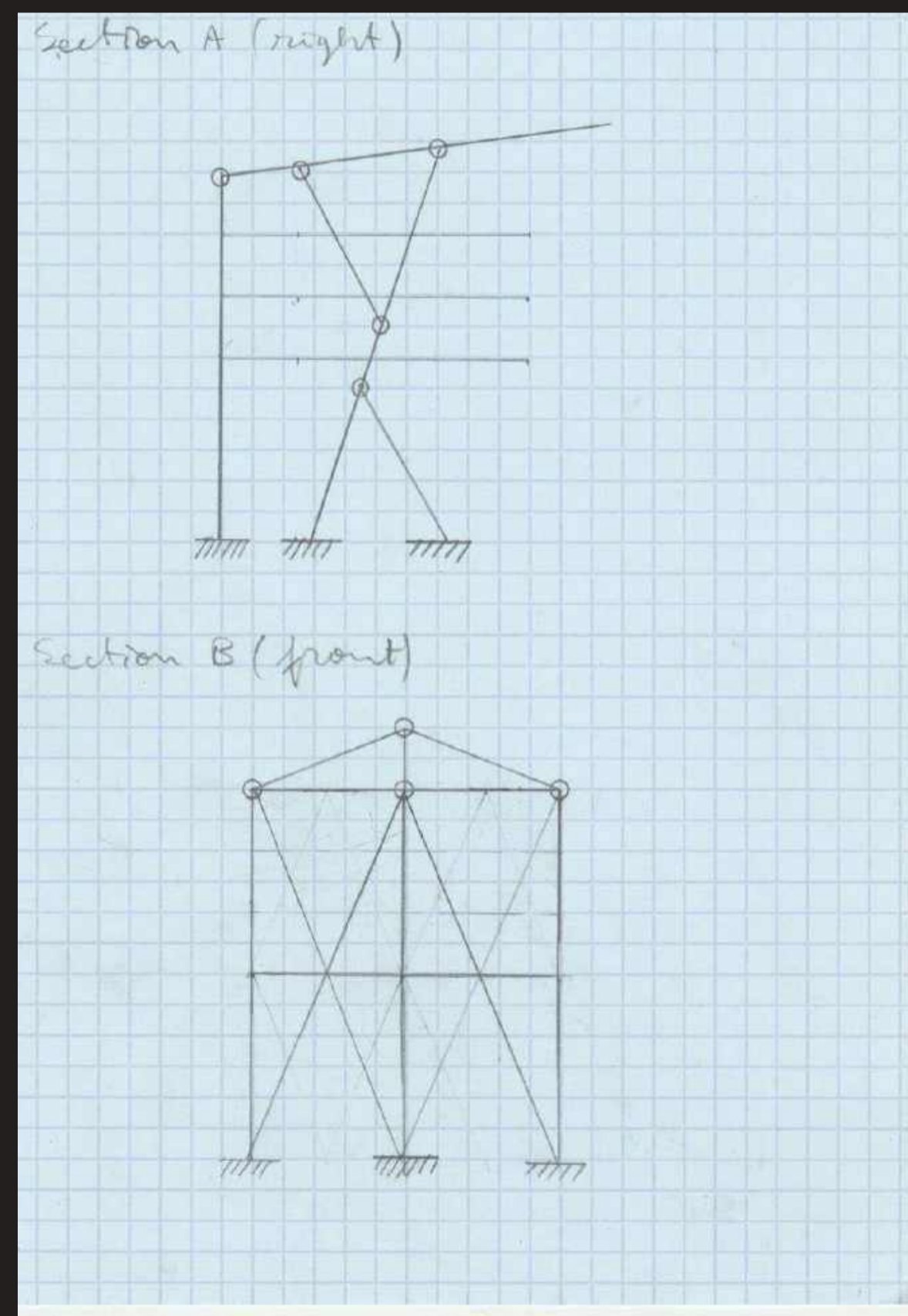
combination of axial load and bending about 2 axes

$P = \text{dead load} + 3(\text{live loads})$

where dead load = $2.1 \text{ kN/m}^2 + 3(0.95)$
 and live load = 1.7 kN/m^2

Total $P = 4.95 + 5.1 = 10.05 \text{ kN/m}^2$

What do I need to find?



Loads:
 Roof: $0.8 + 0.1 + 0.8 + 0.1 + 0.1 = 1.9$ kN/m^2 area 120
 Walls: $0.15 + 0.05 + 0.05 + 0.1 = 0.35$ $8\text{m} \times 8\text{m}$
 Floor 1 live: 4 8×8
 Floor 2 live: 2.5 8×8
 Floor 3 live: 5 12×12

Roof = $1.9 \times 120 = 228$ kN
 Floor 1 = $4 + (0.35 \times 8) + (0.35 \times 8) = 6.4$
 Floor 2 = $2.5 + (0.35 \times 8) + (0.35 \times 8) = 5.4$
 Floor 3 = $5 + (0.35 \times 12) + (0.35 \times 12) = 10.4$

hanging structure
 secondary
 main beam
 adds weight
 cantilever?

10 positions
 10m
 12m
 2.2m

Total = 1554 kN
 Point load for roof = $1.9 \times 160 = 304$ kN
 TOTAL = 300.6 kN

Calculate what point load is applied on the braced columns

$P = 1194$ kN
 $P/2 = 597$ kN

OVERALL STABILITY HAS TO BE ASSURED

Find loads $P/2$
 $H = 320$ mm
 $L = 32$ m
 $P = 1194$ kN
 $P/2 = 597$ kN
 $\alpha = 60^\circ$

$H_A = 597$ kN
 $H_F = \frac{597}{\tan 60} = 345$ kN
 $V_A = 597$ kN
 $V_B = 597$ kN
 $H_A = H_B = 345$ kN

$\sum M = 0$
 $M_A = M_B = 30 \times 345 = 10340$ kNm

2 - find Buckling, α (limit and max bending moment?) to find how many columns bracing we need.

Euler column formula:
 $F = \frac{n \pi^2 EI}{L^2}$

where
 $F =$ allowable load
 $n =$ factor accounting for the end conditions
 $E =$ Young's modulus
 $L =$ length of column
 $I =$ Moment of inertia

$n = 0.25$
 one fixed, one free

Slenderness ratio for long columns
 L/r
 where
 $L =$ height
 $r =$ radius of gyration

$120 < L/r < 200$

0.00158

Stress \rightarrow allowable stress
 $F = \frac{n \pi^2 EI}{L^2}$
 $689 = \frac{0.25 \pi^2 E (100 \times 10^9) I}{32^2}$
 $I = \frac{FL^2}{n \pi^2 E} = \frac{689 \times 32^2 \times 10^3}{0.25 \times \pi^2 \times 100 \times 10^9}$
 $I = 1.58857 \times 10^{-3} \text{ m}^4$

$\frac{\text{Stress allowable}}{\text{connectional area}} \rightarrow \text{Fallowable}$
 $\frac{32}{0.175} = 182 < 200$
 Passes slenderness ratio
 Where to find allowable stress?
 $\frac{689 \times 10^3}{0.595} \leq \text{allowable}$
 $1157983 \leq \text{allowable}$
 $1.16 \text{ MPa} \leq \text{allowable}$
 Buckling parameter = 0.837
 $I \times 0.837 = 83000 \times 0.837 = 153171$
 $153171 < 158857$
 need bigger
 $VC \times 406 \times 509$
 446×416

$\sigma_{max} = \frac{M}{I} \times \frac{h}{2}$
 $\frac{689 \times 10^3}{2.04 \times 10^3} \times \frac{32}{2}$
 $689000 = 8.109 \times 10^{10}$
 $\sigma_{max} = \frac{10340 \times 10^3}{2.04 \times 10^3} \times 16$
 $= 8.109 \times 10^{10}$
 $= 81 \text{ GPa}$
 my stress = $\frac{689 \times 10^3}{0.595} = 1.16 \text{ MPa}$
 use a random I
 $P \leq F_{\text{Buckling}}$
 $F_{\text{Buckling}} = \frac{0.25 \pi^2 (180 \times 10^9) (2.04 \times 10^3)}{32^2}$
 $= 884.774 \text{ kN}$
 $689 < 884 \therefore \text{works}$

$F_{\text{allowable}} = \frac{\pi^2 EI}{L^2}$
 Wind
 3
 4
 5

Wind Load:
 (increases moment at base)
 $W_e = q_p \times C_{pe}$
 where
 $q_p = C_e(z) \times q_b$
 $q_b = 0.613 V_b^2$
 $V_b = C_{dir} \times C_{season} \times C_{prob} \times C_{alt} \times V_{p,map}$
 where
 $C_{dir} = 1$; $C_{season} = 1$; $C_{prob} = 1$; $C_{alt} = 1$
 $V_b = 1 \times 1 \times 1 \times 22 = 22 \text{ m/s}$
 $q_b = 0.613 (22)^2 = 296.7 \text{ N/m}^2$
 $q_p = C_e(z) \times 296.7$
 $C_e(z) = 2.9$
 $q_p = 29 \times 296.7 = 8604.3 = 0.86 \text{ kN/m}^2$

Wind:
 $W_e = q_p \times C_{pe}$
 where
 $C_{pe} = +0.8 \text{ or } -0.7$
 as $h/d = \frac{12}{10} = 1.2$
 $W_{e,1} = 0.8 \times 0.8 = 0.64 \text{ kN/m}^2$ pushing on building
 and
 $W_{e,2} = 0.8 \times -0.7 = -0.56 \text{ kN/m}^2$ pulling on building
 $W_{e,1} + W_{e,2} = 0.64 - 0.56 = 0.08 \text{ kN/m}^2$
 $VC \ 356 \times 406 \times 509$
 $h = 446 \text{ m}$
 $b = 416 \text{ mm} = 0.416 \text{ m}$
 Surface on which wind acts
 Force of wind = $0.08 \times 10 \times 12 = 9.6 \text{ kN}$

VDL = $16.9 \text{ kN/m}^2 \times \frac{10}{4} = 42.25 \text{ kN/m}$
 VERTICAL CANTILEVER
 32 m
 4.8 kN
 $9.345 \times 32 = 300.24 \text{ kN}$
 $300.24 + 4.8 = 305.04 \text{ kN}$
 $132 = 20 \text{ mm}$
 $x = 1.54$
 $150 = 1.5 (181) = 271.5 \text{ mm}$

Vertical cantilever - Eiffel tower
 A
 B
 Both left and right cords experience forces of equal magnitude in response to horizontal wind loads as well as self-weight dead and live loads.
 (c) Force polygon
 Foundations

Graphical method of truss analysis:
 Wind
 Load from Building
 Foundations
 1-2
 3-4
 5-6
 7-8
 9-10
 11-12
 13-14
 15-16
 Brac e
 a, b, c, d
 do the same for columns



Designing trusses:

$$I_{allowable} = \frac{m \pi^2 E I}{L^2} = \frac{0.25 \pi^2 \times (180 \times 10^3) I}{4^2}$$

$$\frac{288 \times 4^2 \times 10^3}{0.25 \pi^2 (180 \times 10^3)} = I = 1.037 \text{ cm}^4$$

VC 152 x 152 x 38
 $I = 1750 \text{ cm}^4$

$$I_{allowable} = \frac{0.25 \pi^2 \times 180 \times 10^3 \times 1750 \times 10^{-8}}{4^2}$$

$$= 485.77 \text{ kN}$$

Find force in each member

D/N JOINT FRAME STRUCTURE

$$M = 2J - 3$$

$$8 = 2(5) - 3$$

$$= 10 - 3 = 7$$

$8 > 7$ ∴ imported and cannot be completely analysed

METHOD OF RESOLUTION AT JOINTS

determine the forces in each member
 use Buckling or $\sigma_{max} = \sigma_{allow}$

$$F_{AB} = 48 \text{ kN} = F_{CD}$$

$$F_{AE} = 7 \text{ kN} = F_{DC}$$

$$F_{BC} = 3.2$$

$$F_{BE} = \frac{48}{\cos 27} = 54$$

$$H = \sqrt{48^2 + 3.2^2} = 48$$

$$F_{AB} = F_{DC} = 48$$

$$m = 2$$

$$j = 5$$

$$M = 2J - 3$$

$$= 2(5) - 3 = 7$$

$8 > 7$ ∴ imported and cannot be completely analysed

pile foundations

$$N = 1 - \frac{e}{90} \left[\frac{(m-1)m + (m-1)m}{mm} \right]$$

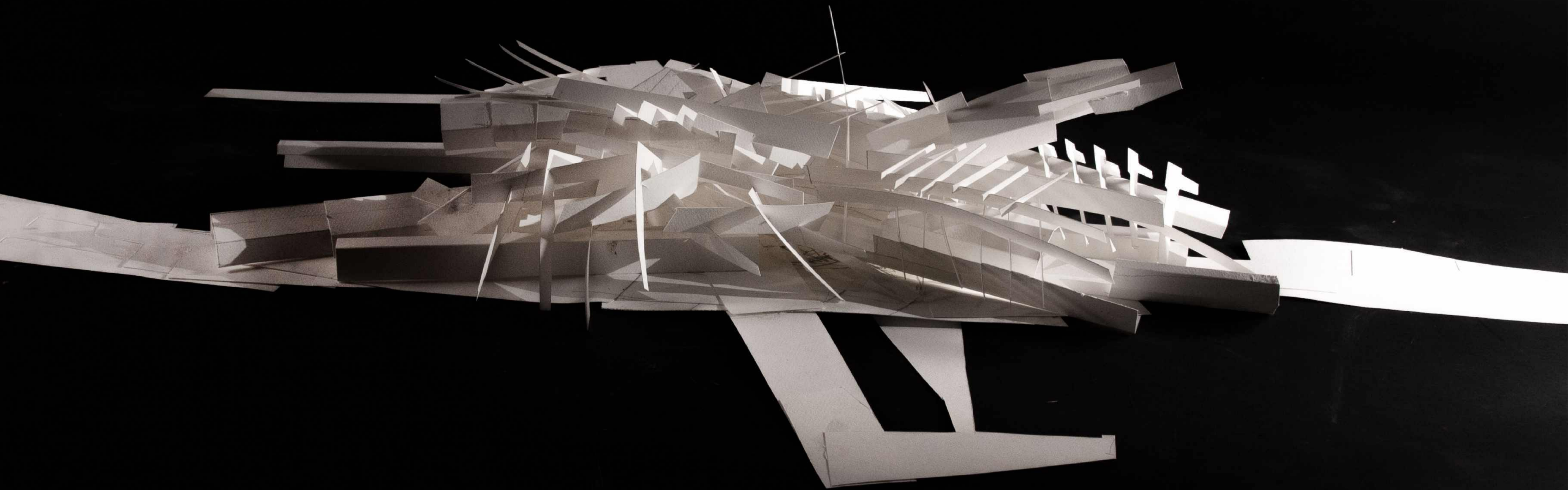
where
 $m =$ number of rows
 $n =$ number of piles
 $e =$ arctan d/s in degrees
 $d =$ pile diameter
 $s =$ pile spacing, center to center

We will be using pile foundations as it works best in clay soils.

$$N = 1 - \frac{e}{90} \left[\frac{(m-1)m + (m-1)m}{mm} \right]$$

where
 $m =$ number of rows
 $n =$ number of piles
 $e =$ arctan d/s in degrees
 $d =$ pile diameter
 $s =$ pile spacing, center to center

$$\begin{matrix} \odot & \odot & \odot \\ \odot & \odot & \odot \end{matrix} \quad \begin{matrix} m = 2 \\ m = 6 \end{matrix}$$

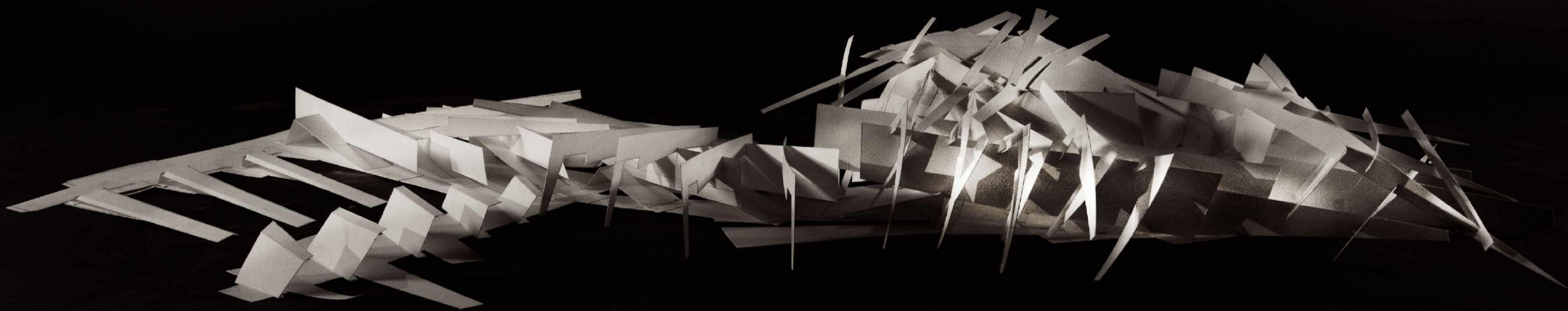





Maxime Ostroverhy

Max Ostroverhy (b. 2000) is a Franco-British artist and aspiring architect and engineer that speaks four languages fluently with extensive knowledge in German and Russian culture. Since his childhood, he has been interested in creating all sorts of things with his hands. His unconventional Parisian education at Steiner-Waldorf School introduced him to a range of creative arts from dancing, singing, drawing and working with wood. This passion and interest can be reflected in his latest work: his architectural diploma, in which he linked architecture with ceramics, an activity he used to do when he was a young child every Wednesday. This love for creativity was united with his love of nature best during the holidays where he used to go landscape painting with his father in the Alps and the French Riviera. There he learnt to paint landscapes, draw plants and people with meticulous care and foster his love for swimming, climbing and long-distance trail running.

From Munich, Bristol, the French Riviera and London, Max was able to develop his drawings and painting skills, especially in Bristol where he developed a passion for chalk graffiti. It has continued to help him to this day, in producing not only works of artistic beauty but works of practical use. The earliest example of this cooperation between the artistic and the practical is when Max was four. At the time, he designed a machine that would tear down all the publicity that can be found in the Parisian underground stations so that it would replace the walls with his father's artworks. A more recent example could be his recent architectural drawings and his Venetian studies that have now been incorporated into his architectural diploma. Max is an individual that is keen in issuing out statements to the world, aided by his love for art, and the unmet desire to reconcile the modern with the ancient, the beautiful with the practical, the artistic with the scientific, the rational with the impulsive. Perhaps that energy has been best reflected when Max was 16 years old, when he decided to sleep during winter on the balcony with only a tarp for a cover, for reasons that remain unclear to this very day.




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